

CELL / MODEL NAME	DESCRIPTION	DATE
BSD-1	Bar splicer details	10/22/2004
DS-1	Steel drainage scupper with bituminous surface	10/22/2004
DS-2	Cast iron drainage scupper with bituminous surface	10/22/2004
DS-3	Steel drainage scupper without bituminous surface	10/22/2004
DS-4	Cast iron drainage scupper without bituminous surface	10/22/2004
EJ-BJS (1 of 2)	Bridge Joint System (Expansion) Sheet 1 of 2	10/22/2004
EJ-BJS (2 of 2)	Bridge Joint System (Expansion) Sheet 2 of 2	10/22/2004
EJ-CS	Neoprene Expansion Joint	10/22/2004
E-S	Top of slab elevations	10/22/2004
FJ-BJS	Bridge Joint System (Fixed)	10/22/2004
G-1	Plate girder details	10/22/2004
I-2	Rocker bearings	10/22/2004
I-2-B	Rocker bearings	10/22/2004
I-2-C	Rocker bearings, diaphragms and splice details	10/22/2004
I-2-D	Diaphragm and splice details	10/22/2004
I-2-E1	Type I elastomeric bearing	10/22/2004
I-2-E2	Type II elastomeric bearing	10/22/2004
I-2-E3	Type III elastomeric bearing	10/22/2004
I-2-G	Rocker bearings	10/22/2004
R-20	Type L (Two element aluminum rail)	10/22/2004
R-23A	Type S1 (Single element side mounted rail)	10/22/2004
R-24A	Type T-1 (Two element side mounted rail)	10/22/2004
R-25	Temporary bridge rail	10/22/2004
R-26	Type TP-1 (Triple element side mounted rail)	10/22/2004
R-27	Temporary concrete barrier	10/22/2004
R-28	Pedestrian railing	10/22/2004
R-29	Bicycle railing	10/22/2004
R-30	Type WT steel railing	10/22/2004
R-31	Steel bridge rail (Curb mounted) 2399	10/22/2004
R-32	Bridge fence railing (parapet mounted)	10/22/2004
R-33	Bridge fence railing (sidewalk mounted)	10/22/2004
R-34	Type SM steel bridge rail (side mounted)	10/22/2004
S-1-0 no skew	Super Plan & X-sect no skew (3 to 6 span steel bridge)	10/22/2004
S-1-D	Superstructure details (3 to 6 span steel bridge)	10/22/2004
S-1-L greater than 15 degrees	Super Plan & X-sect > 15 degrees ahead left (3 to 6 span steel bridge)	10/22/2004
S-1-L less than 15 degrees	Super Plan & X-sect <15 degrees ahead left (3 to 6 span steel bridge)	10/22/2004



CELL / MODEL NAME	DESCRIPTION	DATE
S-1-R greater than 15 degrees	Super Plan & X-sect > 15 degrees ahead right (3 to 6 span steel bridge)	10/22/2004
S-1-R less than 15 degrees	Super Plan & X-sect < 15 degrees ahead right (3 to 6 span steel bridge)	10/22/2004
S-2-0 no skew	Super Plan & X-sect no skew (2 span steel bridge)	10/22/2004
S-2-D	Superstructure details (2 span steel bridge)	10/22/2004
S-2-L greater than 30 degrees	Super Plan & X-sect > 30 degrees ahead left (2 span steel bridge)	10/22/2004
S-2-L less than 30 degrees	Super Plan & X-sect < 30 degrees ahead left (2 span steel bridge)	10/22/2004
S-2-R greater than 30 degrees	Super Plan & X-sect > 30 degrees ahead right (2 span steel bridge)	10/22/2004
S-2-R less than 30 degrees	Super Plan & X-sect < 30 degrees ahead right (2 span steel bridge)	10/22/2004
SA-1-0	Approach span for vaulted abutments with PPC I beams no skew	10/22/2004
SA-1D-0	Approach span for vaulted abutments with PPC I beams no skew	10/22/2004
SA-1D-L	Approach span for vaulted abutments with PPC I beams ahead left	10/22/2004
SA-1D-R	Approach span for vaulted abutments with PPC I beams ahead right	10/22/2004
SA-1-L	Approach span for vaulted abutments with PPC I beams ahead left	10/22/2004
SA-1-R	Approach span for vaulted abutments with PPC I beams ahead right	10/22/2004
SA-2-0	Approach span for vaulted abutments (sand filled) no skew	10/22/2004
SA-2-L	Approach span for vaulted abutments (sand filled) ahead left	10/22/2004
SA-2-R	Approach span for vaulted abutments (sand filled) ahead right	10/22/2004
SB-1	Cantilever forming brackets (W27 and smaller)	10/22/2004
SI-1-0	Super Plan & X-sect no skew (1 span with integral abutments)	10/22/2004
SI-1-L	Super Plan & X-sect ahead left (1 span with integral abutments)	10/22/2004
SI-1-R	Super Plan & X-sect ahead right (1 span with integral abutments)	10/22/2004
SI-2-0	Super Plan & X-sect no skew (2 span with integral abutments)	10/22/2004
SI-2-L	Super Plan & X-sect ahead left (2 span with integral abutments)	10/22/2004
SI-2-R	Super Plan & X-sect ahead right (2 span with integral abutments)	10/22/2004
SI-DS1	Integral abutment diaphragm details (for steel beams/girders > 27")	10/22/2004
SI-DS2	Integral abutment diaphragm details (for steel beams/girders 27" and smaller)	10/22/2004



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -

- SHEETS

Contract #

**NOTES**

Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length.

All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

- ① Minimum Capacity  
(Tension in kips) =  $1.25 \times f_y \times A_l$
- ② Minimum \*Pull-out Strength  
(Tension in kips) =  $1.25 \times f_{s_{allow}} \times A_l$

Where  $f_y$  = Yield strength of lapped reinforcement bars in ksi.

$f_{s_{allow}}$  = Allowable tensile stress in lapped reinforcement bars in ksi (Service Load)

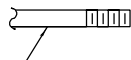
$A_l$  = Tensile stress area of lapped reinforcement bars.

\* = 28 day concrete

BAR SPLICER ASSEMBLIES			
Bar Size to be Spliced	Splicer Rod or Dowel Bar Length	Strength Requirements	
		Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension
#4	1'-8"	14.7	5.9
#5	2'-0"	23.0	9.2
#6	2'-7"	33.1	13.3
#7	3'-5"	45.1	18.0
#8	4'-6"	58.9	23.6
#9	5'-9"	75.0	30.0
#10	7'-3"	95.0	38.0
#11	9'-0"	117.4	46.8

Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."

The diameter of this part is the same as the diameter of the bar spliced.

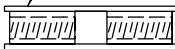


ROLLED THREAD DOWEL BAR



\*\* ONE PIECE

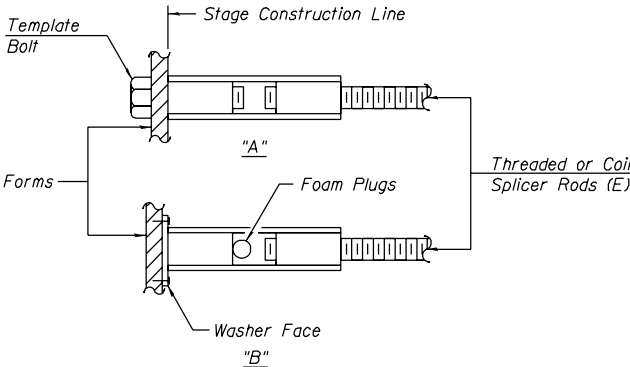
Wire Connector



WELDED SECTIONS

**BAR SPLICER ASSEMBLY ALTERNATIVES**

\*\* Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.

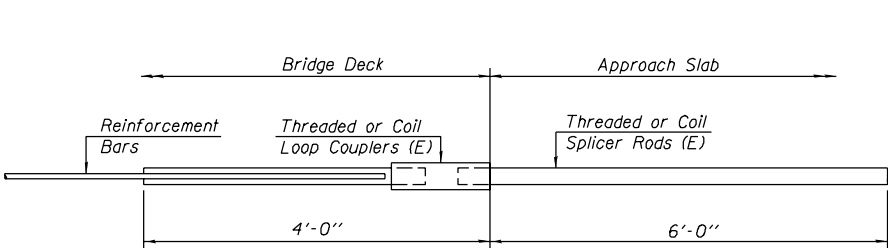


**INSTALLATION AND SETTING METHODS**

"A" : Set bar splicer assembly by means of a template bolt.

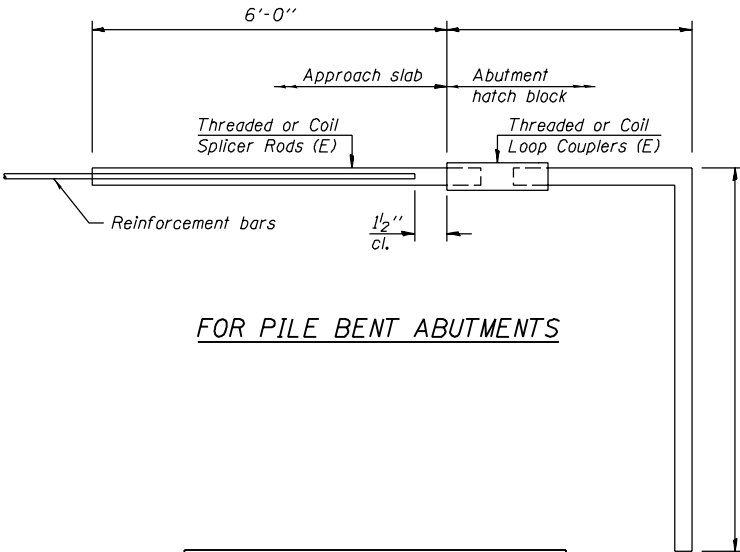
"B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E) : Indicates epoxy coating.



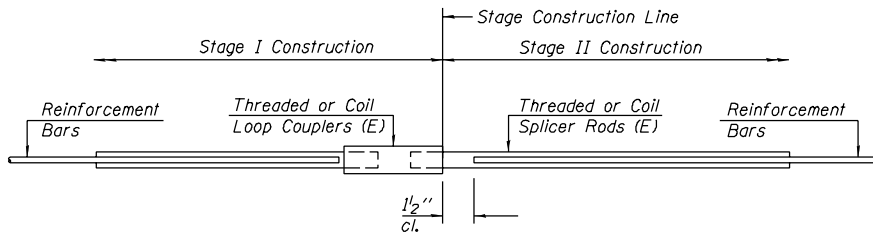
**FOR INTEGRAL OR SEMI-INTEGRAL ABUTMENTS**

Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 9.2 kips - tension
No. Required =



**FOR PILE BENT ABUTMENTS**

Bar Splicer for #5 bar
Min. Capacity = 23.0 kips - tension
Min. Pull-out Strength = 9.2 kips - tension
No. Required =



**STANDARD**

Bar Size	No. Assemblies Required	Location

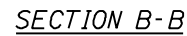
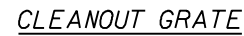
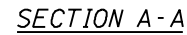
**BAR SPLICER ASSEMBLY DETAILS**

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

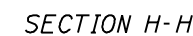
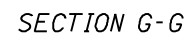
-	200
EXAMINED	ENGINEER OF BRIDGE DESIGN
PASSED	ENGINEER OF BRIDGES AND STRUCTURES



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-		- SHEETS



VIEW D-D



ITEM	UNIT	QUANTITY
Drainage Scupper	Each	

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

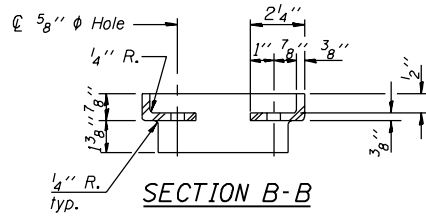
(W.T. to inside of exterior stringer flange shall not be  $> 3'-11''$ )

SECTION G-G

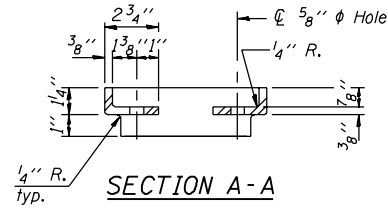


SHEET NO. -  
- SHEETS

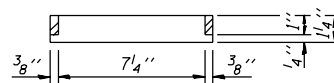
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION



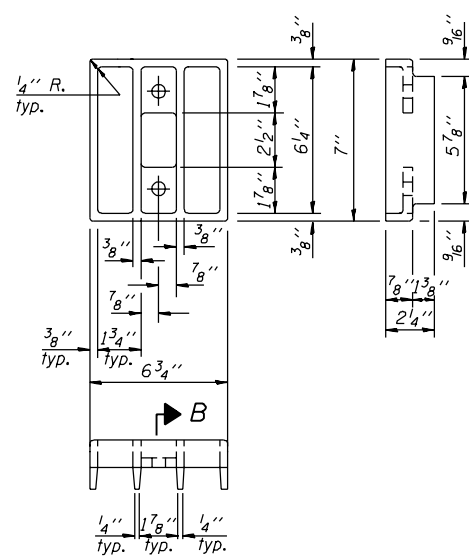
SECTION B-B



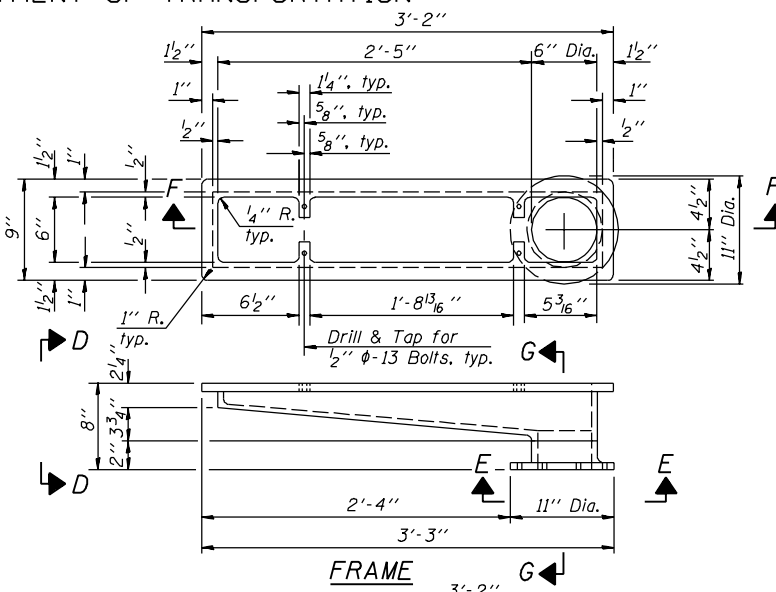
SECTION A-A



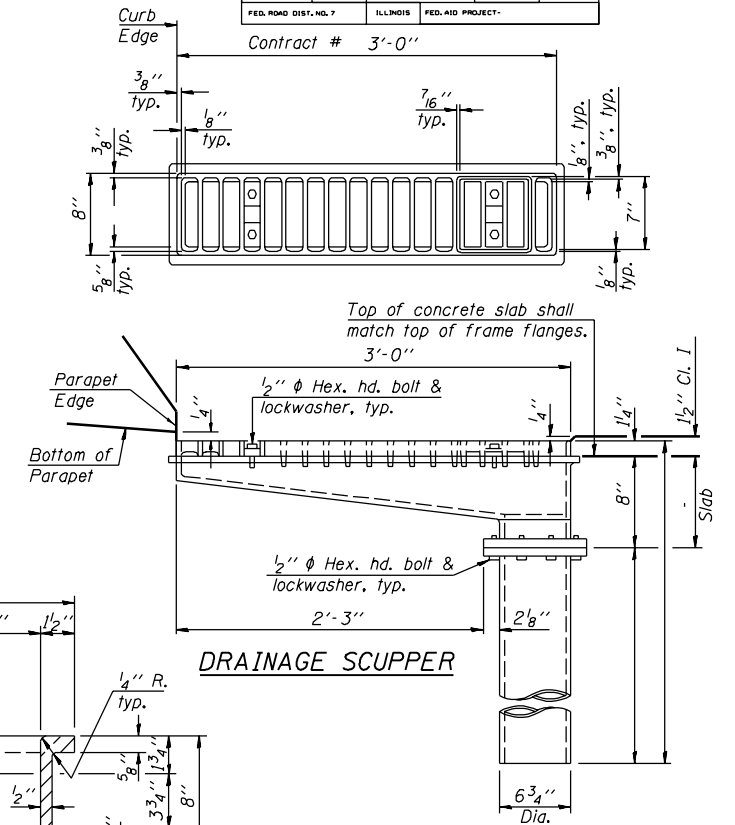
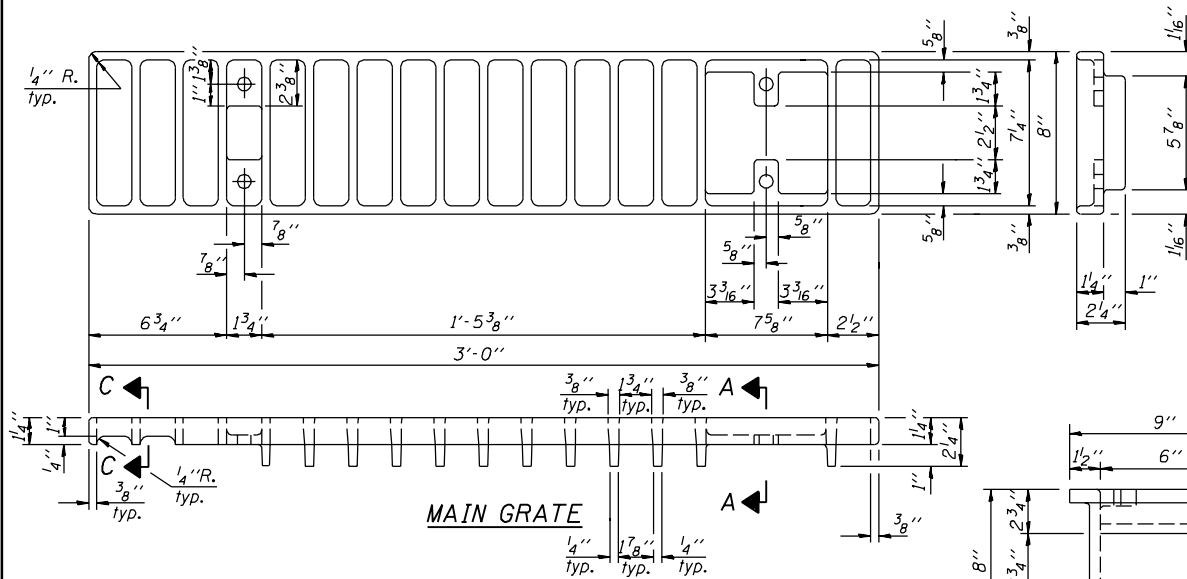
SECTION C-C



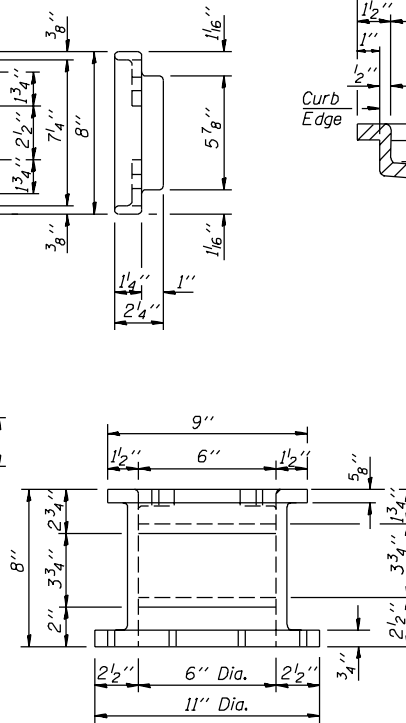
CLEANOUT GRATE



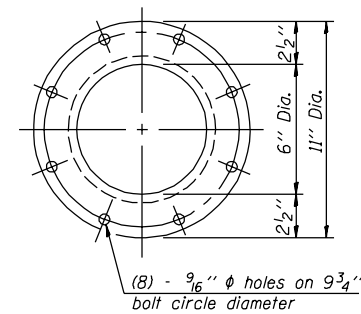
FRAME

DRAINAGE SCUPPER

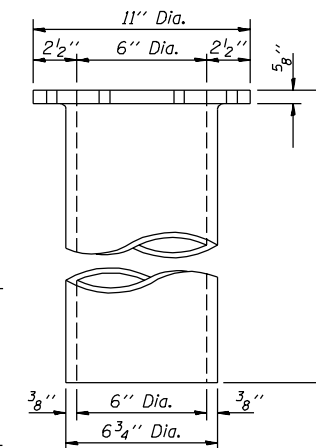
MAIN GRATE



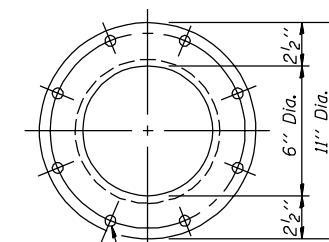
VIEW D-D



(8) -  $\frac{9}{16}$ "  $\phi$  holes on  $9\frac{3}{4}$ '  
bolt circle diameter



*DOWNSPOUT*



SECTION F-F

VIEW E-E

**Notes:**  
*All cast iron parts shall be gray iron conforming to the requirements of AASHTO M 105, Class 30.*  
*Bolts, washers and nuts shall conform to the requirements of ASTM A 307.*  
*All bolts, washers and nuts shall be galvanized according to AASHTO M 232.*  
*The waterproofing membrane system shall be installed such that the membrane covers the frame flanges and extends down into the frame with the grates placed on top of the membrane.*  
*Cost of the Main Grate, Cleanout Grate, Frame, Downspout, Bolts, Washers and Nuts including complete installation of Scupper shall be paid for at the unit bid price each for "DRAINAGE SCUPPERS".*  
*The Contractor may use at his option steel frames and steel grates or cast frames and cast grates, but will not be allowed to use steel grates with cast frames nor cast grates with steel frames.*

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

10-22-04 (W.T. to inside of exterior stringer flange shall not be  $> 3'-11''$ )

(Sheet 2 of 2)

ALTERNATE - CAST IRON  
DRAINAGE SCUPPER

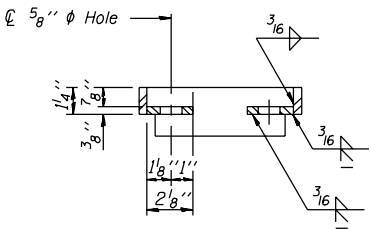


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

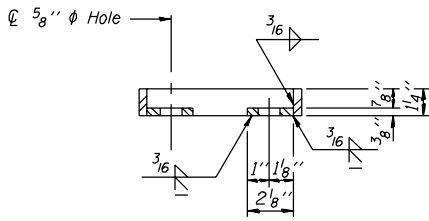
ROUTE NO.	SECTION	COUNTY	100% SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -  
- SHEETS

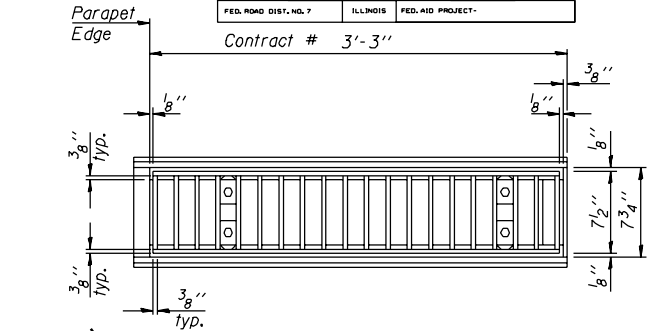
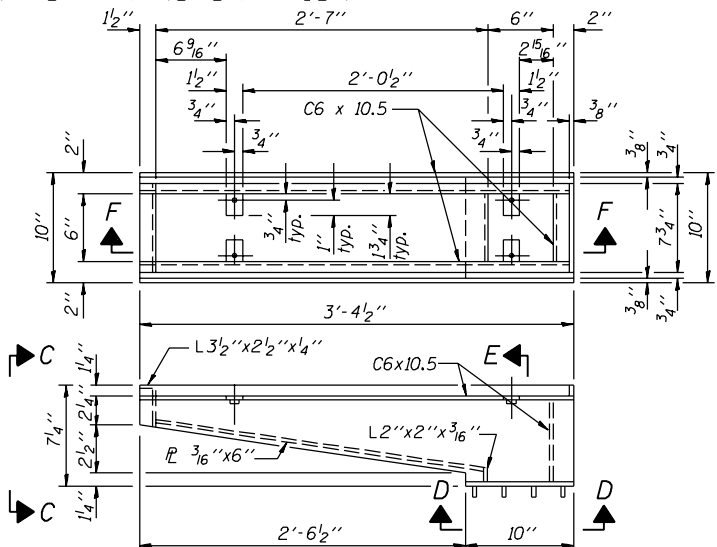
Contract # 3'-3"



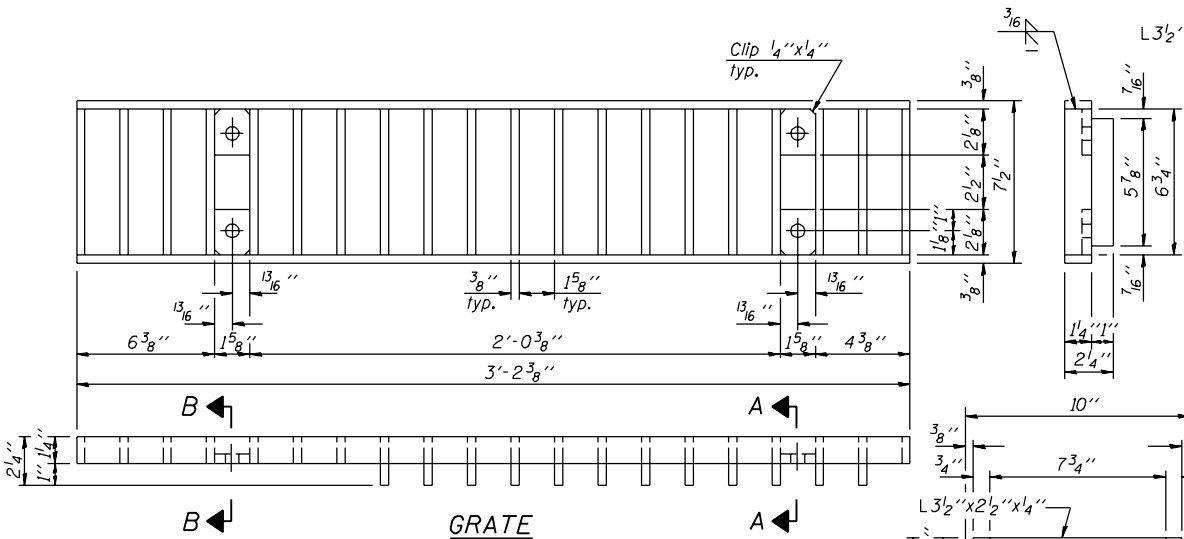
SECTION A-A



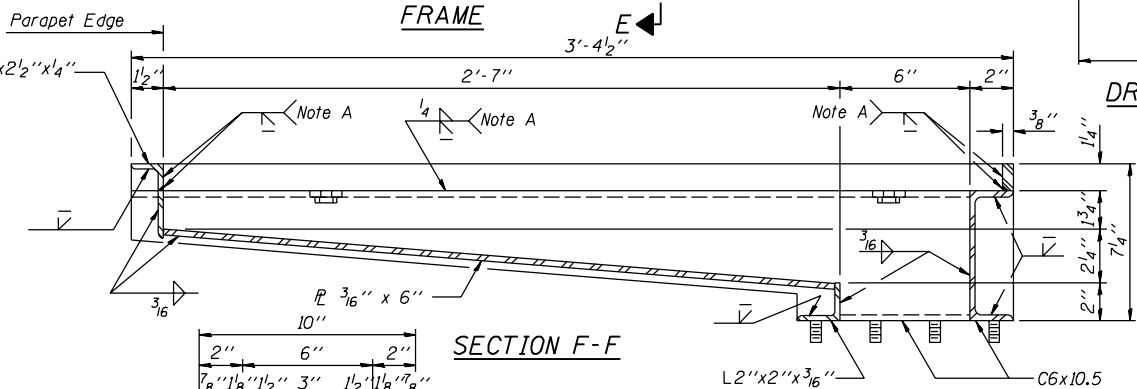
SECTION B-B



DRAINAGE SCUPPER



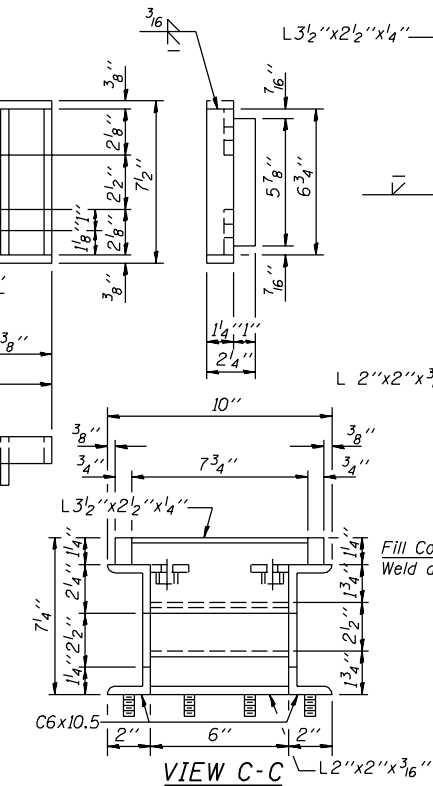
GRATE



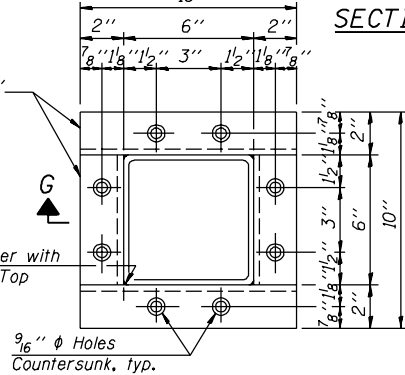
SECTION F-F

Note A: Surface of welds shall be recessed 1/16" Max. or placed flush with inside face of bars to provide clearance for Grate.

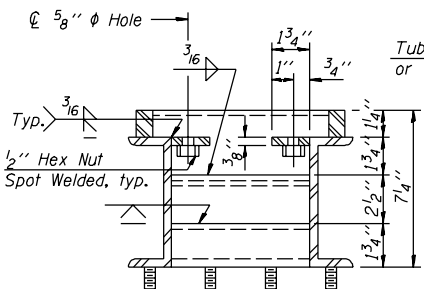
Notes:  
Hollow structural steel tubing shall conform to the requirements of ASTM designation A 500 Grade B, or A 501 Structural Steel Tubing.  
All other shapes, plates and bars shall conform to the requirements of AASHTO M 270 Grade 36.  
Bolts, studs, washers and nuts shall conform to the requirements of ASTM A 307.  
The Grate, Frame and Downspout shall be galvanized after shop fabrication according to AASHTO M 111 & ASTM A 385.  
All bolts, washers and nuts shall be galvanized according to AASHTO M 232.  
Cost of the Grate, Frame, Downspout, Bolts, Washers and Nuts including complete installation of Scupper will be paid for at the unit bid price each for "DRAINAGE SCUPPERS."



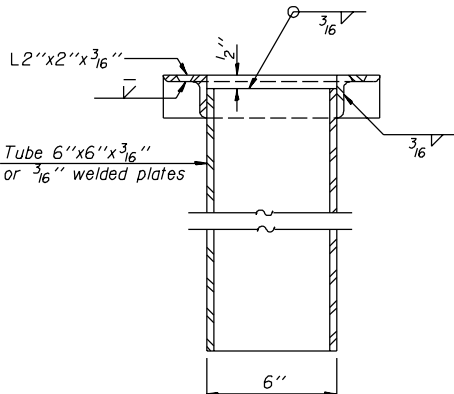
VIEW C-C



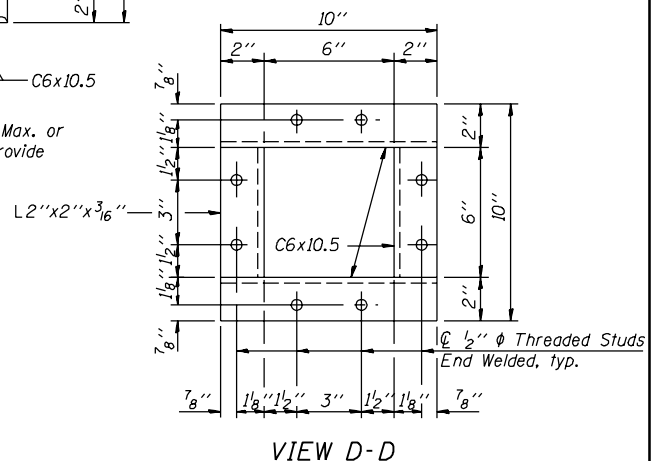
SECTION E-E



DOWNSPOUT



SECTION G-G



VIEW D-D

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Drainage Scupper	Each	

(Sheet 1 of 2)  
STEEL DRAINAGE SCUPPER

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

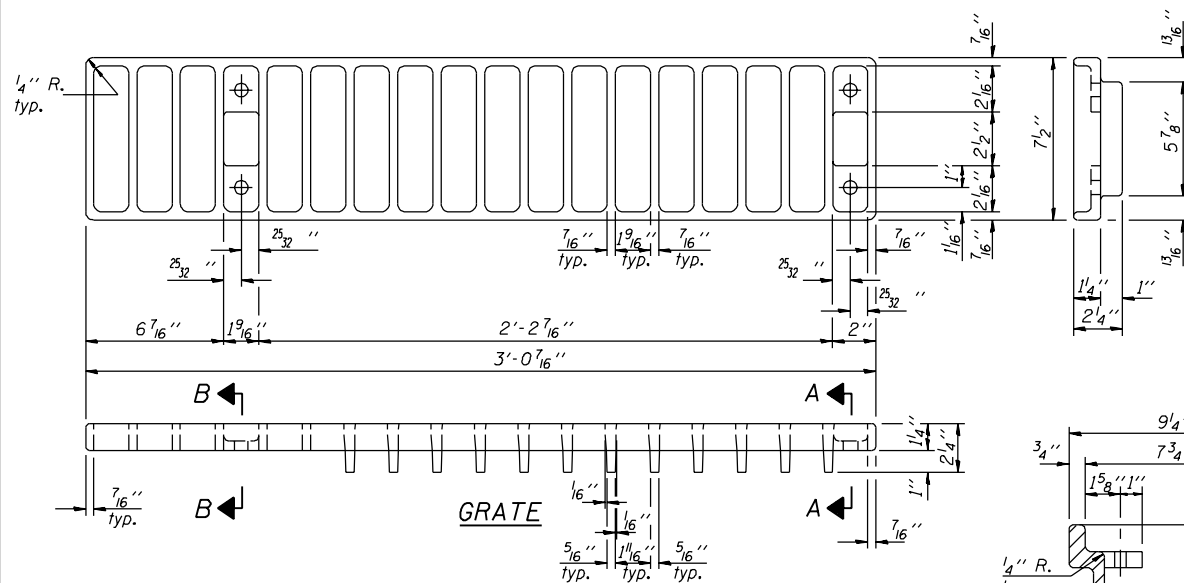
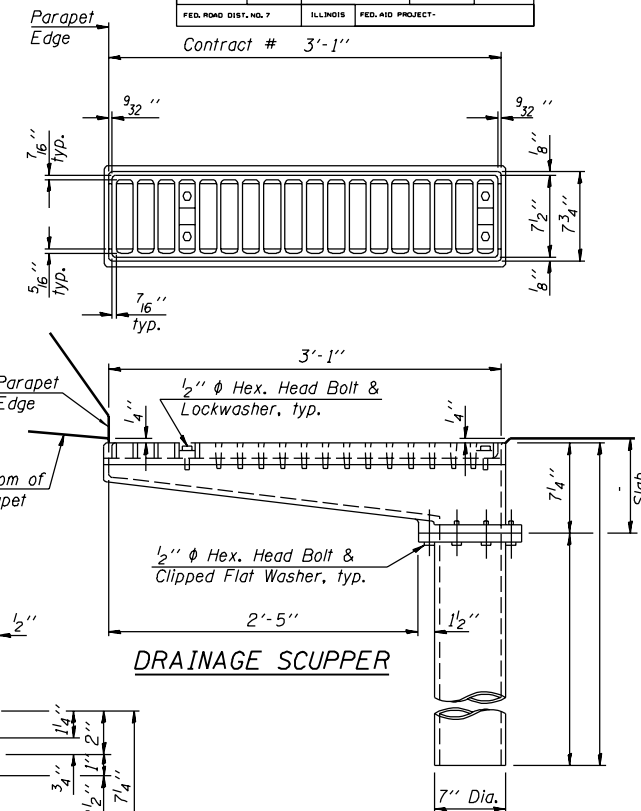
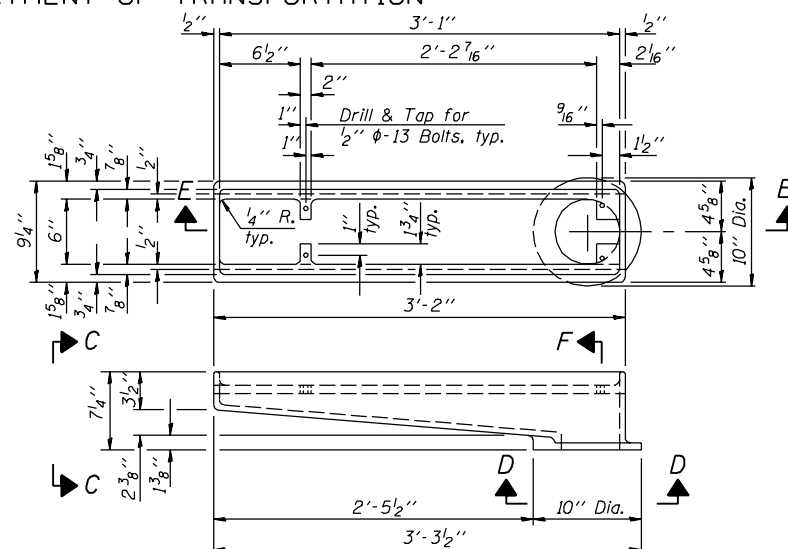
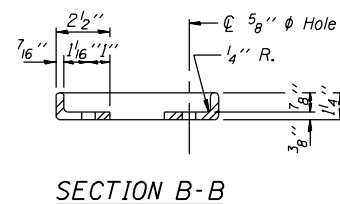
DS-3 10-22-04 (W.T. to inside of exterior stringer flange shall not be 3'-11")



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
"	"	"		
"	"	"		
FED. ROAD DIST., NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. "

" SHEETS



Parapet Edge

FRAME

3'-2" F

2'-7"

6"

1/2"

1/2"

1/4" R.. typ.

3 1/2"

3"

1 1/2"

3 1/2"

1" 2"

7 1/4"

1"

13 3/4"

3 1/4"

SECTION E-E

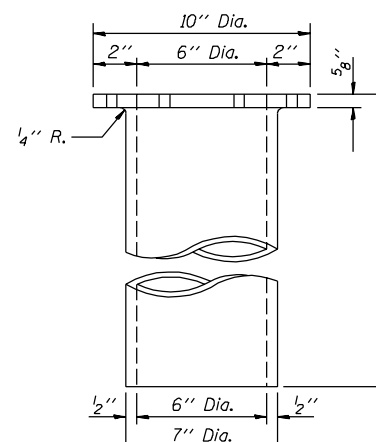
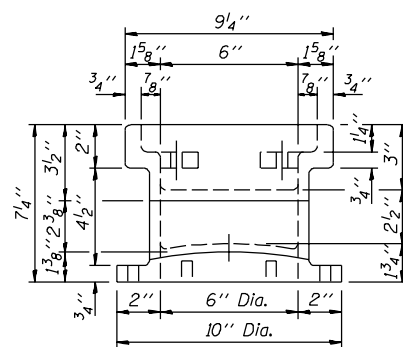
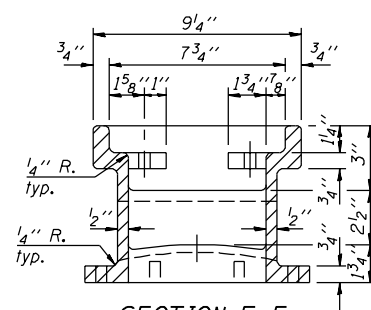
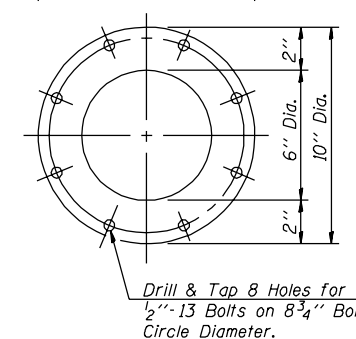
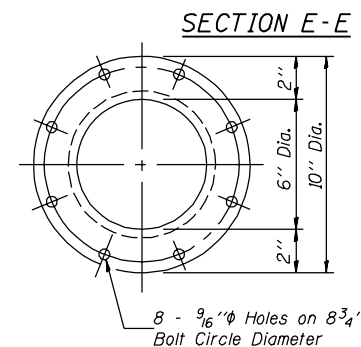
2-1 1/4" Deep Tapped Holes

2"

6" Dia.

2"

10" Dia.



(Sheet 2 of 2)

ALTERNATE - CAST IRON  
DRAINAGE SCUPPER

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

DS-4

10-22-04 (W.T. to inside of exterior stringer flange shall not be  $> 3'-11''$ )



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

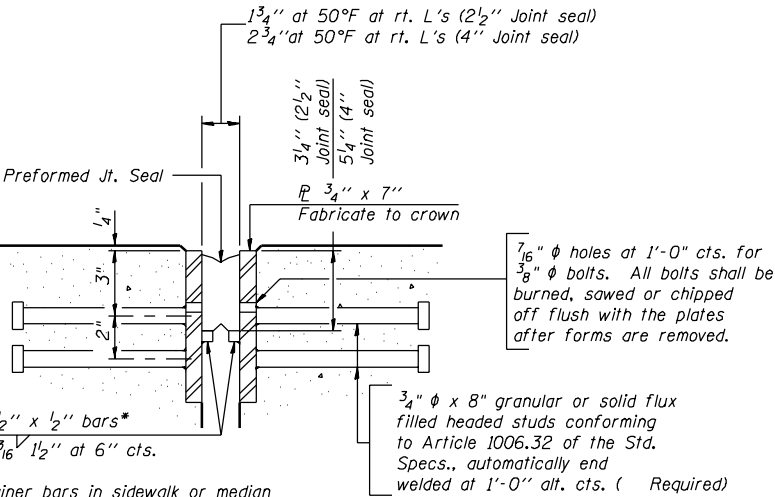
SHEET NO. -  
- SHEETS

Contract #

GENERAL NOTES

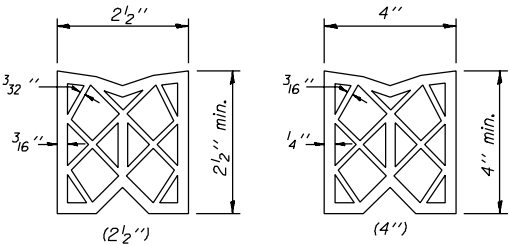
Furnish steel plates in segments of 20 feet maximum length. Maximum space between installed segments shall be  $\frac{3}{16}$ ". Seal space with silicone sealant suitable for structural steel.

Bridge Joint System (Expansion)		
Design Movement	Required Preformed Joint Seal Size	Required Strip Seal Rated movement
1"	2 $\frac{1}{2}$ "	1"
1 $\frac{5}{8}$ "	4"	2"

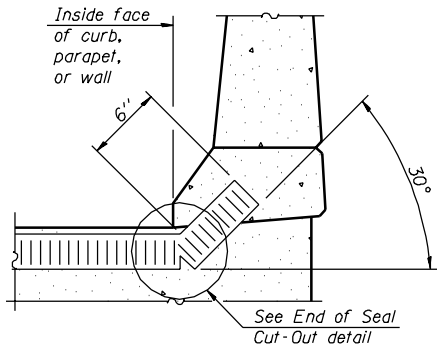


\*Cut retainer bars in sidewalk or median 6" short of the sidewalk or median face.

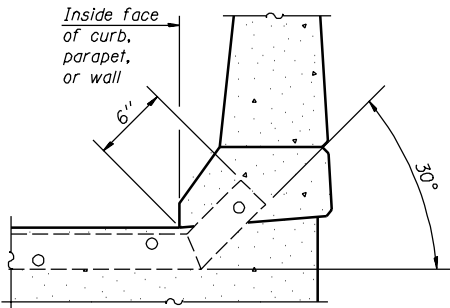
SECTION THRU EXPANSION JOINT  
(2 $\frac{1}{2}$ " and 4" joint seals)



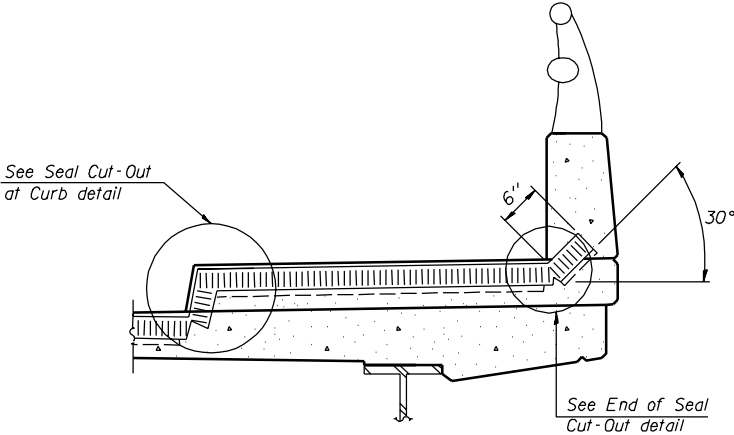
PREFORMED JOINT SEAL



AT CURB, PARAPET, OR WALL  
(Showing seal)



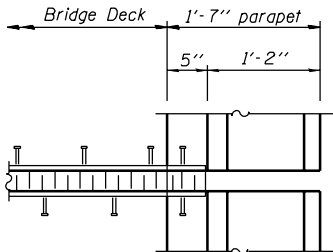
AT CURB, PARAPET, OR WALL  
(Showing plate)



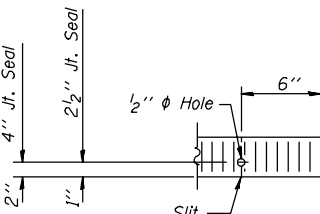
AT SIDEWALK OR MEDIAN\*  
(Showing plate and seal)

\* Shorter plates with a single row of studs at 12" centers may be necessary on medians which are shallower than 9". See manufacturer's recommendation.

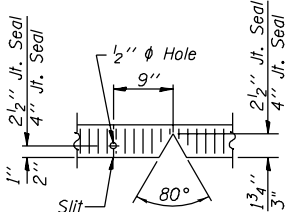
TYPICAL END TREATMENTS



PLAN AT PARAPET



END OF SEAL CUT-OUT



SEAL CUT-OUT AT CURB

BILL OF MATERIAL

Item	Unit	Total
Bridge Joint System (Expansion)	foot	

(Sheet 1 of 2)

BRIDGE JOINT SYSTEM - EXPANSION  
(PREFORMED JOINT SEAL)

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	ENGINEER OF BRIDGE DESIGN	
CHECKED -	PASSED	
	ENGINEER OF BRIDGES AND STRUCTURES	

EJ-BJS

10-22-04



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT-		

SHEET NO. -  
- SHEETS

Contract #

GENERAL NOTES

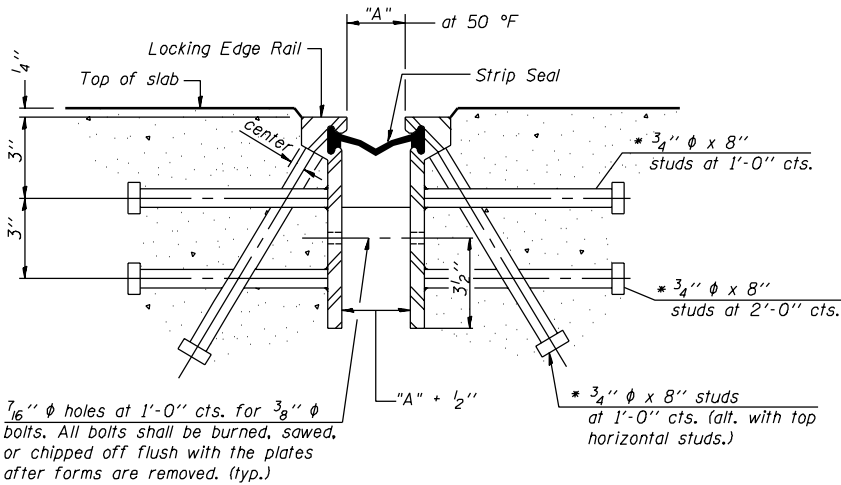
The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the Locking Edge Rails.

The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed.

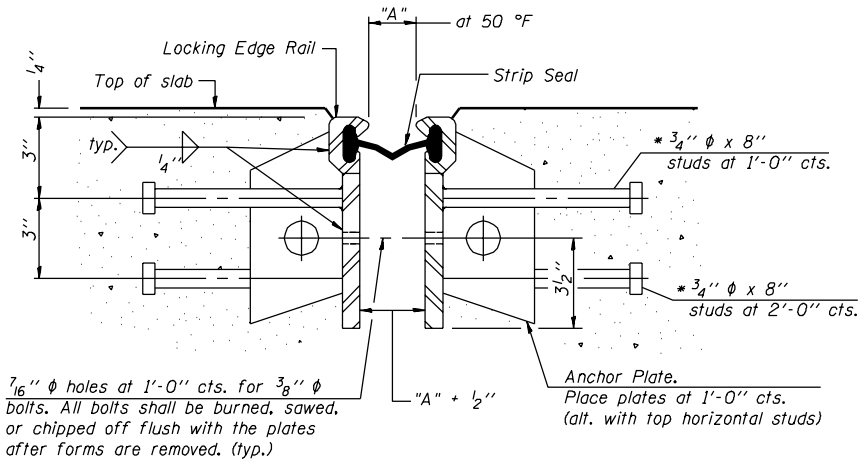
Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed.

The joint opening and deck dimensions detailed on the superstructure are based on a preformed joint seal. If the contractor elects to use the alternate strip seal joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.

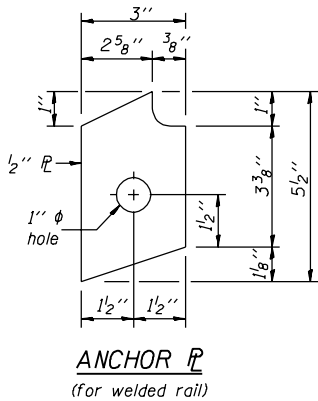
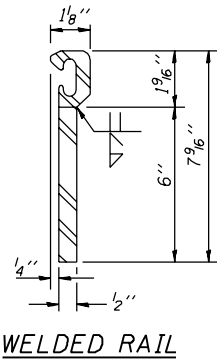
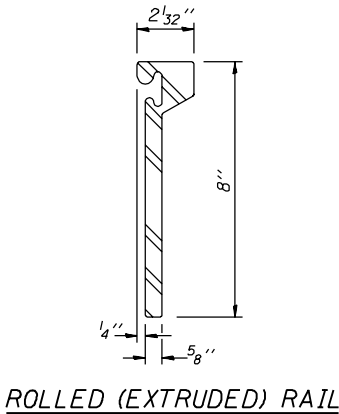


SECTION THRU ROLLED RAIL EXP. JOINT  
( Studs Required)

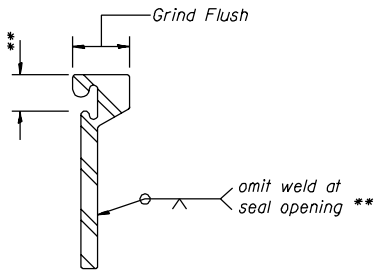


SECTION THRU WELDED RAIL EXP. JOINT  
( Studs Required)  
( Anchor Plates Required)

\* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

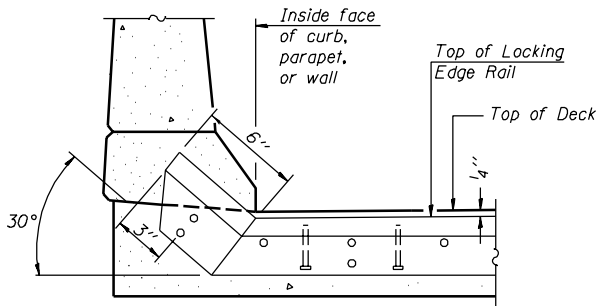


LOCKING EDGE RAILS

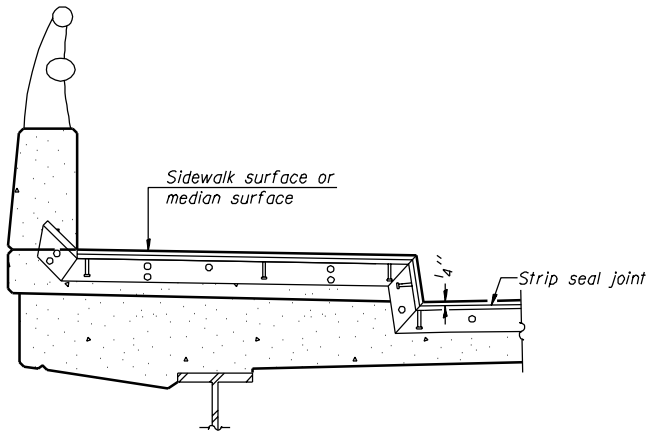


LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue.



AT CURB, PARAPET, OR WALL



AT SIDEWALK OR MEDIAN\*

\* Shorter plates with a single row of studs at 12" centers may be necessary on medians which are shallower than 9". See manufacturer's recommendation.

TYPICAL END TREATMENTS

DESIGNED -	200
CHECKED -	EXAMINED
DRAWN -	ENGINEER OF BRIDGE DESIGN
CHECKED -	PASSED
	ENGINEER OF BRIDGES AND STRUCTURES

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(Sheet 2 of 2)  
BRIDGE JOINT SYSTEM - EXPANSION  
(ALTERNATE-STRIP SEAL)



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	DATE	SHEET NO.
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

Contract #

GENERAL NOTES

Continuous Seal Neoprene Expansion Joint shall consist of molded anchor blocks of elastomer and steel, field assembled over continuous lengths of elastomeric membrane.

The elastomeric membrane shall be premolded with a single or a double upward convolution that will have a "memory" to return to its molded position upon joint closure.

The convolution length shall be such that the extended length will not be greater than the manufactured length when the joint is fully expanded in its design range and will not protrude above the anchor blocks when the joint is fully compressed.

Joint openings shall be adjusted according to Article 503.10(c) of the Standard Specifications when the deck is poured at an ambient temperature other than 50° F.

The parapet and roadway membrane shall be made continuous by an approved vulcanizing process. Lapping will not be permitted.

Joint Size	"C" at 50°F	"D" at 50°F
2"	2"	1 1/2" Min.
2 1/2"	2 1/2"	1 3/4" Min.
4"	3"	2 1/2" Min.

INSTALLATION NOTES

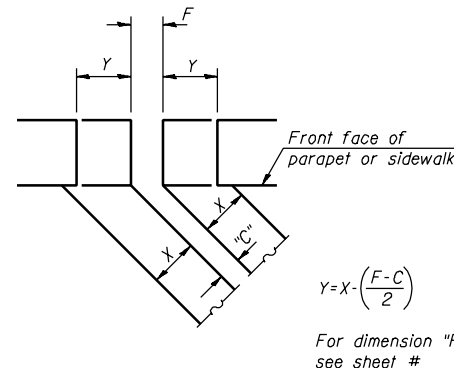
- Install continuous seal in roadway, parapet, curb, and sidewalk.
- Install anchor blocks as indicated.

Note A:  
Maximum spacing of anchor bolts shall be 12" centers.

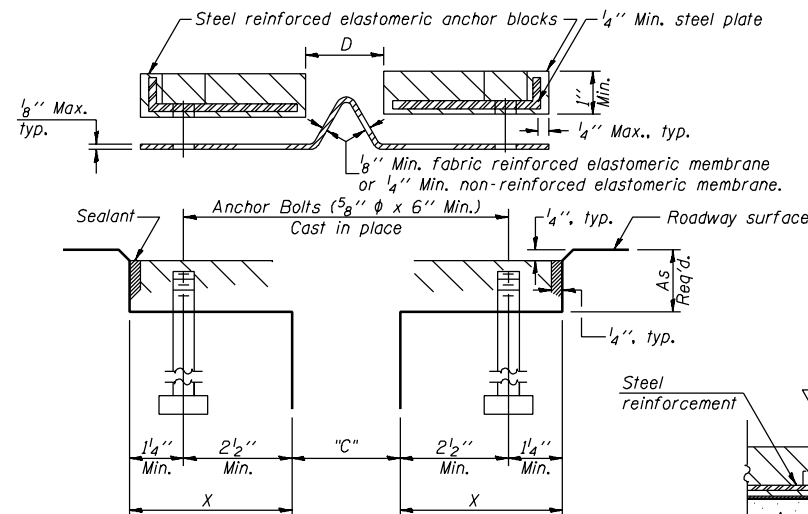
SKEW LIMITATIONS

The details of the anchor blocks and the elastomeric membrane in the parapet, as shown, are for up to 50° skews.

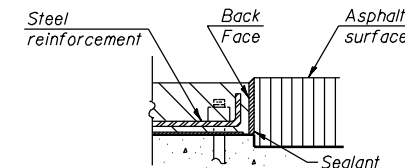
For skews greater than 50°, the anchor blocks and the elastomeric membrane, installed according to dimension "D", might require modifications to insure a minimum clearance of 1/2" from centerline of anchor studs to edge of parapet opening. The anchor blocks and the elastomeric membrane shall also be installed to the top of the parapet with the anchor studs spaced at ±12" cts.



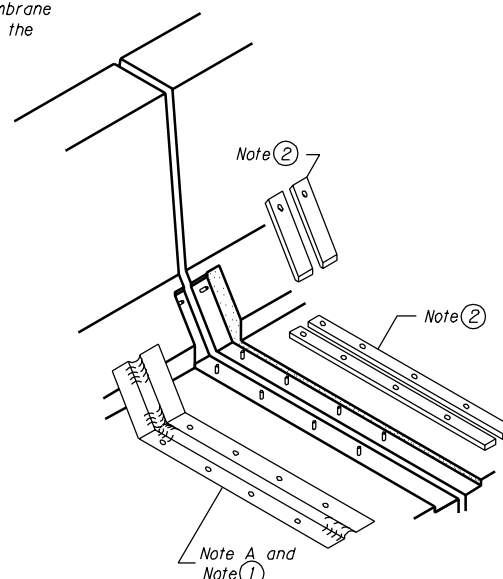
FORMING BLOCKOUT  
SKETCH



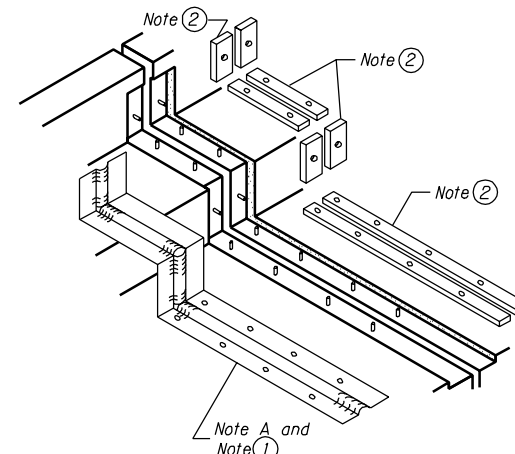
CROSS SECTION



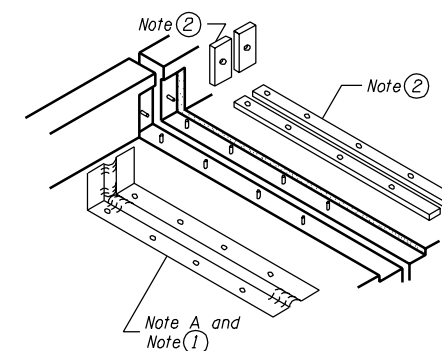
ANCHOR BLOCK  
WITH ASPHALT SURFACE



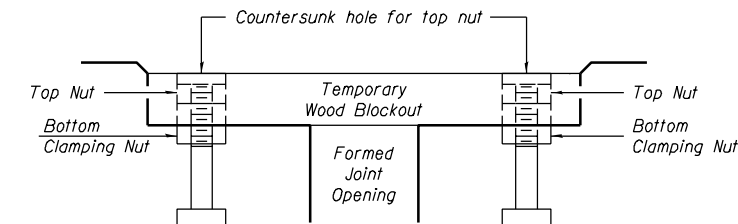
AT PARAPET



AT SIDEWALK OR MEDIAN

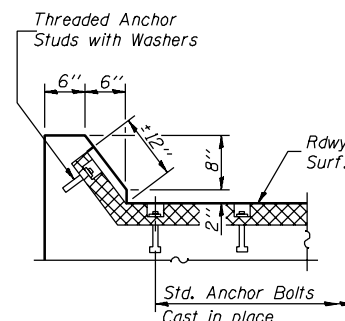


AT WALL

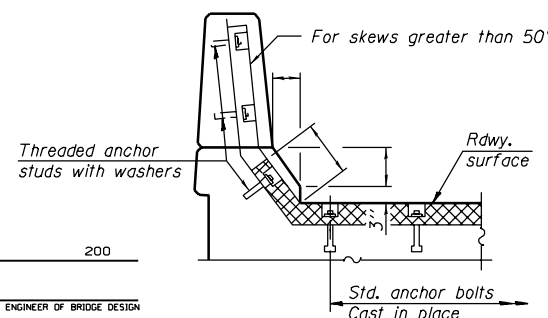


Note:  
Stud needs to be threaded lower to allow for use of clamping nut.

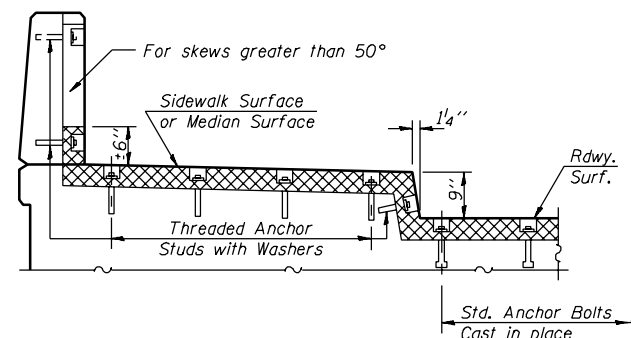
Anchor studs should be stainless  
RECOMMENDED BLOCKOUT DETAIL



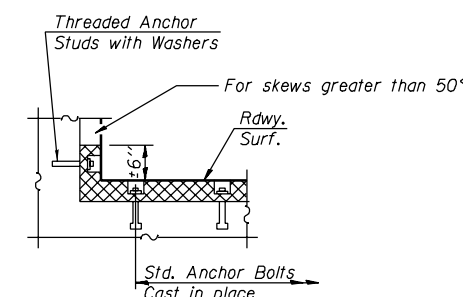
AT CURB



AT PARAPET



AT SIDEWALK OR MEDIAN  
TYPICAL END TREATMENTS

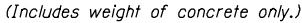


AT WALL

CONTINUOUS SEAL TYPE  
NEOPRENE EXPANSION JOINTS

DESIGNED -	200
CHECKED -	EXAMINED
DRAWN -	PASSED
CHECKED -	ENGINEER OF BRIDGE DESIGN
	ENGINEER OF BRIDGES AND STRUCTURES





The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below.

SHEET NO. -  
- SHEETS

Contract #

To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown below, minus slab thickness, equals the fillet heights "t" above top flange of beams.

### FILLET HEIGHTS

<i>Location</i>	<i>Station</i>	<i>Offset</i>	<i>Theoretical Grade Elevations</i>	<i>Theoretical Grade Elevations Adjusted For Dead Load Deflection</i>

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

200

---

EXAMINED

---

PASSED

---

ENGINEER OF BRIDGE DESIGN

---

ENGINEER OF BRIDGES AND STRUCTURES



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
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SHEET NO. -  
- SHEETS

Contract #

GENERAL NOTES

Furnish PJS steel plates in segments of 20 feet maximum length. Maximum space between installed segments shall be  $\frac{3}{16}$ ". Seal space with silicone sealant suitable for structural steel.

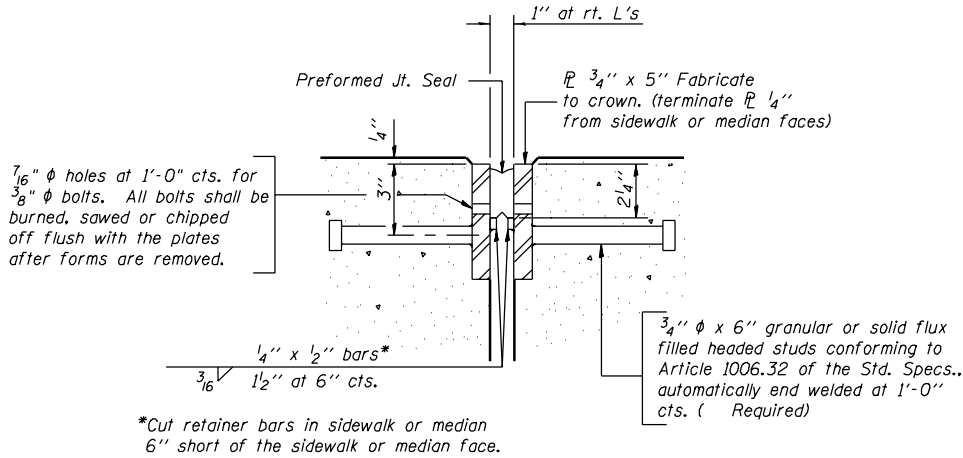
The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the Locking Edge Rails.

The height and thickness of the Locking Edge Rails shown are minimum dimensions. The actual configuration of the Locking Edge Rails and matching strip seal may vary from manufacturer to manufacturer. Flanged edge rails will not be allowed.

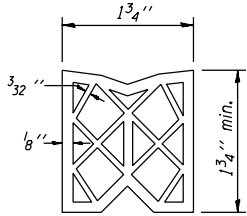
Locking Edge Rails may be spliced at slope discontinuities and stage construction joints.

The manufacturer's recommended installation methods shall be followed.

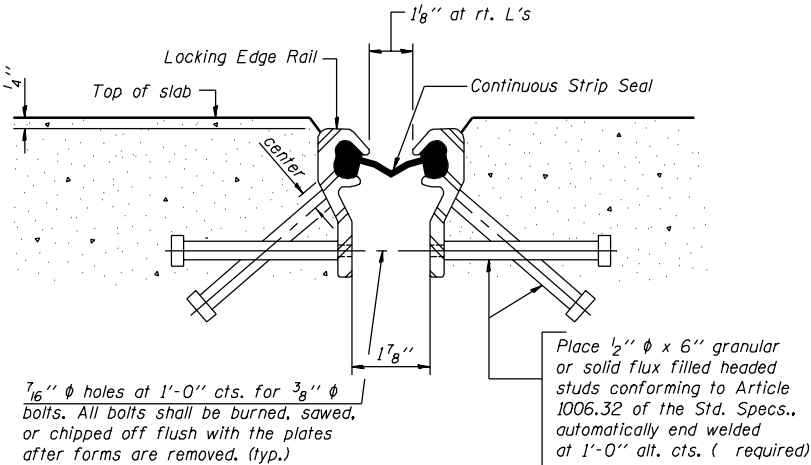
The joint opening and deck dimensions detailed on the superstructure are based on a preformed joint seal. If the contractor elects to use the alternate strip seal joint, the opening and deck dimensions shall be modified according to the dimensions detailed on this sheet. Required modifications shall be made at no additional cost to the State.



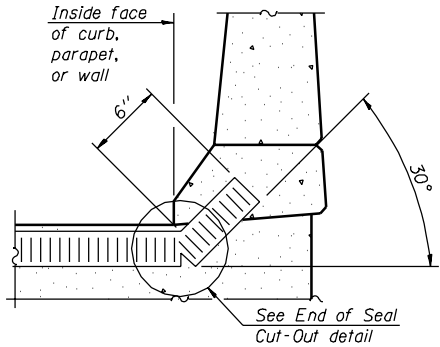
SECTION THRU FIXED PREFORMED JOINT SEAL



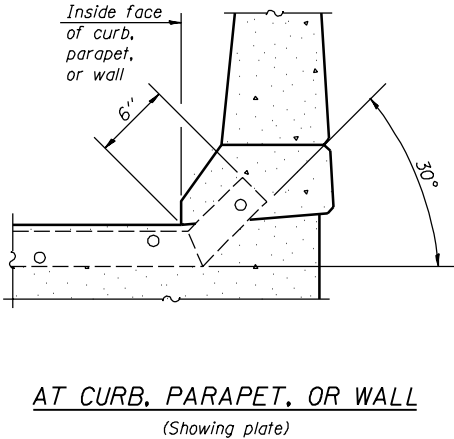
PREFORMED JOINT SEAL



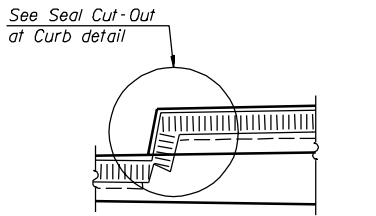
SECTION THRU FIXED STRIP SEAL JOINT



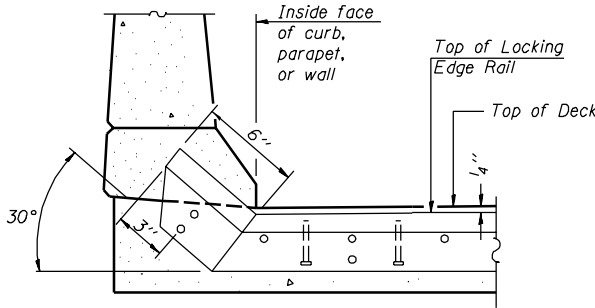
AT CURB, PARAPET, OR WALL  
(Showing seal)



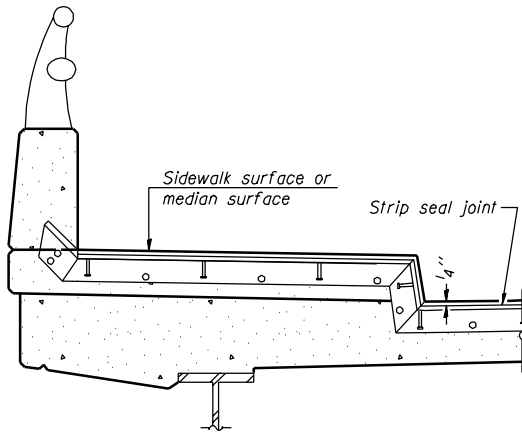
AT CURB, PARAPET, OR WALL  
(Showing plate)



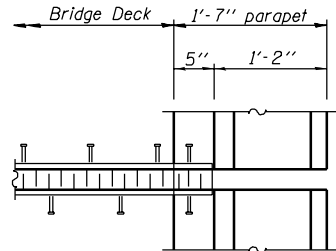
AT SIDEWALK OR MEDIAN



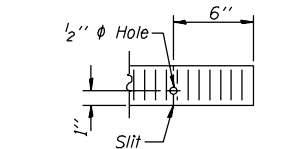
AT CURB, PARAPET, OR WALL



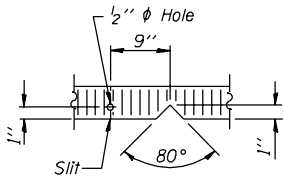
AT SIDEWALK OR MEDIAN



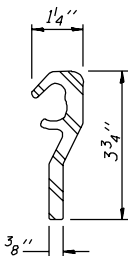
PLAN AT PARAPET



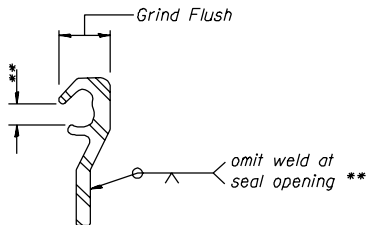
END OF SEAL CUT-OUT



SEAL CUT-OUT AT CURB



LOCKING EDGE RAIL



LOCKING EDGE RAIL SPLICE

The inside of the Locking Edge Rail groove shall be free of weld residue.

BILL OF MATERIAL

Item	Unit	Total
Bridge Joint System (Fixed)	foot	

BRIDGE JOINT SYSTEM - FIXED

DESIGNED -	200
CHECKED -	EXAMINED
DRAWN -	PASSED
CHECKED -	ENGINEER OF BRIDGES AND STRUCTURES

FJ-BJS

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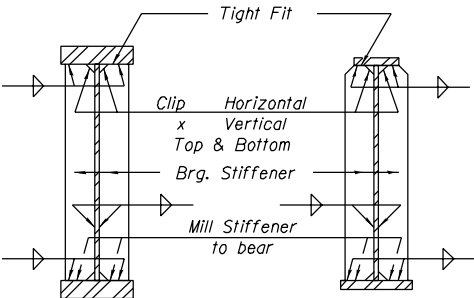


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

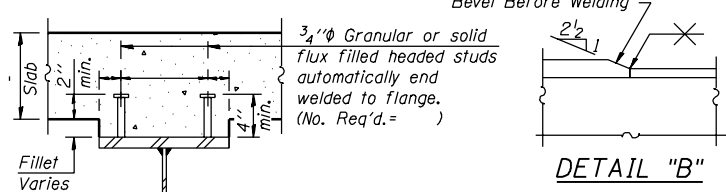
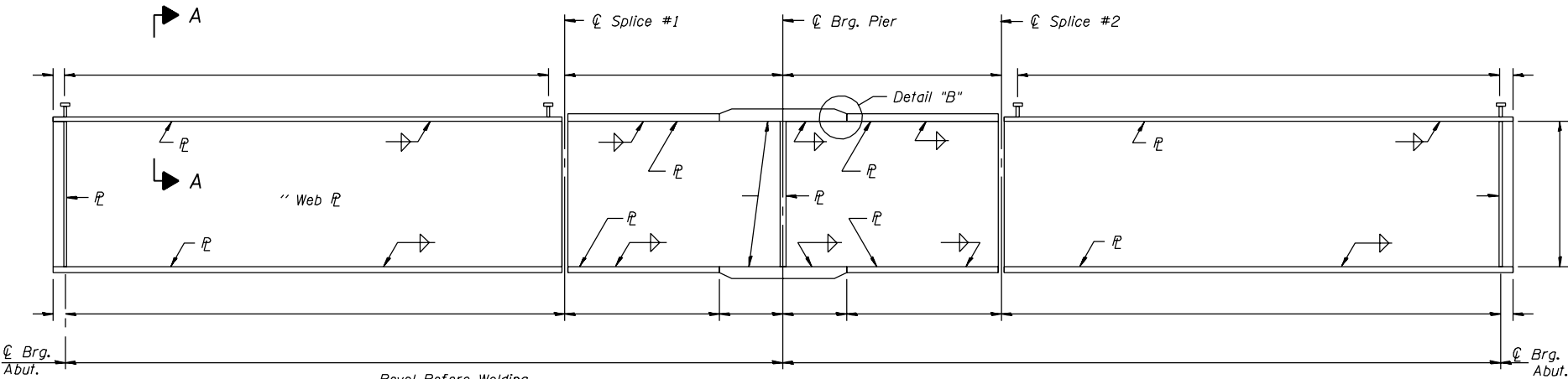
SHEET NO. -  
- SHEETS

Contract #

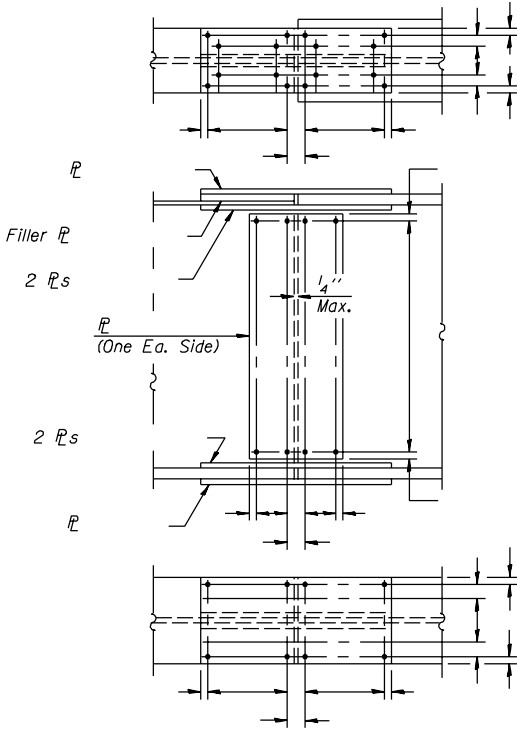


SECTION  
AT PIER

SECTION  
AT ABUTMENT



GIRDER ELEVATION  
"NTR" denotes plates to which notch toughness requirements are applicable.



FIELD SPLICE DETAIL

SECTION A-A

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

200
EXAMINED
ENGINEER OF BRIDGE DESIGN
PASSED
ENGINEER OF BRIDGES AND STRUCTURES

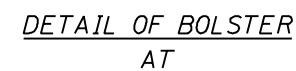
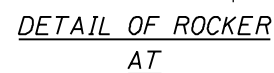


ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
"	"	"		
"	"	"		
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

Contract #



DETAIL OF  
PINTLE



DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

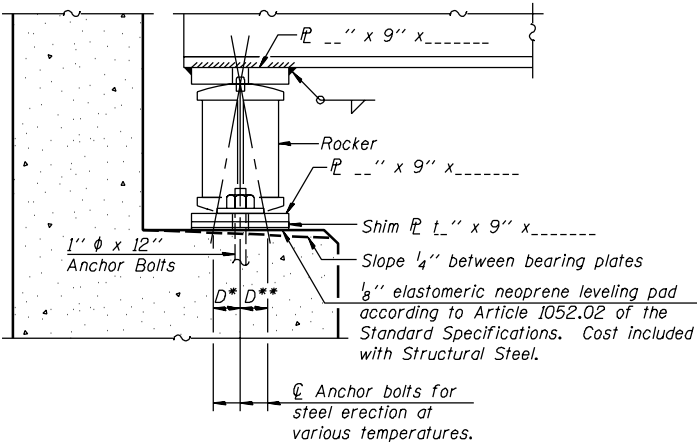


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

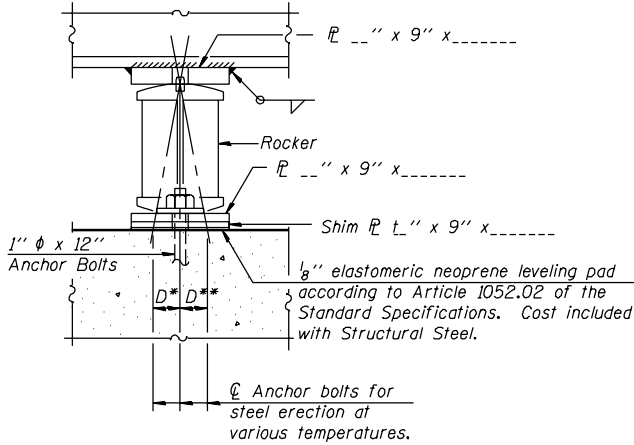
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -  
- SHEETS

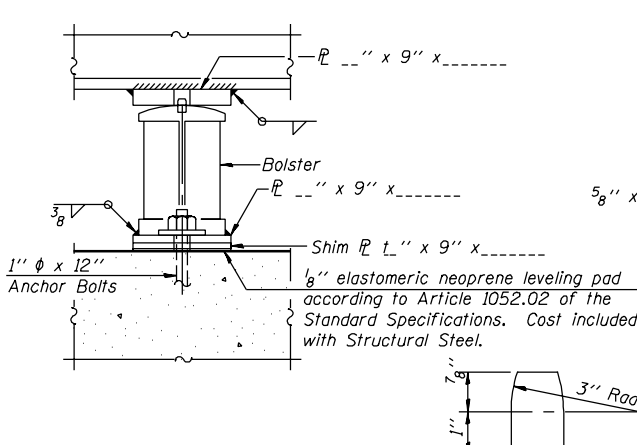
Contract #



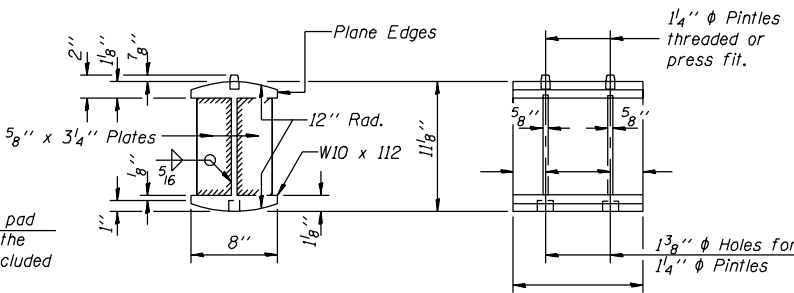
ELEVATION



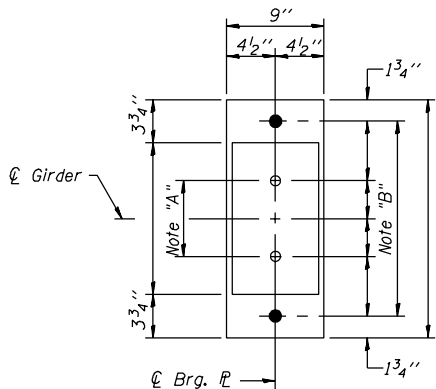
ELEVATION



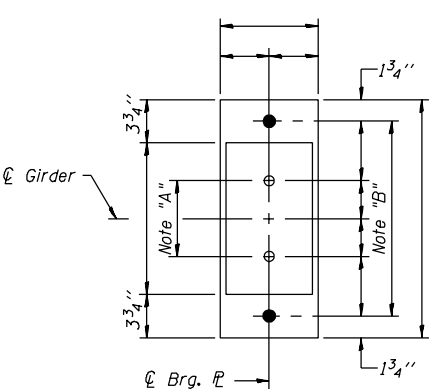
ELEVATION



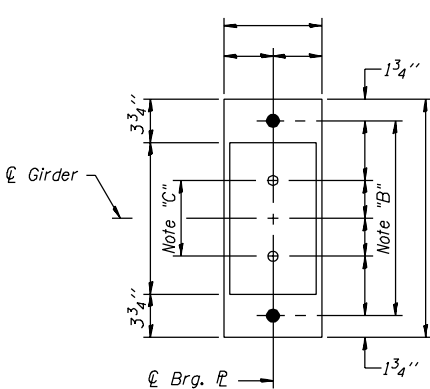
ROCKER



PLAN  
AT ABUTMENT

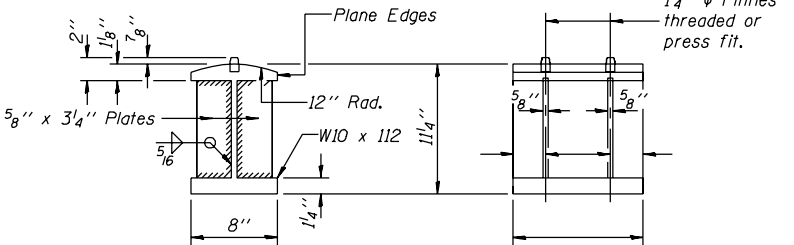


PLAN  
AT PIER



PLAN  
AT PIER

PINTLE



BOLSTER

Note "A"  
1 3/8"  $\phi$  Holes-1" deep in  
top  $\bar{R}$  for 1 1/4"  $\phi$  Pintles.  
Thread or press fit  
pintles in bottom  $\bar{R}$ .

Note "B"  
1 1/2"  $\phi$  Holes for 1"  $\phi$  Anchor  
Bolts-5/16" x 2 1/2" x 2 1/2"  $\bar{R}$   
washer under nut.

Note "C"  
1 3/8"  $\phi$  Holes-1" deep in  
top  $\bar{R}$  only for 1 1/4"  $\phi$  Pintles.

BEARING ASSEMBLY DETAILS

NOTES FOR SETTING OF ANCHOR BOLTS  
AT EXPANSION BEARINGS

- a.)  $D^*$  (Side of brg. away from fixed brg.)  
 $D^* = \frac{1}{8}''$  per each 100' of expansion for  
every 15° fall below the normal temp.  
of 50° F.
- $D^{**}$  (Side of brg. toward fixed brg.)  
 $D^{**} = \frac{1}{8}''$  per each 100' of expansion for  
every 15° rise above the normal temp.  
of 50° F.

- b.) After girders have been erected and dimensions  
 $D^*$  &  $D^{**}$  determined, holes shall be drilled and  
anchor bolts shall be installed as shown on  
Sheet of . All fixed anchor bolts may be  
built into the masonry.

INTERIOR GIRDER MOMENT TABLE						
		0.4 Sp. _	Pier _	0.5 Sp. _	Pier _	0.6 Sp. _
$I_s$	(in 4)					
$I_c$	(in 4)					
$S_s$	(in 3)					
$S_c$	(in 3)					
$Z$	(in 3)					
$\bar{D}$	(K/ft.)					
$M\bar{D}$	('K)					
$s\bar{D}$	(K/ft.)					
$Ms\bar{D}$	('K)					
$M\bar{t}$	('K)					
$M$ (Imp)	('K)					
$S_3(M\bar{t} + I)$	('K)					
$Ma$	('K)					
$Mu$	('K)					
$fs\bar{D}$ non-comp(k.s.i.)						
$fs\bar{D}$ (comp) (k.s.i.)						
$fsS_3(\bar{t} + I)$	(k.s.i.)					
$fs$ (Overload) (k.s.i.)						
$fs$ (Total) (k.s.i.)						
$VR$	(K)					

INTERIOR GIRDER REACTION TABLE				
	--. Abut.	Pier --	Pier --	--. Abut.
$R\bar{D}$	(K)			
$R\bar{t}$	(K)			
Imp.	(K)			
$R$ (Total)	(K)			

$I_s$  and  $S_s$  are the moment of inertia and section modulus  
of the steel section used in computing  $fs$  (Total & Overload).  
 $I_c$  and  $S_c$  are the moment of inertia and section modulus  
of the composite section used in computing  $fs$  (Total & Overload).  
 $VR$  is the maximum live Load + Impact shear range in span.  
 $Z$  is the plastic section modulus used to determine the Fully  
Plastic Moments in the non-composite areas.  
 $Ma$  (Applied Moment)= $1.3[M\bar{D} + Ms\bar{D} + S_3(M\bar{t} + I)]$ .  
 $Mu$  is the Full Plastic Moment Capacity for Compact, Braced  
section.  
 $fs$  (Overload) is the sum of the stresses due to  $M\bar{D} + Ms\bar{D} + S_3(M\bar{t} + I)$ .  
 $fs$  (Total) is the sum of the stresses due to  $1.3[M\bar{D} + Ms\bar{D} + S_3(M\bar{t} + I)]$ .

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	PASSED	ENGINEER OF BRIDGE DESIGN
CHECKED -		ENGINEER OF BRIDGES AND STRUCTURES



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-		
-				
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -  
- SHEETS

TOP OF BEAM ELEVATIONS

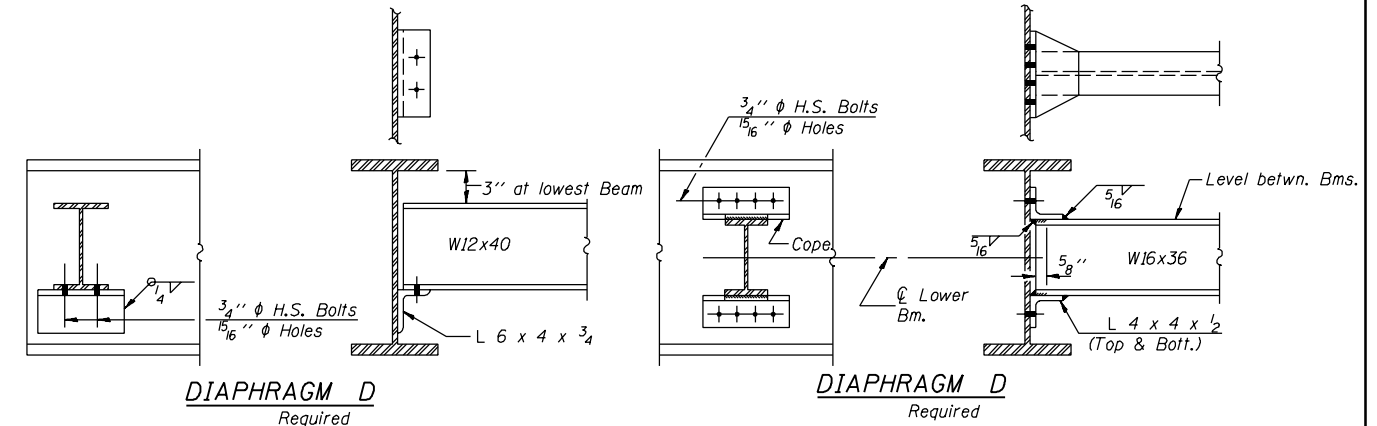
[illegible]

Diagram illustrating the dimensions and bolt spacing for a flange splice. The diagram shows a cross-section of a pipe with a flange splice. The flange thickness is labeled as  $7/8'' \phi$  R.S. Bolts. The bolt spacing is indicated as  $1/2''$  and  $3''$  cts. The flange splice is labeled as  $Flange Splice R$ . The maximum distance between bolts is labeled as  $4''$  Max.

SECTION A-A THROUGH ABUTMENT

$1\frac{1}{2}'' \phi$  Holes for  $1'' \phi \times 12''$  anchor bolts  $\frac{3}{16}'' \times 2\frac{1}{2}'' \times 2\frac{1}{2}''$   $\phi$  washers under nut.

$1\frac{3}{8}'' \phi$  Holes-1" deep in top  $\phi$  for pintles. Thread or press fit pintles into bottom  $\phi$ .

\*  $D = \frac{1}{8}''/100$  ft. of exp. for every  $15^\circ$  below the normal temp. of  $50^\circ F$ .

\*\*  $D = \frac{1}{8}''/100$  ft. of exp. for every  $15^\circ$  above the normal temp. of  $50^\circ F$ .

DETAIL OF PINTLE

PLAN

PLAN

DETAIL OF SPLICE

PLAN

DETAIL OF ROCKER  
AT

DETAIL OF BOLSTER  
AT

DESIGNED -	- 200
CHECKED -	EXAMINED
DRAWN -	ENGINEER OF BRIDGE DESIGN
CHECKED -	PASSED
	ENGINEER OF BRIDGES AND STRUCTURES

I-2-C

10-31-02



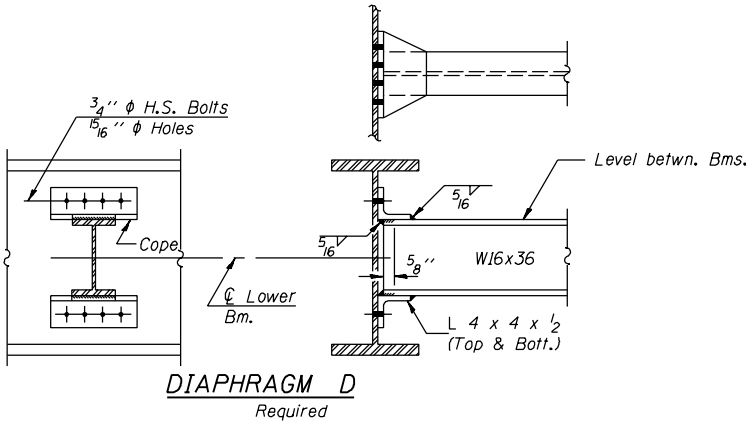
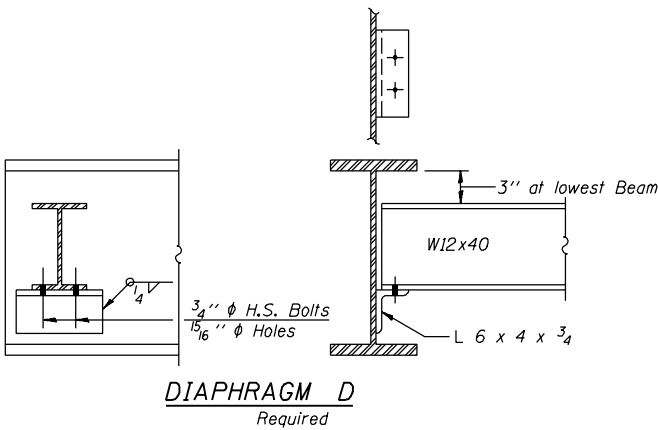
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT -	

SHEET NO. -

- SHEETS

Contract #



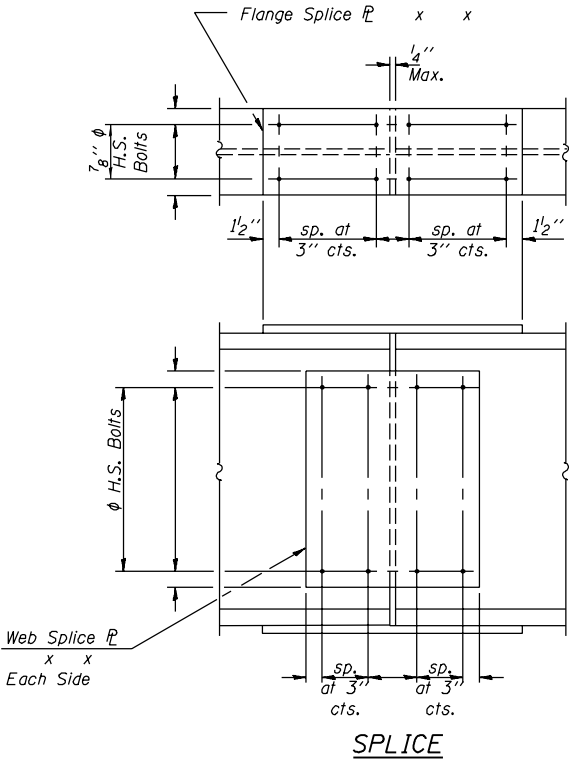
Note:  
Two hardened washers shall be  
required over all oversize holes for  
diaphragms.

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

EXAMINED	200
PASSED	ENGINEER OF BRIDGE DESIGN
	ENGINEER OF BRIDGES AND STRUCTURES

I-2-D

10-22-04



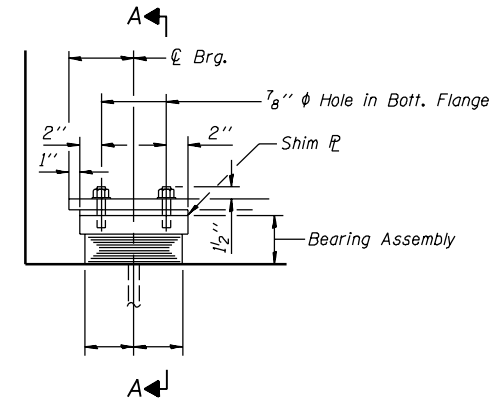


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

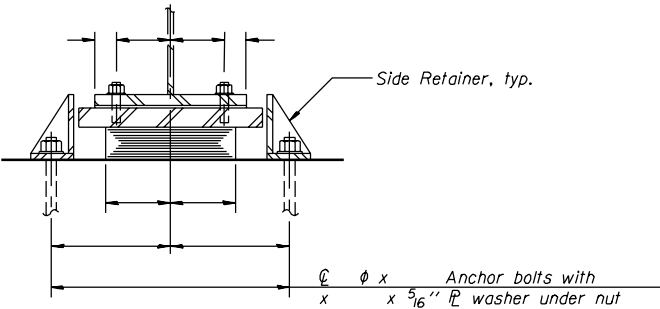
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

SHEET NO. -  
- SHEETS

Contract #



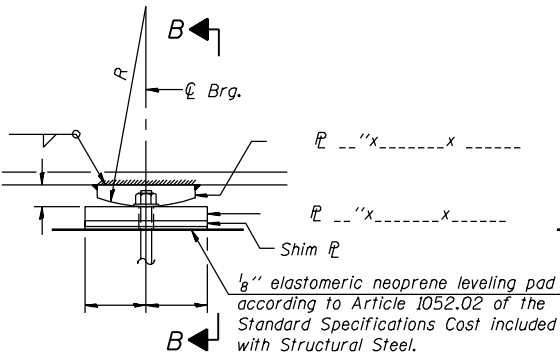
ELEVATION AT ABUT.



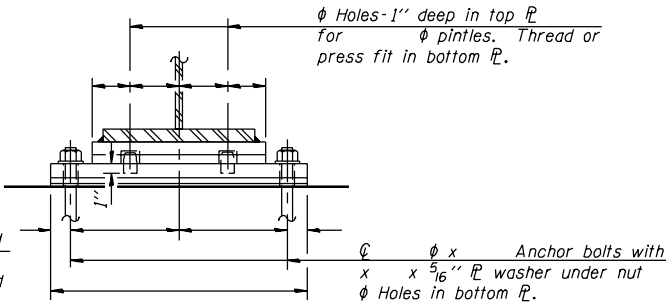
SECTION A-A

TYPE I ELASTOMERIC EXP. BRG.

Notes:  
Anchor bolts at fixed bearings may be  
built into the masonry.  
See sheet for Anchor Bolt installation.

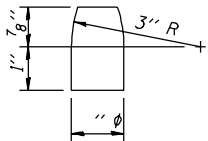


ELEVATION AT PIER

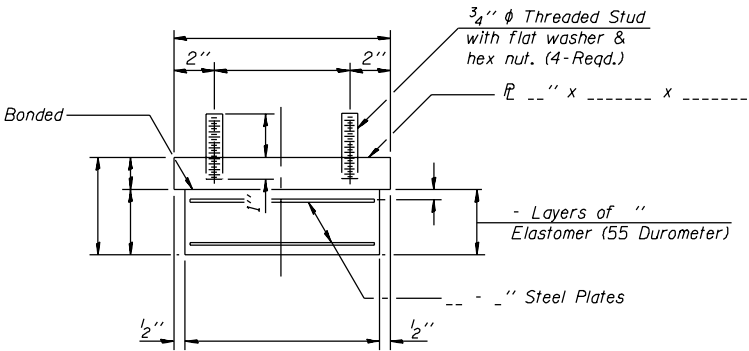


SECTION B-B

FIXED BEARING

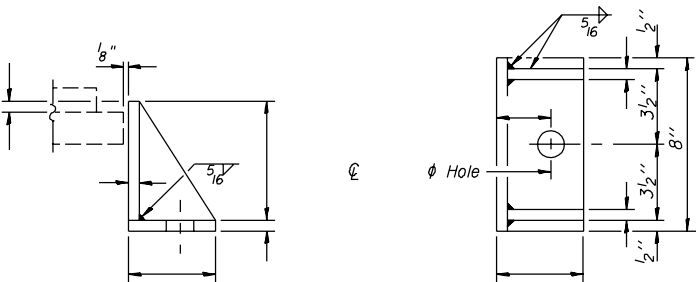


PINTLE



BEARING ASSEMBLY

Note:  
Shim plates shall not be placed  
under Bearing Assembly.



SIDE RETAINER

Equivalent rolled angle with stiffeners  
will be allowed in lieu of welded plates.  
Weight included with Structural Steel.

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	ENGINEER OF BRIDGE DESIGN	
CHECKED -	PASSED	
	ENGINEER OF BRIDGES AND STRUCTURES	

I-2-E1

10-22-04

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type I	Each	

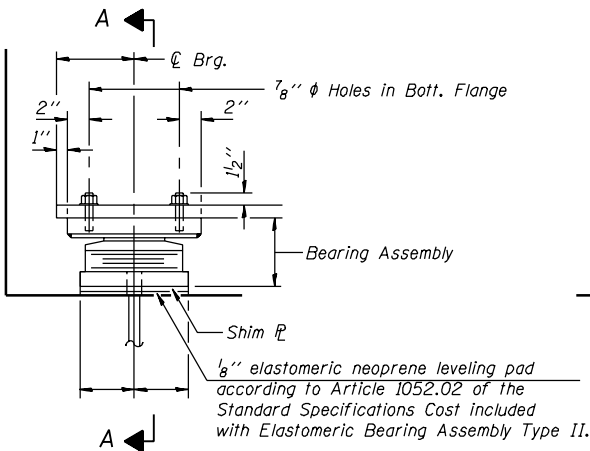


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

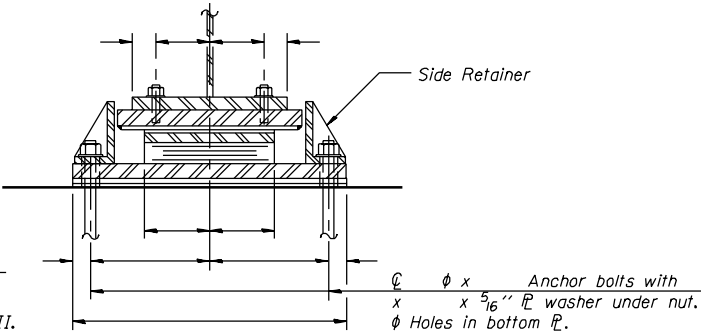
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT-		

SHEET NO. -  
- SHEETS

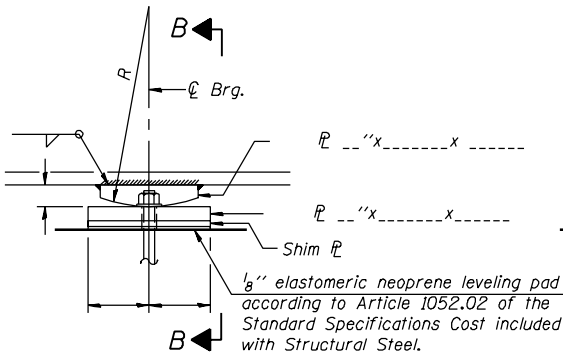
Contract #



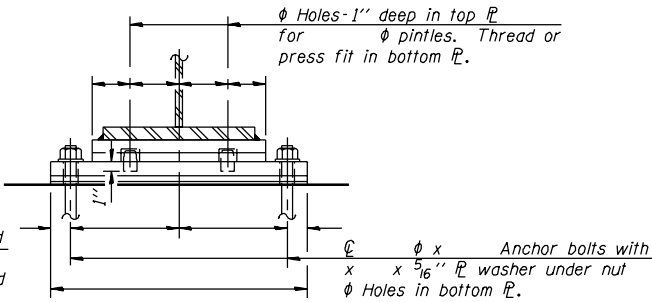
ELEVATION AT ABUT.



SECTION A-A



ELEVATION AT PIER

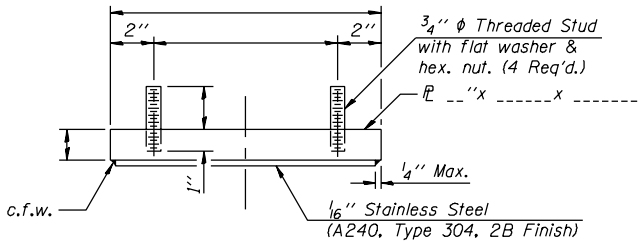


SECTION B-B

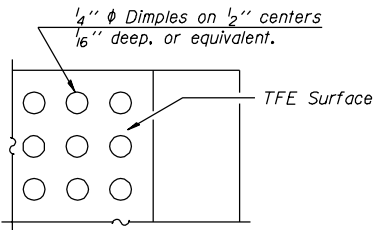
TYPE II ELASTOMERIC EXP. BRG.

Notes:  
Anchor bolts at fixed bearings may be built into the masonry.  
See sheet for Anchor Bolt installation.

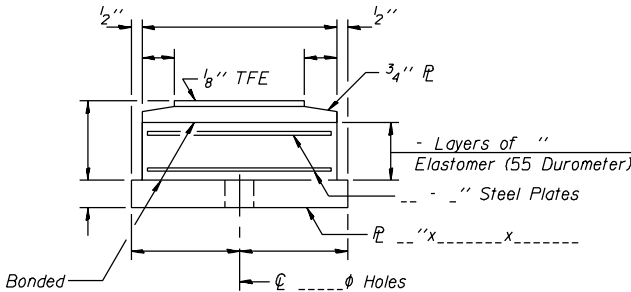
FIXED BEARING



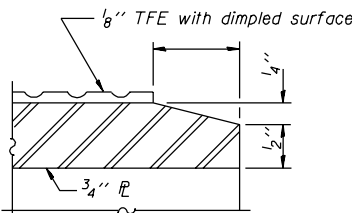
TOP BEARING ASSEMBLY



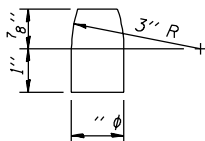
PLAN-TFE SURFACE



BOTTOM BEARING ASSEMBLY

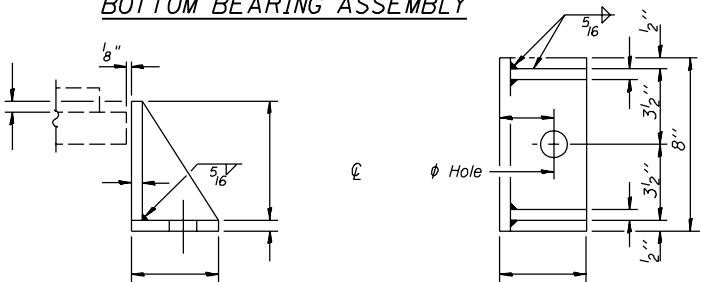


SECTION THRU TFE



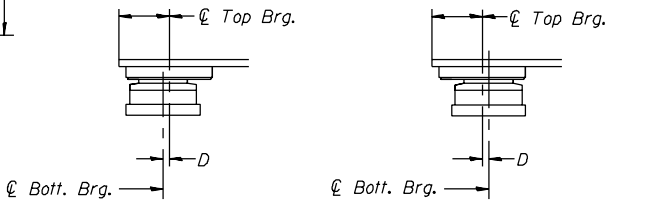
PINTLE

Notes:  
The 1/8" TFE sheet shall be bonded directly to the top steel plate with a two-component, medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MMM-A-134, Type I. The bond agent shall be applied on the full area of the contact surfaces.  
Bonding of 1/8" TFE sheet during vulcanizing process will be permitted provided the process and method of adjusting assembly height is approved by the Engineer.



SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates. Weight included with Structural Steel.



BELOW 50°F. ABOVE 50°F.  
(Move bott. brg. away from fixed brg.) (Move bott. brg. toward fixed brg.)

SETTING ANCHOR BOLTS AT EXP. BRG.

D=1/8" per each 100' of expansion for every 15° temp. change from the normal temp. of 50°F.

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type II	Each	

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	ENGINEER OF BRIDGE DESIGN	
CHECKED -	PASSED	
	ENGINEER OF BRIDGES AND STRUCTURES	

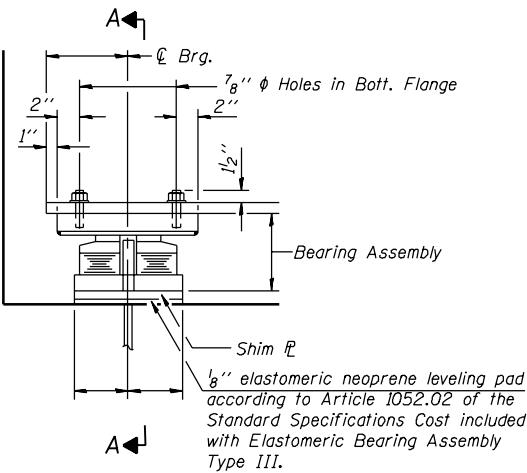


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT-		

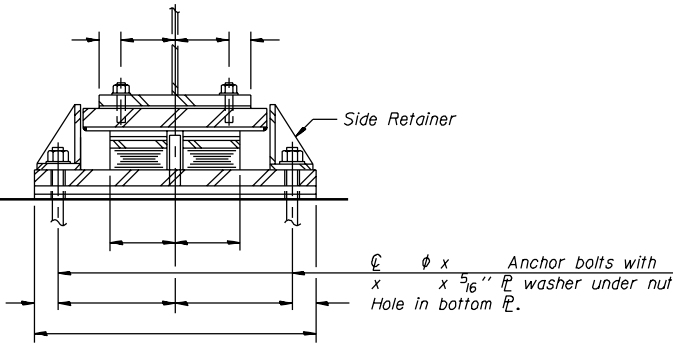
SHEET NO. -  
- SHEETS

Contract #

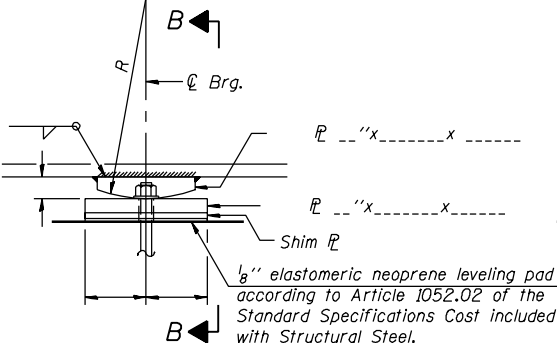


ELEVATION AT ABUT.

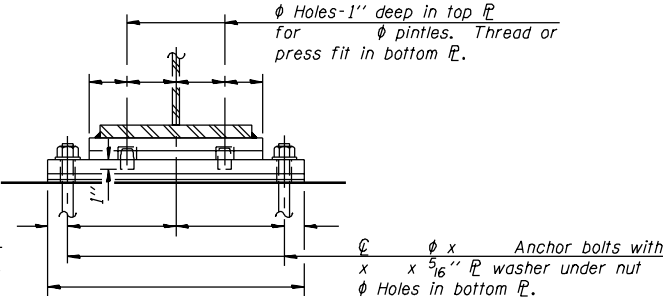
TYPE III ELASTOMERIC EXP. BRG.



SECTION A-A

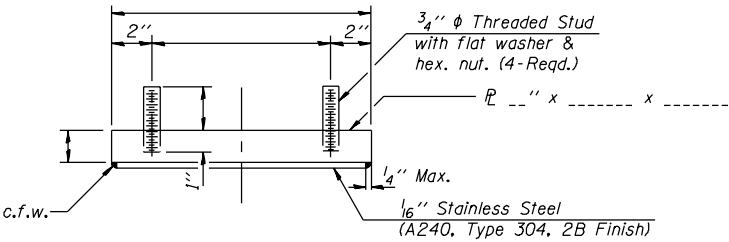


ELEVATION AT PIER

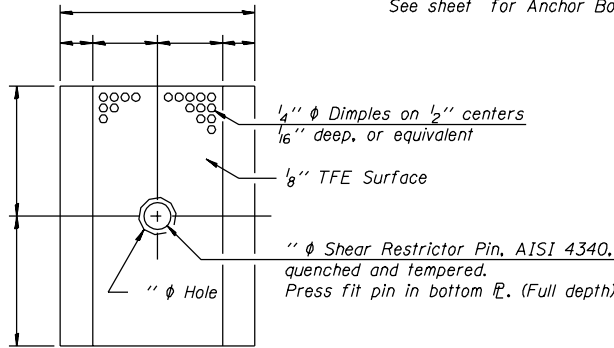


SECTION B-B

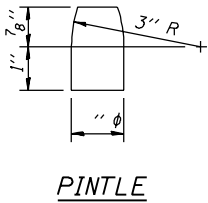
FIXED BEARING



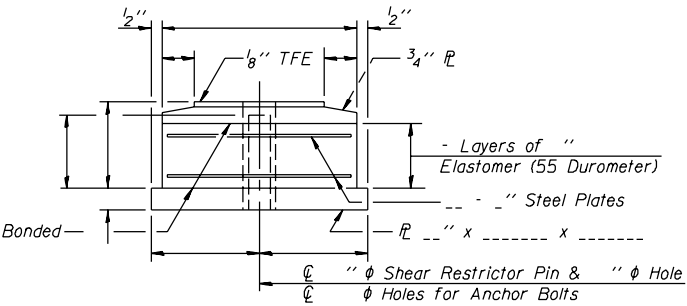
TOP BEARING ASSEMBLY



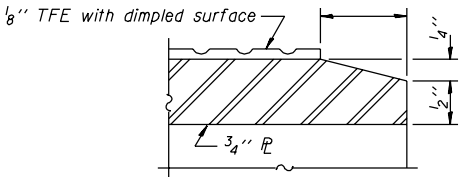
PLAN-TFE ELASTOMERIC BRG.



PINTLE

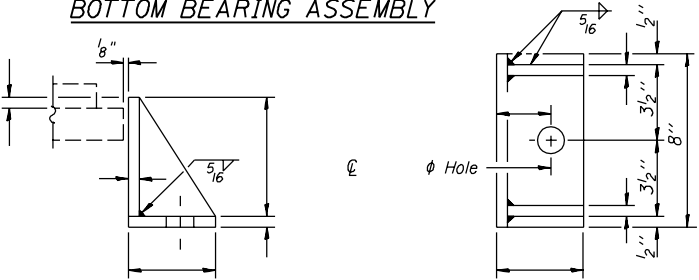


BOTTOM BEARING ASSEMBLY



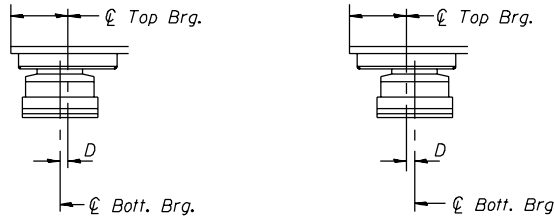
SECTION THRU TFE

Notes:  
The 1/8" TFE sheet shall be bonded directly to the top steel plate with a two-component, medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MMM-A-134, Type I. The bond agent shall be applied on the full area of the contact surfaces. Bonding of 1/8" TFE sheet during vulcanizing process will be permitted provided the process and method of adjusting assembly height is approved by the Engineer.



SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates. Weight included with Structural Steel.



BELOW 50° F.

ABOVE 50° F.

(Move bott. brg. away from fixed brg.) (Move bott. brg. toward fixed brg.)

SETTING ANCHOR BOLTS AT EXP. BRG.

D=1/8" per each 100' of expansion for every 15° temp. change from the normal temp. of 50° F.

BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type III	Each	

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	PASSED	
CHECKED -	ENGINEER OF BRIDGES AND STRUCTURES	

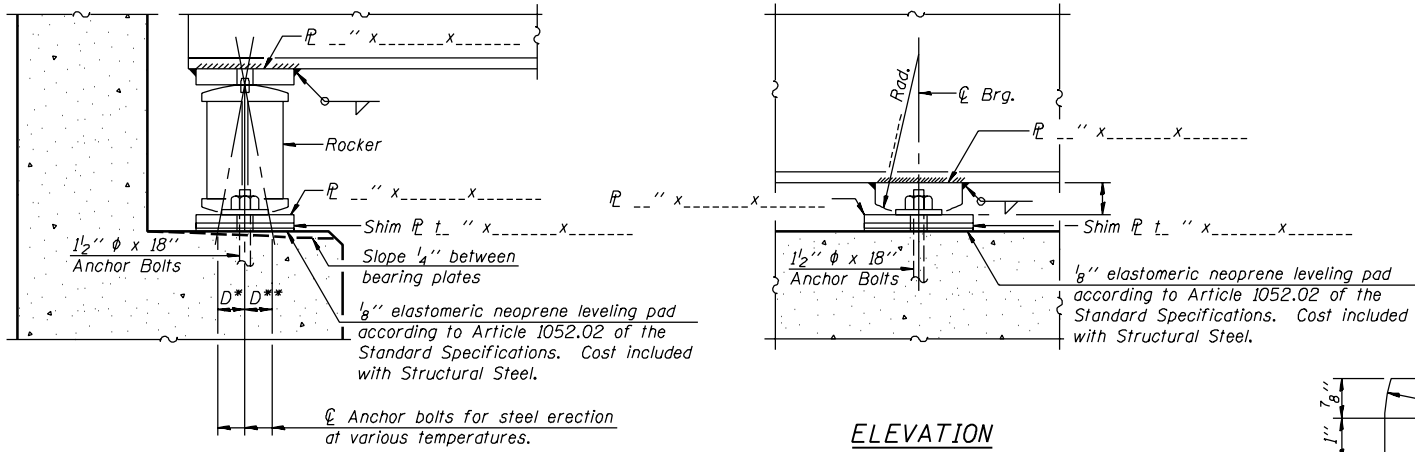


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

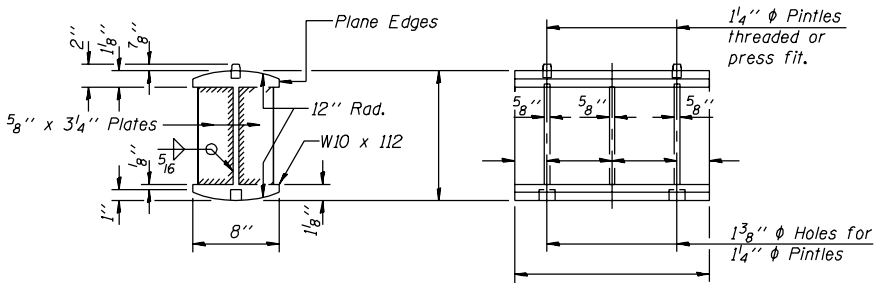
SHEET NO. -  
- SHEETS

Contract #

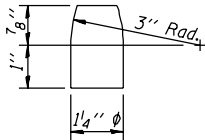


ELEVATION

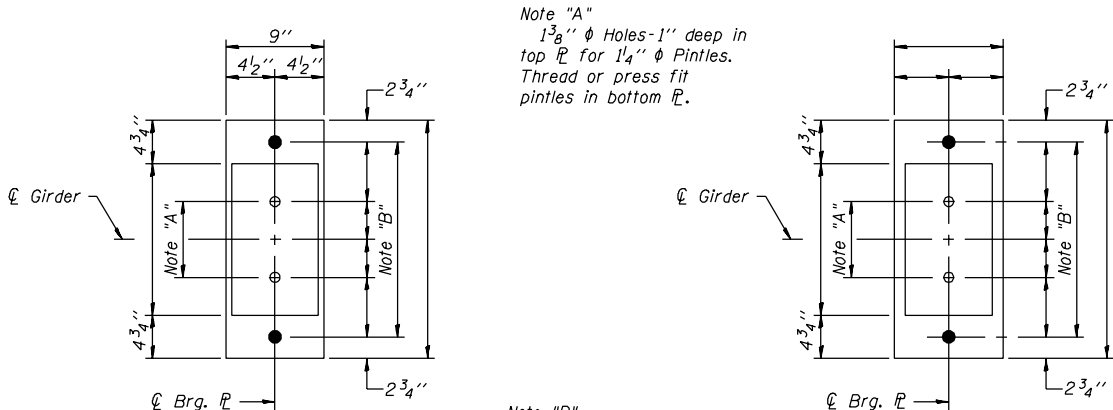
ELEVATION



ROCKER



PINTLE



PLAN  
AT ABUTMENT

PLAN  
AT PIER

NOTES FOR SETTING OF ANCHOR BOLTS  
AT EXPANSION BEARINGS

- a.)  $D^*$  (Side of brg. away from fixed brg.)  
 $D^* = \frac{1}{8}$  per each 100' of expansion for every 15° fall below the normal temp. of 50° F.
- $D^{**}$  (Side of brg. toward fixed brg.)  
 $D^{**} = \frac{1}{8}$  per each 100' of expansion for every 15° rise above the normal temp. of 50° F.

- b.) After girders have been erected and dimensions  $D^*$  &  $D^{**}$  determined, holes shall be drilled and anchor bolts shall be installed as shown on Sheet of . All fixed anchor bolts may be built into the masonry.

INTERIOR GIRDER MOMENT TABLE		
	0.4 Sp. 1	Pier
$I_s$	(in <sup>4</sup> )	
$I_c$	(in <sup>4</sup> )	
$S_s$	(in <sup>3</sup> )	
$S_c$	(in <sup>3</sup> )	
$Z$	(in <sup>3</sup> )	
$\bar{Q}$	(K/ft.)	
$M\bar{Q}$	('K)	
$s\bar{Q}$	(K/ft.)	
$Ms\bar{Q}$	('K)	
$M\bar{L}$	('K)	
$M$ (Imp)	('K)	
$S_3(M\bar{L} + I)$	('K)	
$Ma$	('K)	
$Mu$	('K)	
$fs\bar{Q}$ non-comp(k.s.i.)		
$fs\bar{Q}$ (comp) (k.s.i.)		
$fsS_3(\bar{L} + I)$ (k.s.i.)		
$fs$ (Overload) (k.s.i.)		
$fs$ (Total) (k.s.i.)		
VR	(K)	

INTERIOR GIRDER REACTION TABLE		
	Abut.	Pier
$R\bar{Q}$	(K)	
$R\bar{L}$	(K)	
Imp.	(K)	
$R$ (Total)	(K)	

$I_s$  and  $S_s$  are the moment of inertia and section modulus of the steel section used in computing  $fs$  (Total & Overload).

$I_c$  and  $S_c$  are the moment of inertia and section modulus of the composite section used in computing  $fs$  (Total & Overload).

VR is the maximum Live Load + Impact shear range in span.

$Z$  is the plastic section modulus used to determine the fully plastic moments in the non-composite areas.

$Ma$  (Applied Moment) =  $1.3[M\bar{Q} + Ms\bar{Q} + S_3(M\bar{L} + I)]$ .

$Mu$  is the Full Plastic Moment Capacity for Compact, Braced section.

$fs$  (Overload) is the sum of the stresses due to  $M\bar{Q} + Ms\bar{Q} + S_3(M\bar{L} + I)$ .

$fs$  (Total) (Non-compact section) is the sum of the stresses due to  $1.3[M\bar{Q} + Ms\bar{Q} + S_3(M\bar{L} + I)]$ .

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	PASSED	ENGINEER OF BRIDGE DESIGN
CHECKED -		ENGINEER OF BRIDGES AND STRUCTURES



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

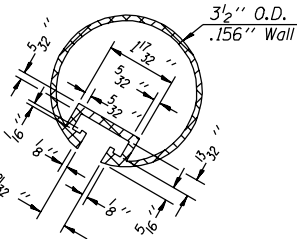
ROUTE NO.	SECTION	COUNTY	10% SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -  
- SHEETS

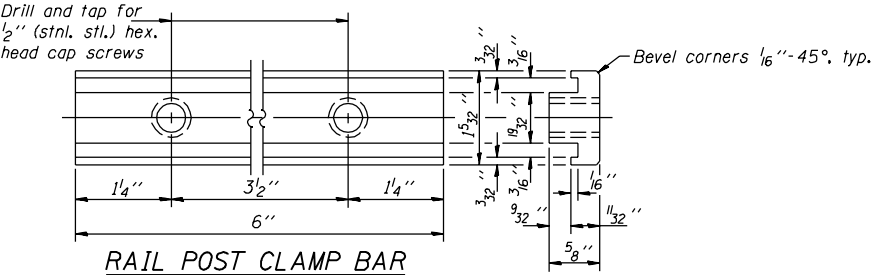
Contract #

Notes:

All Posts shall be normal to parapet.  
All Aluminum Alloy Extruded Rail shall be supplied in modular lengths of 30 feet, except at the end of bridge or over open joints in bridge deck where the rail shall be attached to a minimum of 2 posts. If the rail is on a horizontal curve of 2300 foot radius or less, the modular lengths may be reduced but shall be attached to a minimum of 2 posts.  
All joints in rail shall be spliced per detail.  
Provide 1- $\frac{1}{8}$ " and 2- $\frac{1}{16}$ " Aluminum Shims for 25% of the Posts.  
Rail elements shall be parallel to Grade-high spots will be ground and low spots shimmed.  
Railing shall be according to Section 509 of the Standard Specifications, except as noted, and will be paid for at the contract unit price per foot for ALUMINUM RAILING, TYPE L.  
Aluminum alloy rail shall conform to ASTM B 221 alloy 6061-T6 or 6351-T5 with min. yield 35 ksi, min. tensile 38 ksi, and elongation of 10% in 2 inches.

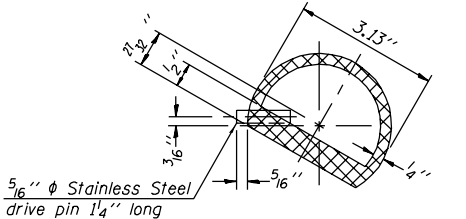


SECTION THRU TOP RAIL



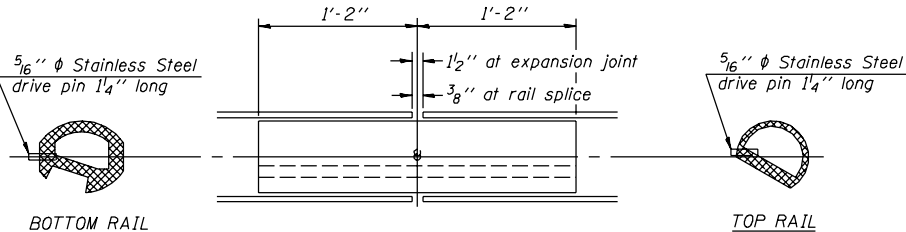
RAIL POST CLAMP BAR

For Top Rail

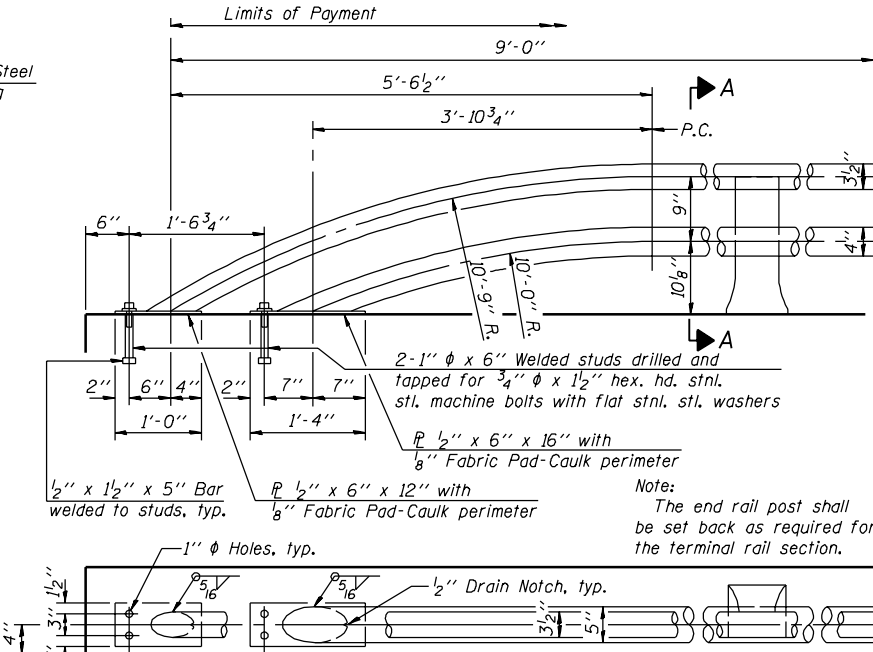


SECTION THRU SPLICE

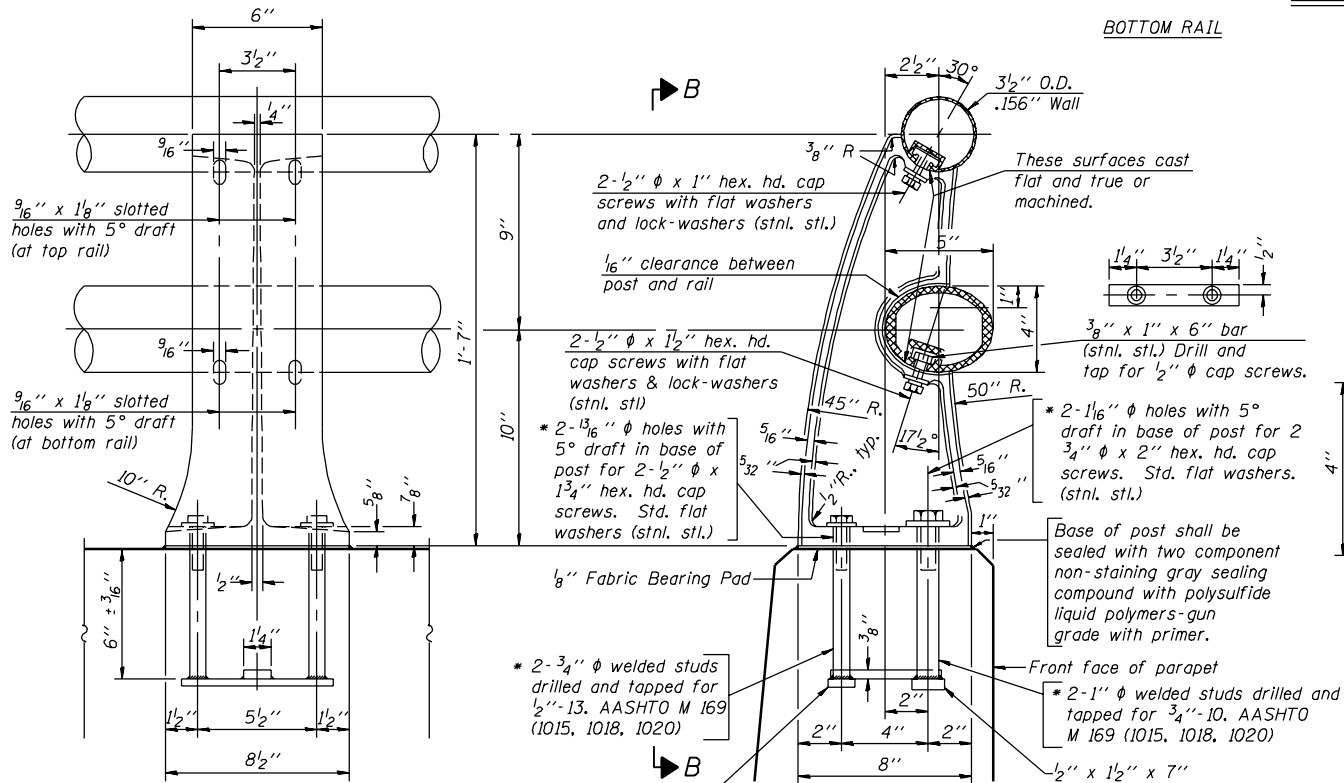
For Top Rail



RAIL SPLICE

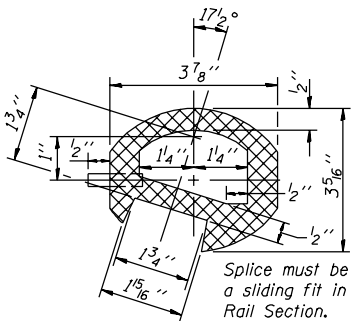


RAIL TERMINAL SECTION



RAIL POST DETAILS

SEC. THRU ELLIPTICAL  
RAIL SECTION



SEC. THRU SPLICE

BILL OF MATERIAL

Item	Unit	Quantity
Aluminum Railing, Type L	Foot	

TYPE L  
ALUMINUM RAILING

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	ENGINEER OF BRIDGE DESIGN	
CHECKED -	PASSED	
	ENGINEER OF BRIDGES AND STRUCTURES	

\* In lieu of the cast-in-place anchor device shown, the Contractor has the option of drilling and epoxy grouting stainless steel anchor rods of the same diameter and grade as the specified cap screws. Embedment shall be according to the manufacturer's specifications.

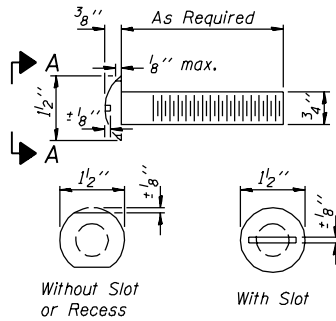


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

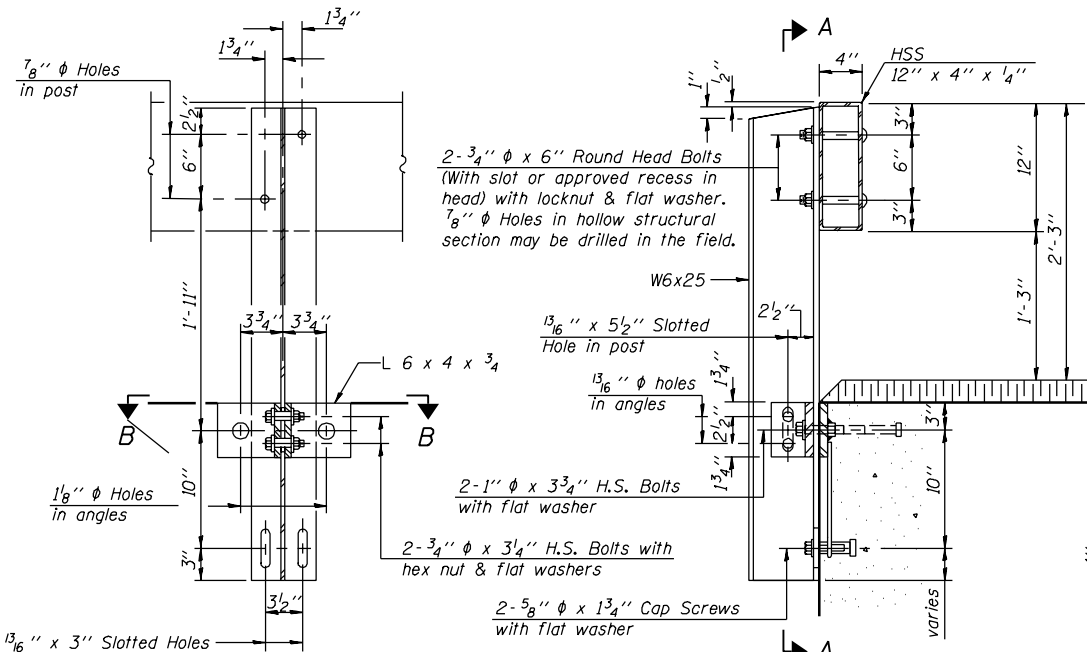
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT-		

SHEET NO. -  
- SHEETS

Contract #

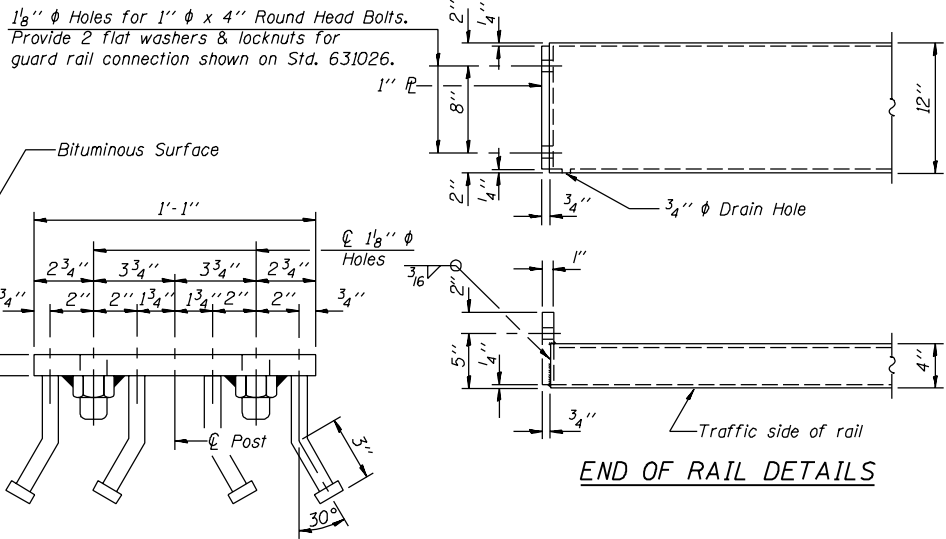


VIEW A-A  
ROUND HEAD BOLT



SECTION A-A

SECTION AT RAIL POST



VIEW C-C

END OF RAIL DETAILS

NOTES

Hollow structural sections shall conform to the requirements of ASTM designation A 500 Grade B Structural Steel Tubing and shall meet the longitudinal CVN requirements of 15 ft-lbs at 0° F.

All other steel shapes and plates shall conform to the requirements of AASHTO M 270 Grade 36 except posts and angles shall conform to AASHTO M 270, Grade 50.

Bolts, cap screws, and nuts shall conform to the requirements of ASTM designation A 307 except for high strength bolts, nuts and washers noted which shall conform to AASHTO M 164.

All bolts, nuts, cap screws, washers and lock washers shall be galvanized according to AASHTO M 232.

All posts, railing, rail splices, anchor devices and angles shall be galvanized after shop fabrication according to AASHTO M 111 and ASTM A 385. Galvanized rail shall not be painted.

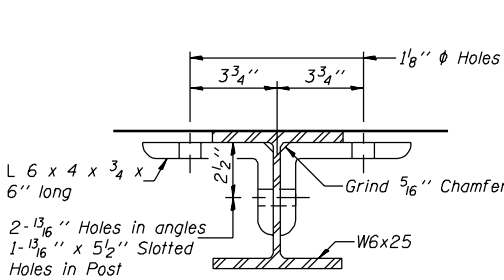
Railing shall be according to Section 509 of the Standard Specifications, except as noted, and will be paid for at the contract unit price per foot for STEEL RAILING, TYPE S-1.

All field drilled holes shall be coated with an approved zinc rich paint before erection.

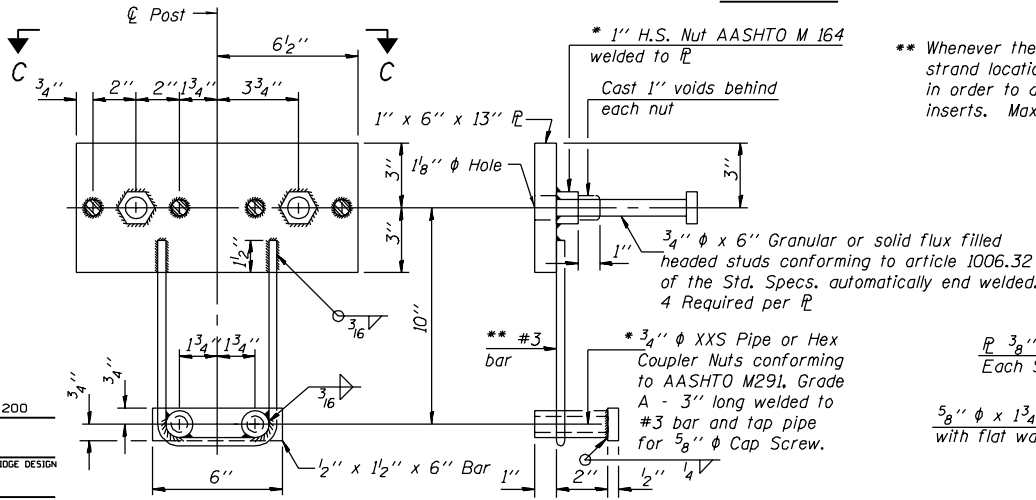
The lower portion of the post flange in contact with concrete shall receive two coats of asphalt paint conforming to Section 1060.07 Type II or place 1/8 inch fabric bearing pad between the post and concrete.

The 3/4 inch high strength bolts used to connect the 6 x 4 x 3/4 angles to the post shall be tightened according to Article 505.04(f)(2) of the Standard Specifications. The 1 inch high strength bolts connecting the angles to the concrete shall be tightened to a snug fit and given an additional 1/8 turn. The 5/8 inch cap screws in bottom of posts shall be tightened to a snug fit only.

For multi-span bridges, sufficient 1/4 inch x 6 inch x 1'-2 inch galvanized steel shims shall be provided to align rail between adjacent spans. Cost included with STEEL RAILING, TYPE S-1.

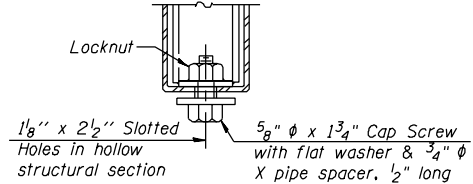


SECTION B-B



ANCHOR DEVICE

\*\* Whenever the lower insert assemblies interfere with strand locations, the #3 bars shall be cut and adjusted in order to allow raising or lowering of the lower inserts. Maximum adjustment not to exceed 1/2 inch.

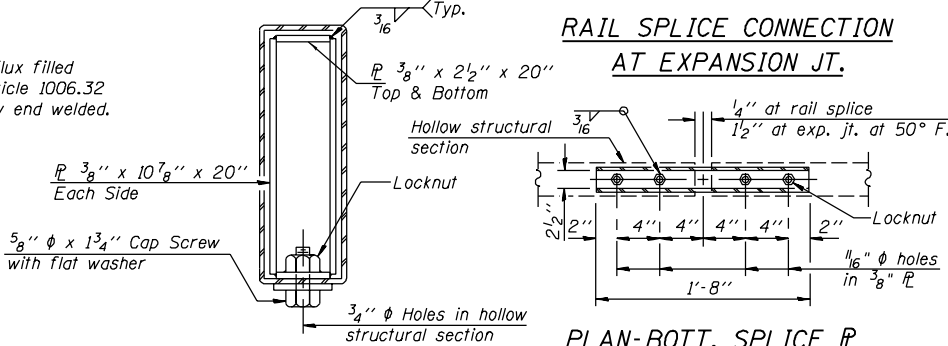


RAIL SPLICE CONNECTION  
AT EXPANSION JT.

BILL OF MATERIAL

Item	Unit	Quantity
Steel Railing Type S-1	Foot	

TYPE S-1  
STEEL RAILING



SECTIONS AT RAIL SPLICE

PLAN-BOTT. SPLICE R  
TYPICAL

DESIGNED -	200
CHECKED -	ENGINEER OF BRIDGE DESIGN
DRAWN -	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -	

R-23A 10-22-04 (10'-9" Maximum Post Spacing)

\* Threaded areas shall be plugged or blocked off during casting of beam.



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-		
-	-	-		
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -  
- SHEETS

$5/8'' \phi$  Holes in post  
 $7/8'' \phi$  Holes in post  
 $1/8'' \phi$  Holes in angles  
 $2-1/2'' \phi \times 6''$  Round Head Bolts (With slot or approved recess in head.) with locknut & flat washer  
 $5/8'' \phi$  Holes in hollow structural section may be drilled in the field.  
 $2-3/4'' \phi \times 6''$  Round Head Bolts (With slot or approved recess in head.) with locknut & flat washer  
 $7/8'' \phi$  Holes in hollow structural section may be drilled in the field.  
 $W 6 \times 25$   
 $13/16'' \times 4''$  Slotted Hole in post  
 $13/16'' \phi$  Holes in angle  
 $2-1'' \phi \times 3 3/4''$  H.S. Bolts with flat washer  
 $2-3/4'' \phi \times 3 1/4''$  H.S. Bolts with hex nut & flat washers  
 $2-5/8'' \phi \times 1 3/4''$  Cap Screws with flat washer  
 $4'' \times 4'' \times 1/4''$  HSS  
 $12'' \times 4'' \times 1/4''$  HSS  
 Dimensions:  $1 1/2''$ ,  $3 1/2''$ ,  $10''$ ,  $6''$ ,  $12 1/2''$ ,  $11''$ ,  $3''$ ,  $3 1/2''$ ,  $4''$ ,  $2''$ ,  $1 1/2''$ ,  $2''$ ,  $4''$ ,  $5''$ ,  $12''$ ,  $3''$ ,  $6''$ ,  $2 1/2''$ ,  $1 1/4''$ ,  $2 1/2''$ ,  $4 1/2''$ ,  $3''$ ,  $10''$ ,  $5''$ ,  $3'-0''$

Technical drawing of a steel beam-to-column connection. The drawing shows a side elevation of the joint. A horizontal beam is connected to a vertical column. The beam has two  $1\frac{1}{8}$ "  $\phi$  Holes, each spaced  $3\frac{3}{4}$ " from the centerline. The column has a  $2\frac{13}{16}$ "  $\phi$  Holes in angles and  $1\frac{13}{16}$ " x 4" Slotted Holes in Post. The beam is made of L 6 x 4 x  $\frac{3}{4}$ " x 5" long. The column is a W 6 x 25. A Grind  $\frac{5}{16}$ " Chamfer is shown on the column. Dimensions are given in inches.

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

10-22-04 (9'-6" Maximum Post Spacing)

1/8"  $\phi$  Holes for 1"  $\phi$  x 4" Round Head Bolts  
Provide 2 flat washers & locknuts for  
guard rail connection shown on Std. 631026.

\*  $\frac{3}{4}$ "  $\phi$  (nominal i.d.) XXS Pipe -  
or Hex Coupler Nuts conforming  
to AASHTO M291, Grade A - 3  
long. Tap pipe for  $\frac{5}{8}$ "  $\phi$  Cap  
Screw

[illegible]

**BOTTOM ANCHOR DEVICE**

1" H.S. Nut AASHTO M 164 welded to R

Cast 1" voids behind each nut.

R 1" x 5" x 13"

1/8"  $\phi$  Hole

2"

2 1/2"

3 1/2"

35°

3/4"  $\phi$  x 7" Granular

1"

5/8"

W

d.) XXS Pipe

$\frac{3}{4}$ "  $\phi$  x 6" Round Head Bolts  
(with slot or approved recess in head  
with locknut & flat washer)

Traffic side of rail

Diagram illustrating the dimensions and components of a hollow structural section (HSTO) assembly:

- Top & Bottom:**  $\frac{P}{P}$   $3\frac{3}{8}'' \times 2\frac{1}{2}'' \times 20''$
- Each Side:**  $\frac{P}{P}$   $3\frac{3}{8}'' \times 10\frac{7}{8}'' \times 20''$
- Locknut:** Indicated by a label pointing to the bottom fastener.
- Cap Screw:**  $5\frac{5}{8}'' \phi \times 1\frac{3}{4}''$  Cap Screw with flat washer.
- Hollow structural section:** Indicated by a label pointing to the main vertical member.
- Holes:**  $3\frac{3}{4}'' \phi$  Holes in hollow structural section.
- Typ.:** Typical section indicated by a bracket.

Technical drawing of a locknut assembly for a rail splice. The drawing shows a cross-section of a rail with a locknut and a hollow structural section. Dimensions include:  $\frac{3}{16}$  inch for the rail thickness,  $\frac{1}{2}$  inch for the rail splice,  $\frac{1}{2}$  inch for the exp. jt. at  $50^\circ F.$ ,  $2\frac{1}{2}$  inch for the locknut height, 2 inch for the locknut width, 4 inch for the spacing between the four holes, 1'-8 inch for the total length of the locknut, and  $\frac{1}{16}$  inch diameter holes in  $\frac{3}{8}$  inch plate. The locknut is labeled "Locknut" and the hollow structural section is labeled "Hollow structural section".

Follow structural sections shall conform to the requirements of ASTM designation A 500 Grade B Structural Steel Tubing and shall meet the longitudinal CVN requirements of 15 ft.-lbs at 0 °F.

All other steel shapes and plates shall conform to the requirements of AASHTO M 270 Grade 36 except posts and angles shall conform to AASHTO M 270, Grade 50.

Bolts, cap screws, and nuts shall conform to the requirements of ASTM designation A 307 except for high strength bolts, nuts and washers noted which shall conform to AASHTO M 164.

All bolts, nuts, cap screws, washers and lock washers shall be galvanized according to AASHTO M 232.

All posts, railing, rail splices, anchor devices and angles shall be galvanized after shop fabrication according to AASHTO M 111 and ASTM A 385. Galvanized rail shall not be painted.

Railing shall be according to Section 509 of the Standard Specifications, except as noted, and will be paid for at the contract unit price per foot for STEEL RAILING, TYPE T-1.

All field drilled holes shall be coated with an approved zinc rich paint before erection.

The lower portion of the post flange in contact with concrete shall receive two coats of asphalt paint conforming to Section 1060.07 Type II or place  $\frac{1}{8}$ " fabric bearing pad between the post and concrete.

The  $\frac{3}{4}$ "  $\phi$  high strength bolts used to connect the 6 x 4 x  $\frac{3}{4}$  angles to the post shall be tightened according to Article 505.04f(x2) of the Standard Specifications. The 1"  $\phi$  high strength bolts connecting the angles to the concrete shall be tightened to a snug fit and given an additional  $\frac{1}{8}$  turn. The  $\frac{3}{8}$ "  $\phi$  cap screws in bottom of posts shall be tightened to a snug fit only.

For multi-span bridges, sufficient  $\frac{1}{4}$ " x 6" x 1'-5" galvanized steel shims shall be provided to align rail between adjacent spans. Cost included with Steel Railing.

Item	Unit	Quantity
Steel Railing Type T-1	Foot	

TYPE T-1  
STEEL RAILING



SHEET NO. -  
- SHEETS



\* 4-1 $\frac{1}{4}$ "  $\phi$  Formed holes for 1"  $\phi$  H.S. bolts with nut & flat washers. Grout holes in deck after bolt removal.

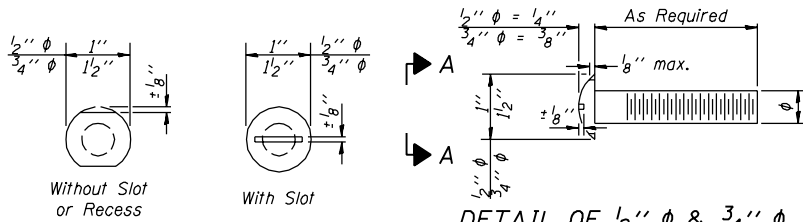
NEW &amp; EXISTING DECKS



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

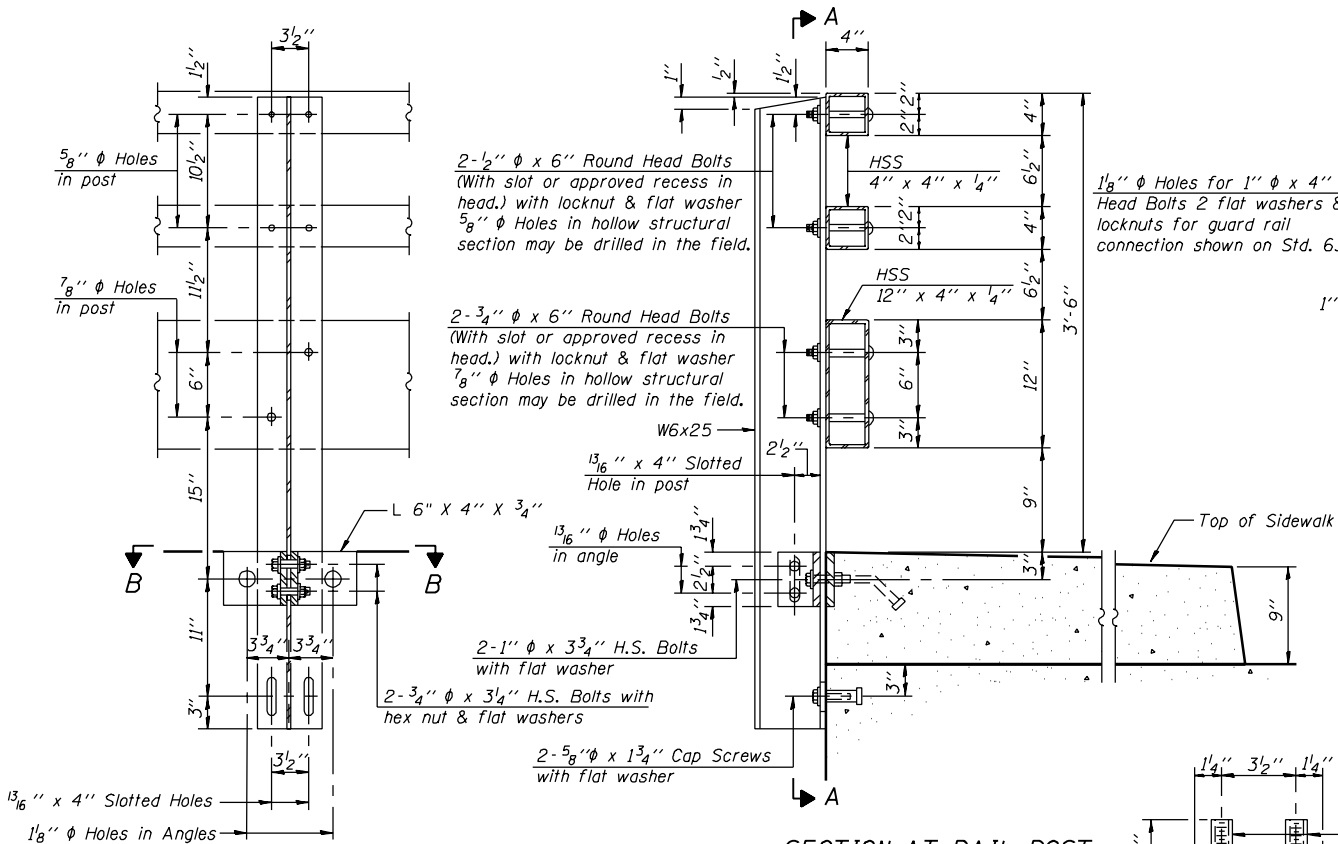
ROUTE NO.	SECTION	COUNTY	DATE	SHEET NO.	SHEET NO.
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT			

Contract #

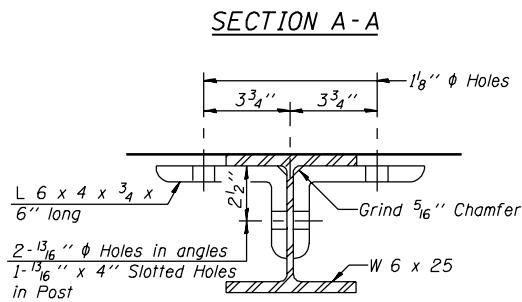


VIEW A-A

DETAIL OF 1/2"  $\phi$  & 3/4"  $\phi$   
ROUND HEAD BOLTS



SECTION AT RAIL POST



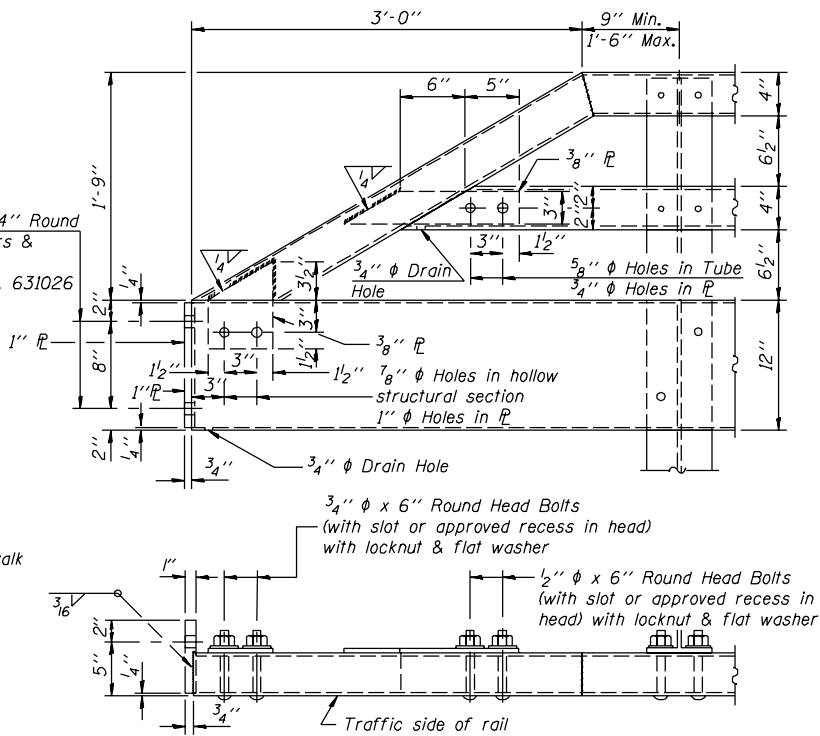
SECTION B-B

DESIGNED -	200
CHECKED -	ENGINEER OF BRIDGE DESIGN
DRAWN -	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -	

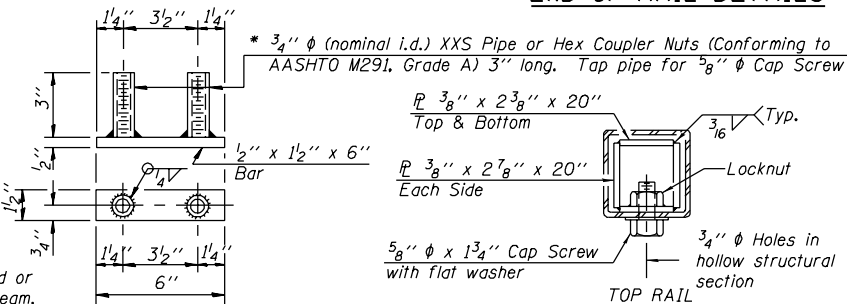
R-26

10-22-04

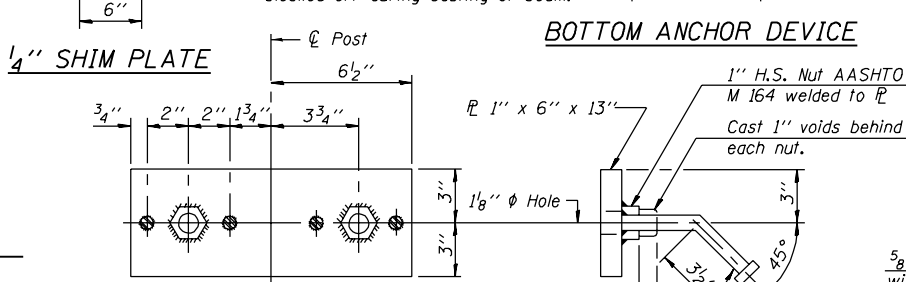
(10'-6" Maximum Post Spacing)



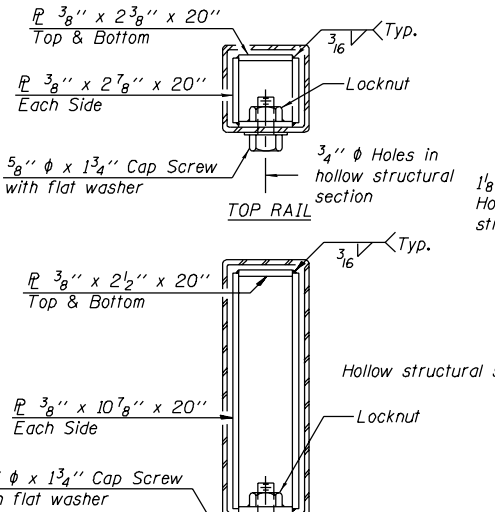
END OF RAIL DETAILS



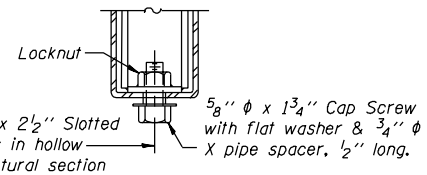
BOTTOM ANCHOR DEVICE



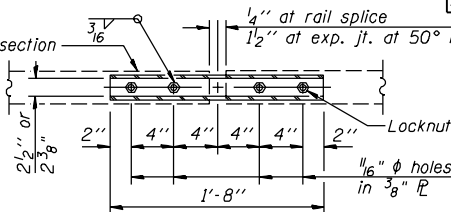
TOP ANCHOR DEVICE



SECTIONS AT RAIL SPLICE



RAIL SPLICE CONNECTION  
AT EXPANSION JT.



PLAN-BOTT. SPLICE RAIL  
TYPICAL

NOTES

Hollow structural sections shall conform to the requirements of ASTM designation A 500, Grade B, Structural Steel Tubing and shall meet the longitudinal CVN requirements of 15 ft-lbs at 0°F.

All other steel shapes and plates shall conform to the requirements of AASHTO M 270 Grade 36 except posts and angles shall conform to AASHTO M 270, Grade 50.

Bolts, cap screws, and nuts shall conform to the requirements of ASTM designation A 307 except for high strength bolts, nuts and washers noted which shall conform to AASHTO M 164.

All bolts, nuts, cap screws, washers and lock washers shall be galvanized according to AASHTO M 232.

All posts, railing, rail splices, anchor devices and angles shall be galvanized after shop fabrication according to AASHTO M 111 and ASTM A 385. Galvanized rail shall not be painted.

Railing shall be according to Section 509 of the Standard Specifications, except as noted, and will be paid for at the contract unit price per foot for STEEL RAILING, TYPE TP-1.

All field drilled holes shall be coated with an approved zinc rich paint before erection.

The lower portion of the post flange in contact with concrete shall receive two coats of asphalt paint conforming to Section 1060.07 Type II or place 1/8" fabric bearing pad between the post and concrete.

The 3/4"  $\phi$  high strength bolts used to connect the 6 x 4 x 3/4" angles to the post shall be tightened according to Article 505.04(f)(2) of the Standard Specifications. The 1"  $\phi$  high strength bolts connecting the angles to the concrete shall be tightened to a snug fit and given an additional 1/8 turn. The 5/8"  $\phi$  cap screws in bottom of posts shall be tightened to a snug fit only.

For multi-span bridges, sufficient 1/4" x 6" x 1'-3/2" galvanized steel shims shall be provided to align rail between adjacent spans. Cost included with Steel Railing.

BILL OF MATERIAL

Item	Unit	Quantity
Steel Railing, Type TP-1	Foot	

TYPE TP-1  
COMBINATION STEEL RAILING



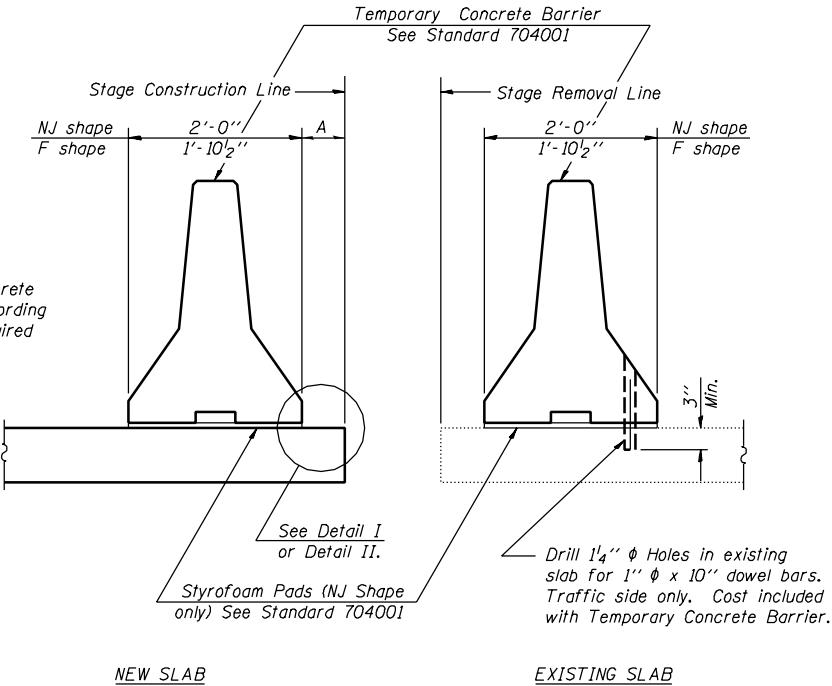
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

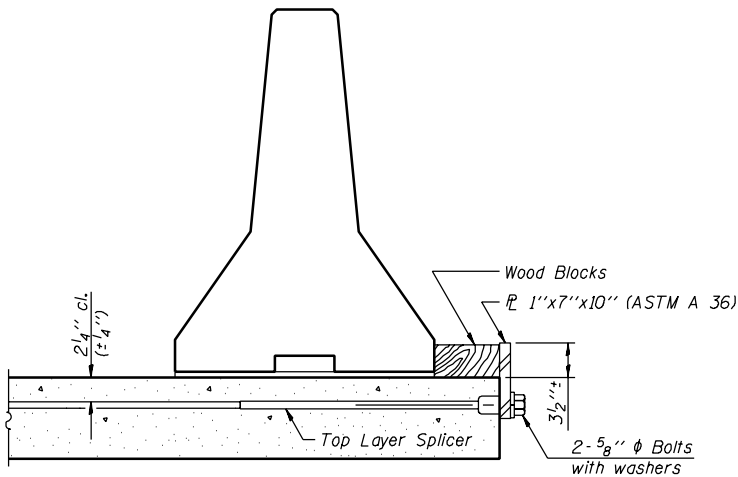
SHEET NO. -  
- SHEETS

Contract #

When "A" is 3'-6" or less, the temporary concrete barrier shall be anchored to the new slab according to Detail I or Detail II. No anchorage is required when "A" is greater than 3'-6".

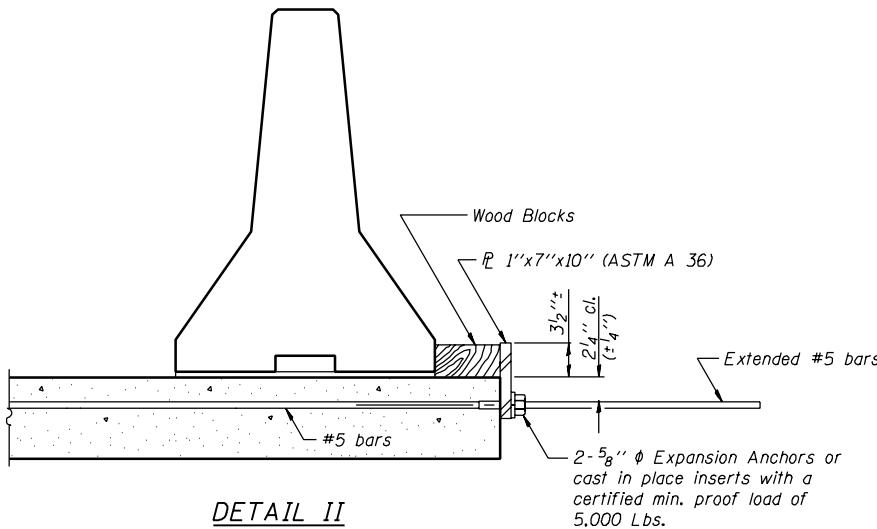


SECTIONS THRU SLAB



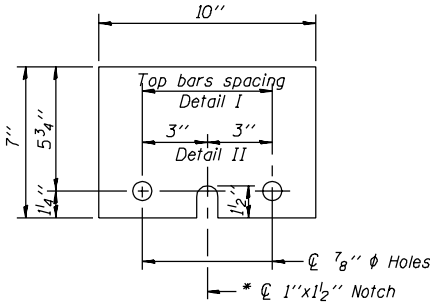
DETAIL I

The 1"x7"x10" Plate shall not be removed until  
Stage II Construction forms and reinforcement bars  
are in place.



DETAIL II

The 1"x7"x10" Plate shall not be removed until  
Stage II Construction forms and all reinforcement  
bars are in place and the concrete is ready to be  
placed.



1"x7"x10"

\* Required only with Detail II

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	ENGINEER OF BRIDGE DESIGN	
CHECKED -	PASSED	
	ENGINEER OF BRIDGES AND STRUCTURES	

R-27

10-22-04

TEMPORARY CONCRETE BARRIER  
FOR STAGE CONSTRUCTION



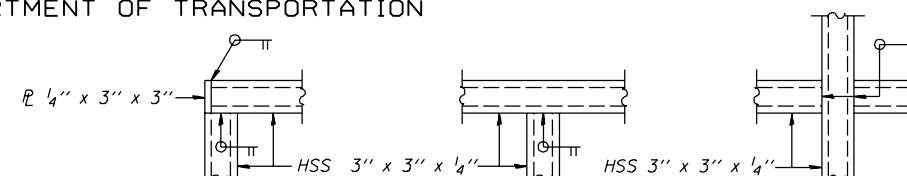
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
"	"	"		"
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -  
 - SHEETS

NOTES

Space reinforcement to miss anchor rods.

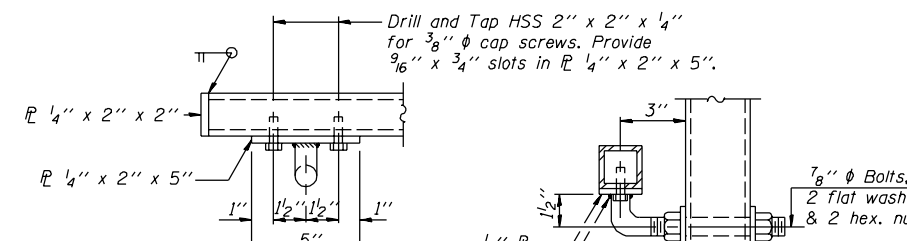
Only one of the above notes would appear on Contract Plans.



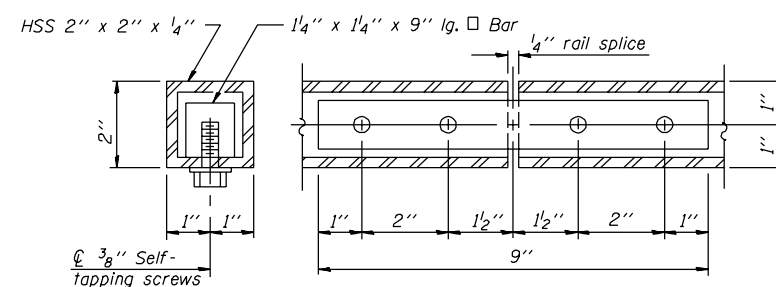
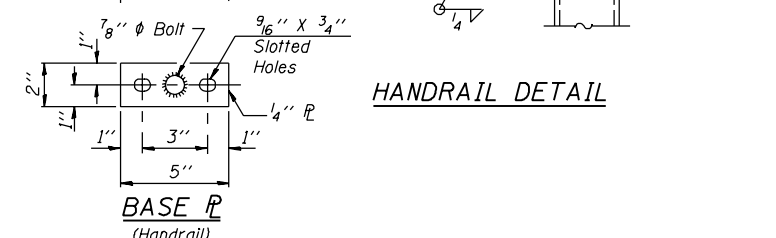
DETAIL A

DETAIL B

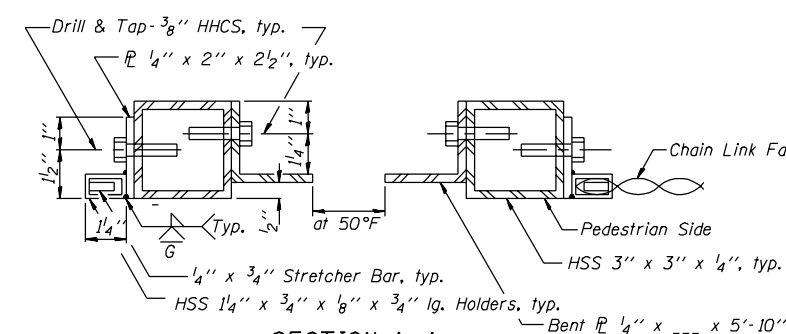
DETAIL C



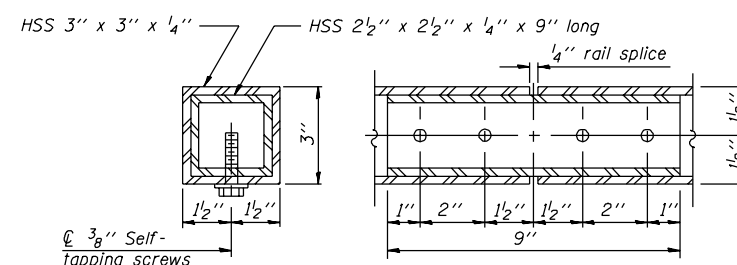
### HANDRAIL DETAIL



## HANDRAIL SPLICE



SECTION A-A

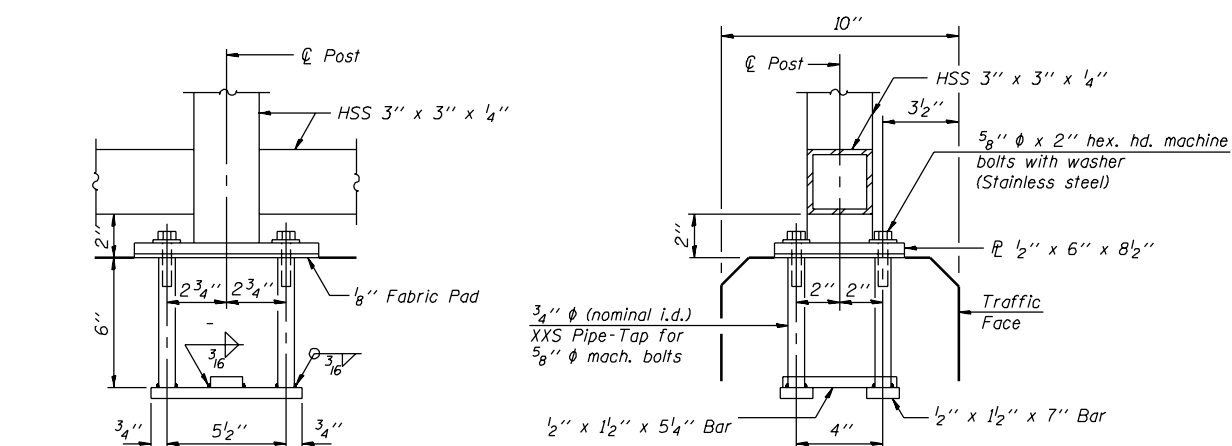


RAIL SPLICE

### BILL OF MATERIAL

Item	Unit	Quantity
Pedestrian Railing	Foot	

## PEDESTRIAN RAILING



### ANCHOR BOLT DETAILS

Technical drawing of the BASE PLATE. The drawing shows a square plate with overall dimensions of 8 1/2" by 6". The plate has four mounting holes, each with a diameter of 1/2". The mounting holes are spaced 2" apart horizontally and vertically. The plate has a central slot with a width of 1" and a length of 3". The slot is located 1" from the top and bottom edges. The plate has a thickness of 1/4". The plate is labeled "BASE PL" at the bottom.

BASE  $\mathbb{P}$

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -		ENGINEER OF BRIDGE DESIGN
CHECKED -	PASSED	
		ENGINEER OF BRIDGES AND STRUCTURE

R-28

10-22-04 (10'-0" Maximum Post Spacing)



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	100% SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT-		

SHEET NO. -  
- SHEETS

Contract # **NOTES**

Railing shall be according to Section 509 of the Standard Specifications, except as noted, and will be paid for at the Contract Unit Price per foot for Bicycle Railing.

The 9 gauge fabric ties shall be according to Article 1006.27 (d) of the Standard Specifications.

Installation of the chain link fabric shall be according to Section 664 of the Standard Specifications.

Hollow structural sections shall conform to the requirements of ASTM designation A 500, Grade B, structural steel tubing.

All other steel shapes and plates shall conform to the requirements of AASHTO M 270 Grade 36.

The Parapet Railing, furnished and installed shall not be paid for separately but shall be included in the unit bid price for Bicycle Railing.

The chain link fabric shall be placed along Bicycle Side as shown on Section A-A.

Stretcher bars shall be used at all four sides of each panel.

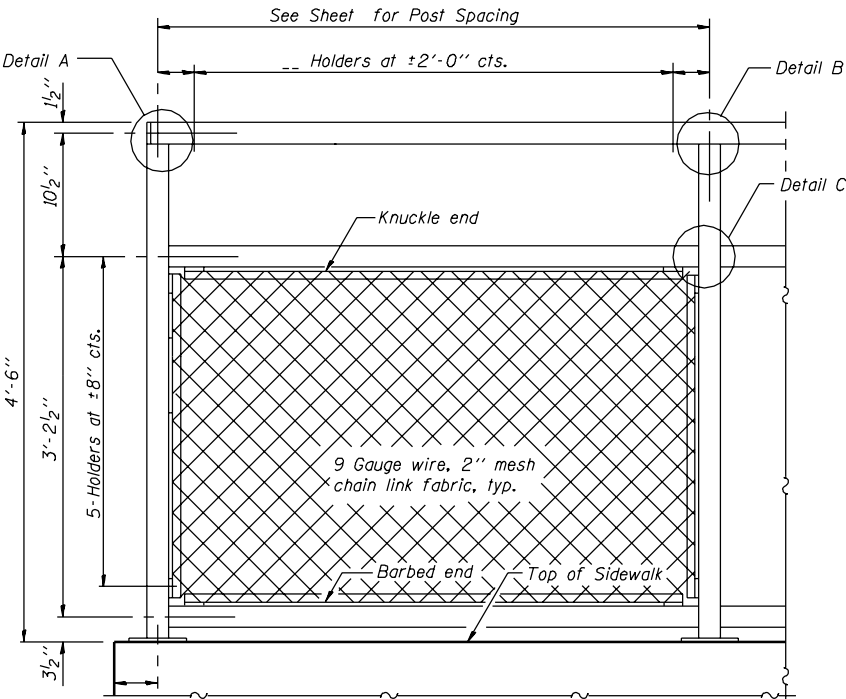
If the option of drilling and epoxy grouting the anchor rods is chosen, the Contractor shall use the capsule or the adhesive cartridge type anchor rods that have been previously tested and given a prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures. The capsule or the adhesive cartridge shall be sealed with pre-measured amounts of the adhesive chemical.

Space reinforcement to miss anchor rods.

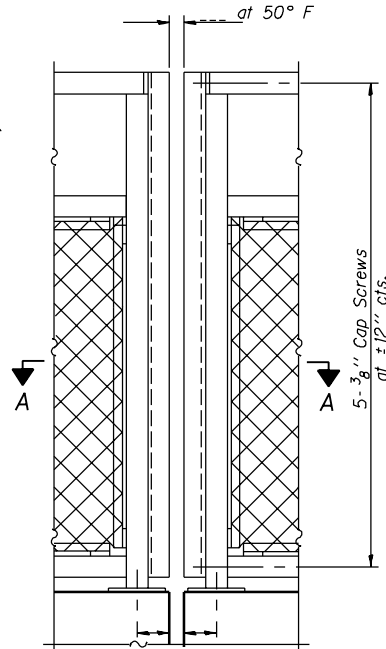
The designer should add the appropriate note as applicable.

- A. When railing is galvanized:
- All posts, railing, splices, anchor devices, and bent plates shall be galvanized after shop fabrication according to AASHTO M III and ASTM A 385. All bolts, nuts, washers, and anchor rods shall be galvanized according to AASHTO M 232 except stainless steel bolts as noted.
- Vent holes for galvanizing shall be placed in the posts and rails at locations that will not allow the accumulation of moisture in the members.
- The chain link fabric shall conform to the requirements of Article 1006.27(a)(1)a, b or c of the Standard Specifications.
- B. When railing is painted:
- All post, railing, splices, anchor devices, and bent plates shall be painted using the (List the appropriate paint system for Structural Steel).
- The chain link fabric shall conform to the requirements of Article 1006.27(a)(1)d of the Standard Spec's.

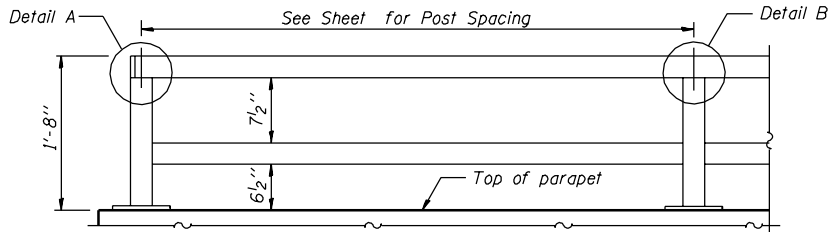
Only one of the above notes would appear on Contract Plans.



**BICYCLE RAILING**

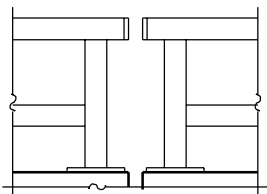


**BICYCLE RAILING**



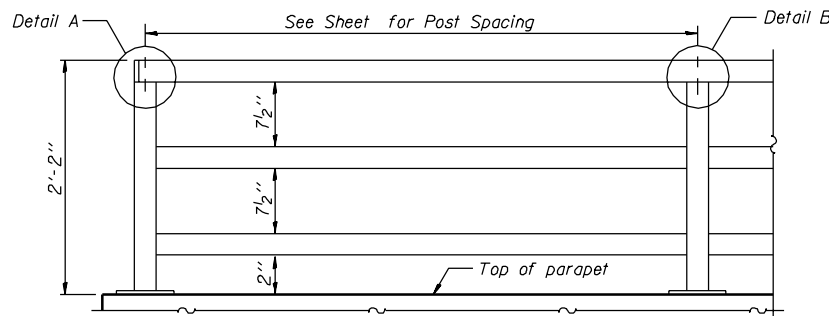
**PARAPET RAILING  
ELEVATION**

(Inside Face of Two Element Rail)



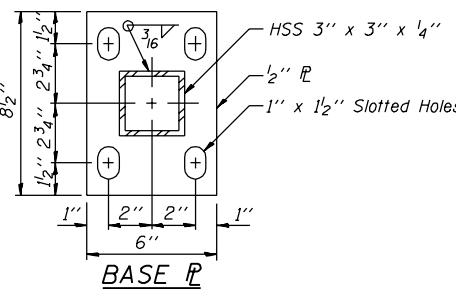
**PARAPET RAILING  
ELEVATION AT EXPANSION JOINT**

(Two Element Rail Shown - Three Element Rail Similar)

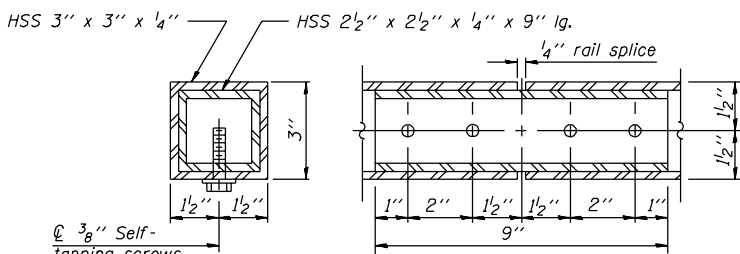


**PARAPET RAILING  
ELEVATION**

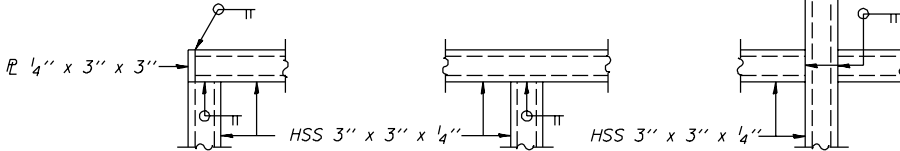
(Inside Face of Three Element Rail)



**BASE P**



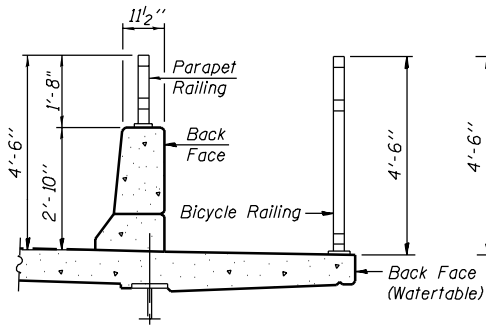
**RAIL SPLICE**



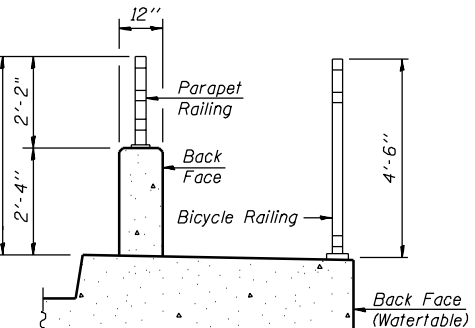
**DETAIL A**

**DETAIL B**

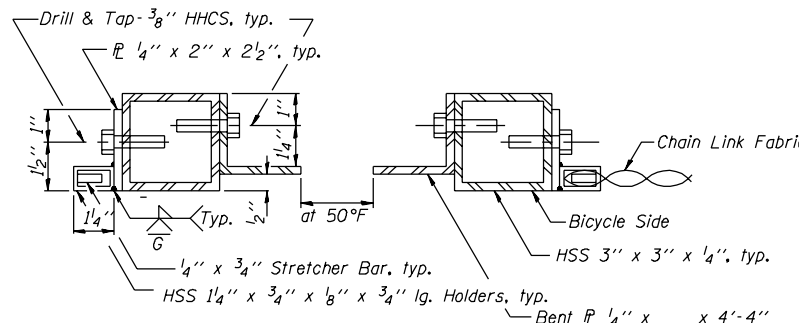
**DETAIL C**



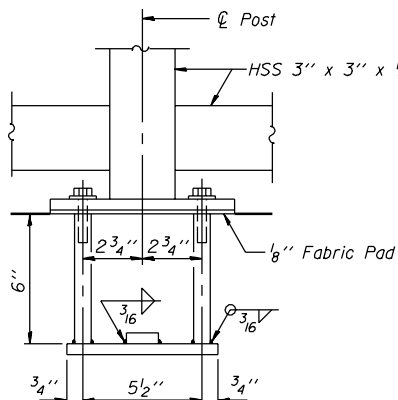
**SECTION THRU DECK**



**SECTION THRU SIDEWALK**

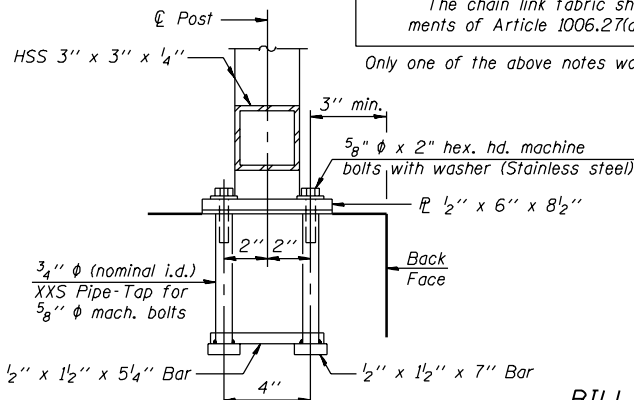


**SECTION A-A**



**ANCHOR BOLT DETAILS**

In lieu of the cast-in-place anchor device shown, the Contractor has the option of drilling and epoxy grouting 5/8"  $\phi$  anchor rods. Embedment shall be according to the manufacturer's specifications.



**BILL OF MATERIAL**

Item	Unit	Quantity
Bicycle Railing	Foot	

**BICYCLE RAILING**

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	PASSED	ENGINEER OF BRIDGE DESIGN
CHECKED -		ENGINEER OF BRIDGES AND STRUCTURES



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	101% SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT-		

SHEET NO. -  
- SHEETS

Contract #NOTES

Hollow structural sections shall conform to the requirements of ASTM designation A 500 Grade B Structural Steel Tubing and shall meet the longitudinal CVN requirements of 15 ft-lbs at 0° F.

All other steel shapes and plates shall conform to the requirements of AASHTO M 270 Grade 36 except posts and angles shall conform to AASHTO M 270, Grade 50.

Bolts, cap screws and nuts shall conform to the requirements of ASTM designation A 307 except for high strength bolts, nuts and washers noted which shall conform to AASHTO M 164.

All bolts, nuts, cap screws, washers and lock washers shall be galvanized according to AASHTO M 232.

All posts, railing, rail splices, anchor devices and angles shall be galvanized after shop fabrication according to AASHTO M 111 and ASTM A 385. Galvanized rail shall not be painted.

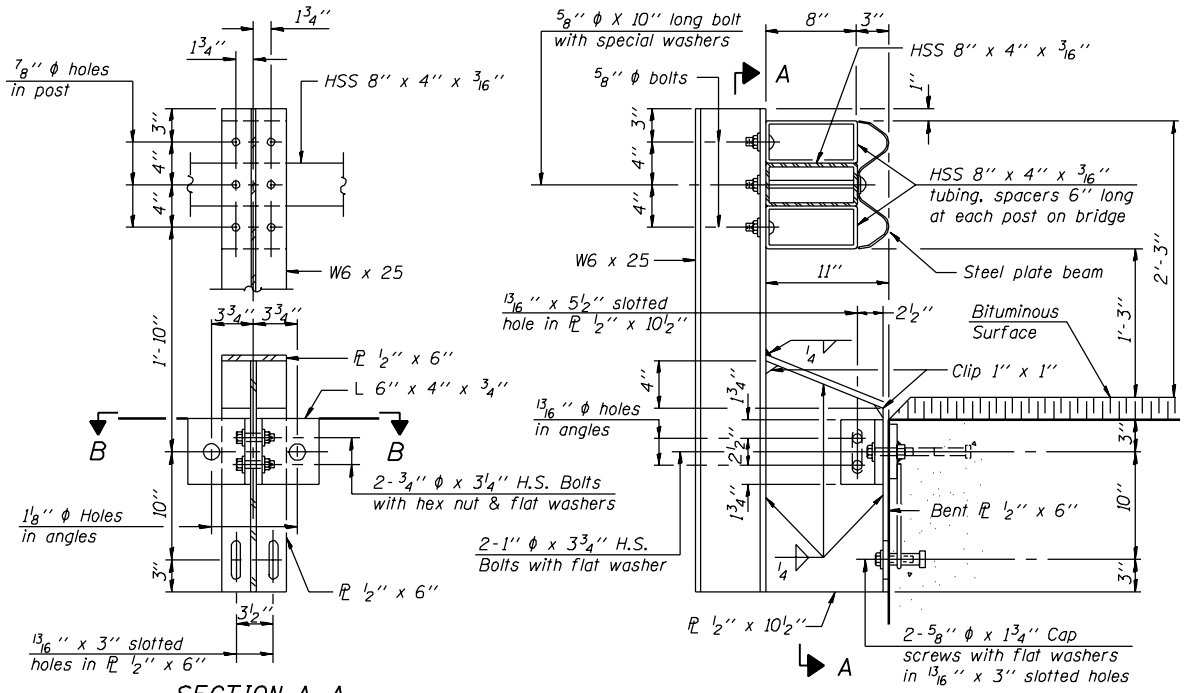
Railing shall be according to Section 509 of the Standard Specifications, except as noted, and will be paid for at the contract unit price per foot for STEEL RAILING, TYPE WT.

All field drilled holes shall be coated with an approved zinc rich paint before erection.

The 1/2" x 6" plates that come in contact with concrete shall receive two coats of asphalt paint conforming to Section 1060.07 Type II or place 1/8" fabric bearing pads between the plates and concrete.

The 3/4" high strength bolts used to connect the 6" x 4" x 3/4" angles to the post shall be tightened according to Article 505.04(F)(2) of the Standard Specifications. The 1" high strength bolts connecting the angles to the concrete shall be tightened to a snug fit and given an additional 1/8 turn. The 5/8" cap screws in bottom of posts shall be tightened to a snug fit only.

For multi-span bridges, sufficient 1/4" x 6" x 1'-2" galvanized steel shims shall be provided to align rail between adjacent spans. Cost included with Steel Railing.

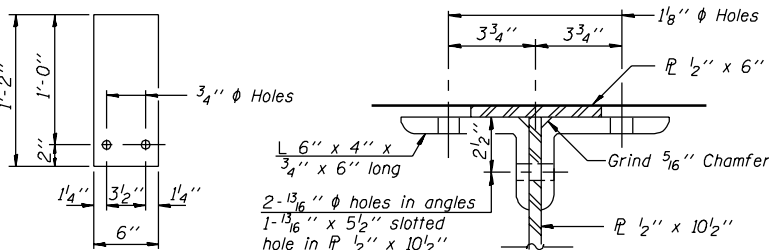


SECTION A-A

SECTION AT RAIL POST

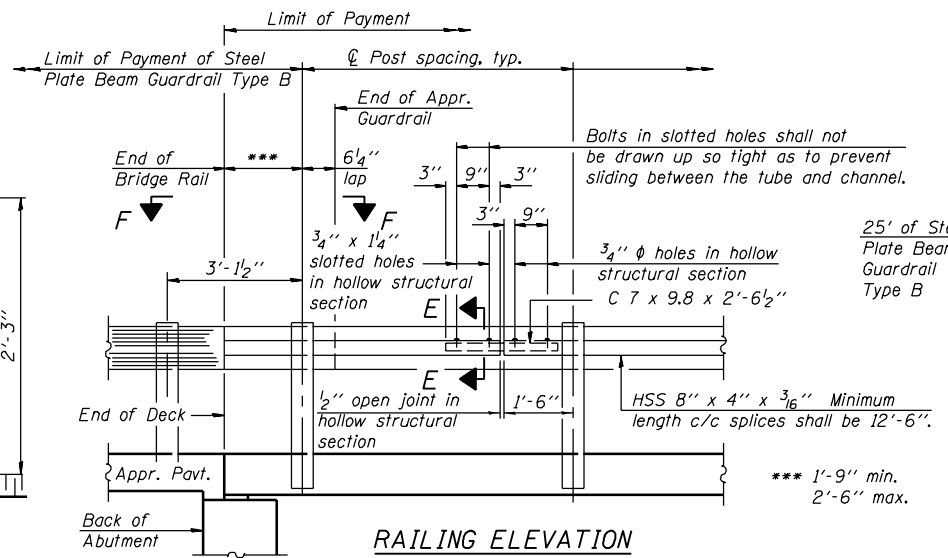
DESIGNED -	200
CHECKED -	ENGINEER OF BRIDGE DESIGN
DRAWN -	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -	

R-30 10-22-04 6'-3" Maximum Post Spacing

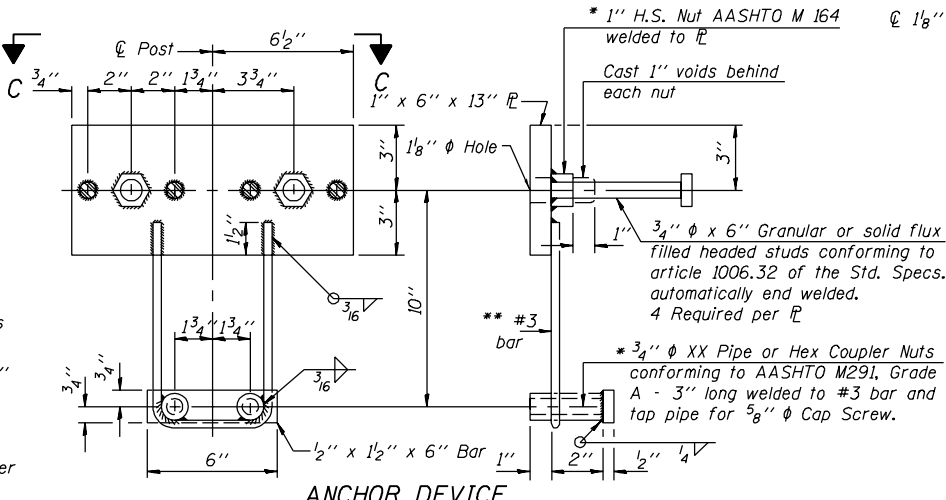


1/4" SHIM PLATE

SECTION B-B

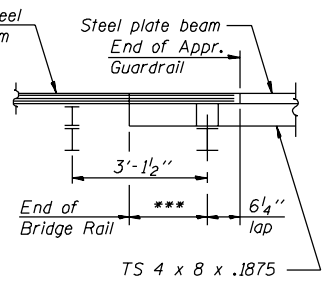


RAILING ELEVATION

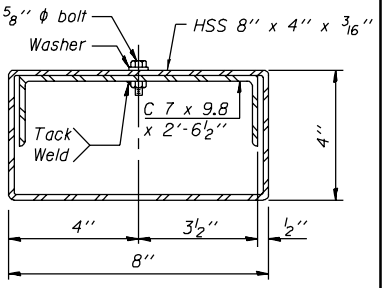


ANCHOR DEVICE

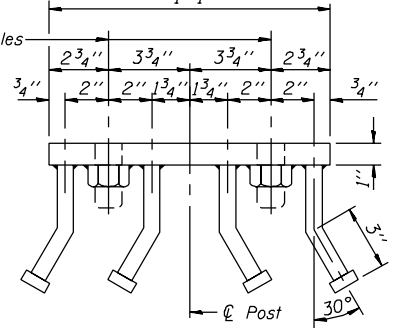
- \* Threaded areas shall be plugged or blocked off during casting of beam.
- \*\* Whenever the lower insert assemblies interfere with strand locations, the #3 bars shall be cut and adjusted in order to allow raising or lowering of the lower inserts. Maximum adjustment not to exceed 1/2".



VIEW F-F



SECTION E-E



VIEW C-C

BILL OF MATERIAL

Item	Unit	Quantity
Steel Railing, Type WT	Foot	

TYPE WT  
STEEL RAILING



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	101% SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

SHEET NO. -  
SHEETS

Contract #

NOTES

Hollow structural sections shall conform to the requirements of ASTM designation A 500, Grade B, Structural Steel Tubing and shall meet the longitudinal CVN requirements of 15 ft-lbs at 0°F.

All other steel shapes and plates shall conform to the requirements of AASHTO M 270 Grade 36 except posts shall conform to AASHTO M 270, Grade 50.

Bolts, cap screws and nuts shall conform to the requirements of ASTM designation A 307 except that threaded rods, nuts and washers shall conform to AASHTO M 164.

All bolts, nuts, cap screws, washers and lock washers shall be galvanized according to AASHTO M 232.

All posts, railing, rail splices and anchor rods shall be galvanized after shop fabrication according to AASHTO M 111 and ASTM A 385. Galvanized rail shall not be painted.

Railing shall be according to Section 509 of the Standard Specifications, except as noted, and will be paid for at the contract unit price per foot for Steel Bridge Rail.

All field drilled holes shall be coated with an approved zinc rich paint before erection.

Posts shall not be located closer than 1'-3" to an existing bridge expansion joint or end of bridge.

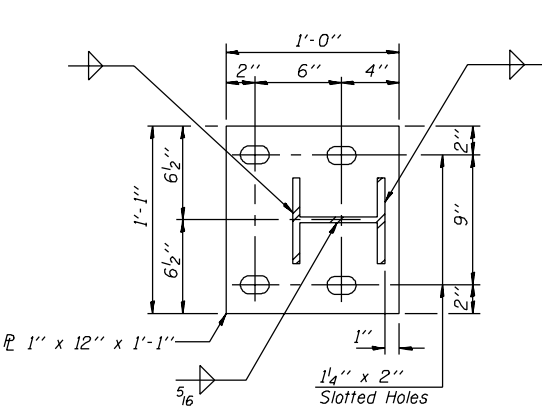
Steel Bridge Rail expansion joint shall be provided between any two (2) posts which span a bridge expansion joint. Bolts located at expansion joint shall be provided with locknuts and shall be tightened only to a point that will allow railing movement.

Provide one 1/8" and two 1/16" steel shims for 25% of the posts. Shims shall be similar to base plates in size and holes.

Expansion joint width shall be "D" at 50° F and shall be adjusted for other temperatures according to Article 503.10(c) of the Standard Specifications.

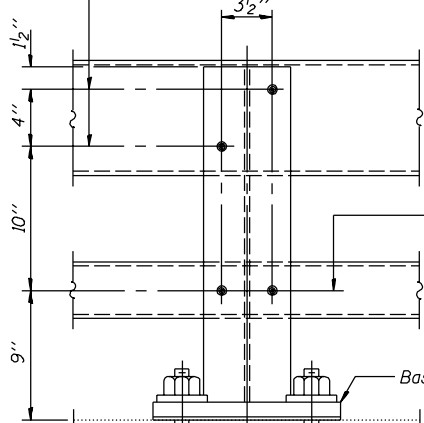
The Contractor shall use the capsule or the adhesive cartridge type anchor rods that have been previously tested and given a prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures. The capsule or the adhesive cartridge shall be sealed with pre-measured amounts of the adhesive chemical.

Nuts for 1"  $\phi$  threaded anchor rods connecting the base plate to the concrete shall be tightened to a snug fit and given an additional 1/8 turn.

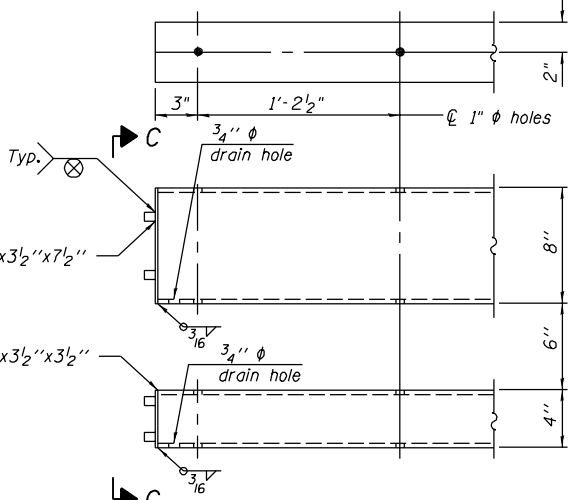


BASE PLATE DETAIL

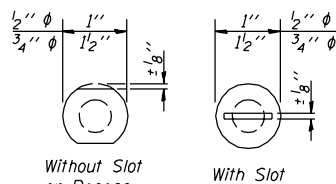
2-3/4"  $\phi$  x 6" Round Head Bolts (With slot or approved recess in head.) with locknut and flat washer. 7/8"  $\phi$  Holes in tubing and posts. Holes in hollow structural section may be drilled in the field.



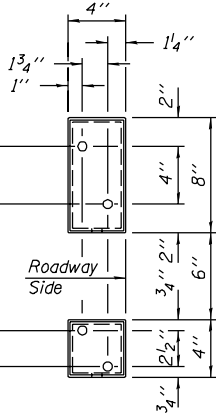
SECTION A-A



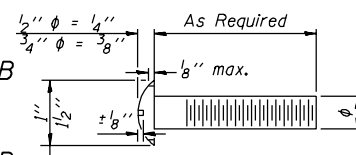
END OF RAIL DETAILS



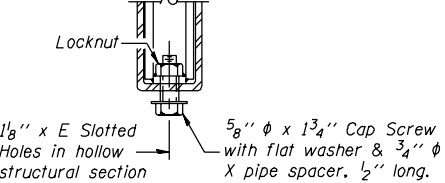
VIEW B-B



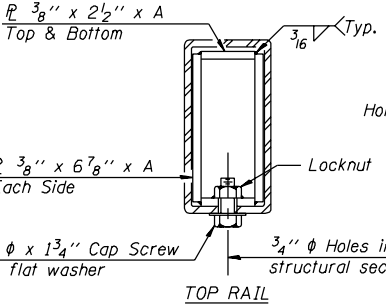
VIEW C-C



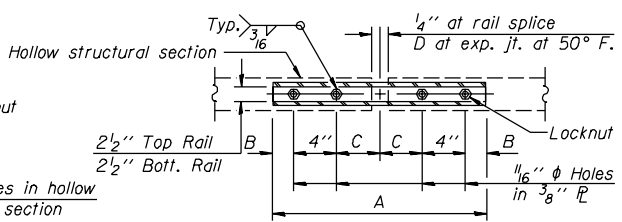
DETAIL OF 1/2"  $\phi$  & 3/4"  $\phi$  ROUND HEAD BOLTS



RAIL SPLICE CONNECTION AT EXPANSION JT.



SECTIONS AT RAIL SPLICE



PLAN-BOTT. SPLICE P TYPICAL

BILL OF MATERIAL

Item	Unit	Quantity
Steel Bridge Rail	Foot	

SPLICE DIMENSIONS

T	D	A	B	C	E
≤4"	2 1/2"	1'-8"	2"	4"	2 1/2"
>4" ≤6 1/2"	3 3/4"	2'-0"	2 1/2"	5 1/2"	3 1/2"
>6 1/2" ≤9"	5"	2'-4"	3 1/2"	6 1/2"	9"
>9" ≤13"	7"	2'-10"	4 1/2"	8 1/2"	11"
Rail Splice	1/4"	1'-8"	2"	4"	—

T = Total movement at expansion joint as shown on the design plans.

STEEL BRIDGE RAIL  
CURB MOUNTED  
(2399)

DESIGNED -	200
CHECKED -	EXAMINED
DRAWN -	PASSED
CHECKED -	ENGINEER OF BRIDGE DESIGN
	ENGINEER OF BRIDGES AND STRUCTURES

R-31

10-22-04

(6'-3" Maximum Post Spacing)



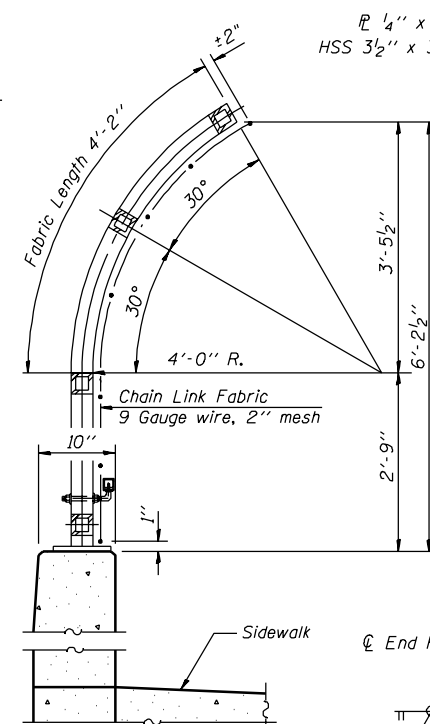
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -  
 - SHEETS

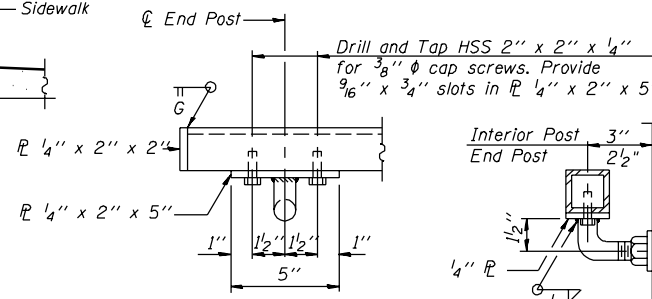
NOTES

Space reinforcement to miss anchor rods.

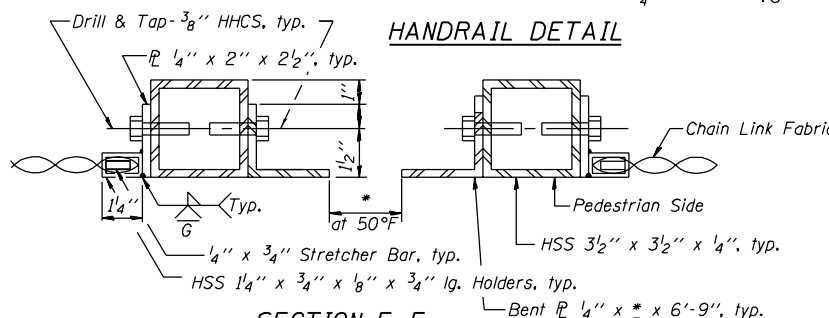
Only one of the above notes would appear on Contract Plans.



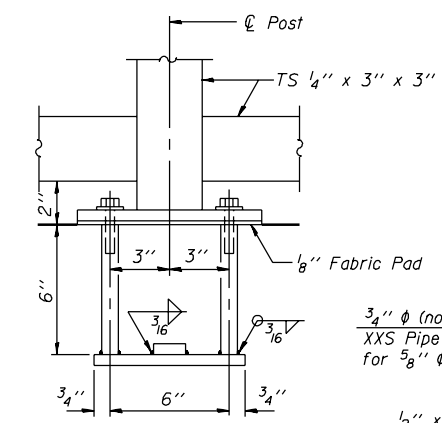
SECTION A-A



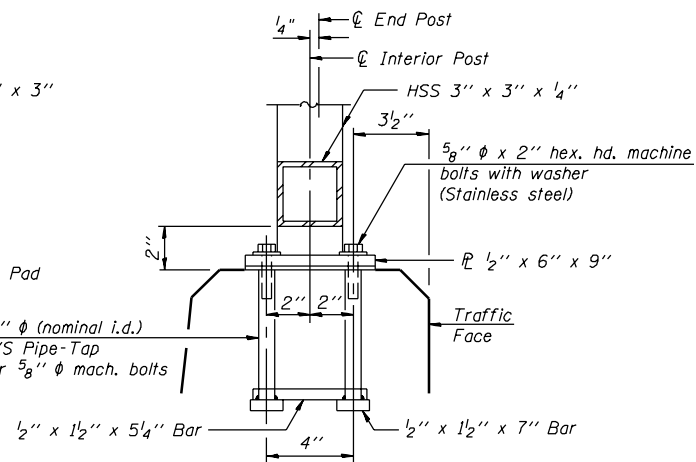
### HANDRAIL DETAIL



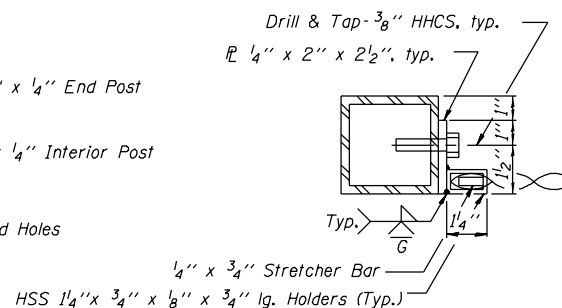
SECTION E-E  
(At Expansion Joint)



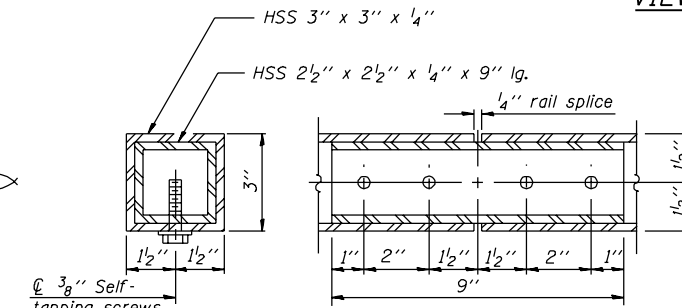
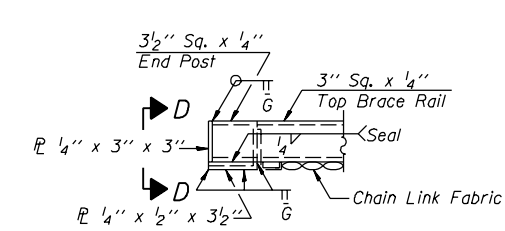
*In lieu of the cast-in-place anchor device shown, the Contractor has the option of drilling and epoxy grouting 5/8"  $\phi$  anchor rods. Embedment shall be according to the manufacturer's specifications.*



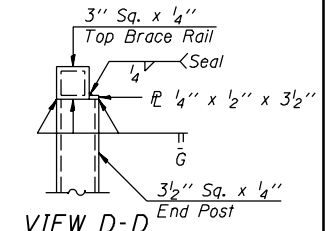
BASE P



SECTION B-B

RAIL SPLICE

VIEW C-C



VIEW D-D

Item	Unit	Quantity
Bridge Fence Railing	Foot	

DESIGNED -	-	200
CHECKED -	EXAMINED	
	ENGINEER OF BRIDGE DESIGN	
DRAWN -	PASSED	
	ENGINEER OF BRIDGES AND STRUCTURES	
CHECKED -		*variable - See F

\*Variable - See Plans  
(10'-0" Maximum Post Spacing)

R-32

10-22-04



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	100% DATE	SHEET NO.
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		SHEETS

Contract # NOTES

Railing shall be according to Section 509 of the Standard Specifications, except as noted, and will be paid for at the Contract Unit Price per foot for Bridge Fence Railing (Sidewalk) and Parapet Railing.

The 9 gauge fabric ties shall be according to Article 1006.27(d) of the Standard Specifications.

Installation of the chain link fabric shall be according to Section 664 of the Standard Specifications.

Hollow structural sections shall conform to the requirements of ASTM designation A 500, Grade B, structural steel tubing.

All other steel shapes and plates shall conform to the requirements of AASHTO M 270 Grade 36.

The chain link fabric shall be placed along pedestrian side as shown on Section A-A.

Stretcher bars shall be used at each end of fabric.

A minimum of one complete turn is required at ends of all fabric ties.

If the option of drilling and epoxy grouting the anchor rods is chosen, the Contractor shall use the capsule or the adhesive cartridge type anchor rods that have been previously tested and given a prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures. The capsule or the adhesive cartridge shall be sealed with pre-measured amounts of the adhesive chemical.

Space reinforcement to miss anchor rods.

The designer should add the appropriate note as applicable.

A. When railing is galvanized:

All posts, railing, splices, anchor devices, and plates shall be galvanized after shop fabrication according to AASHTO M 111 and ASTM A 385. All bolts, nuts, washers, and anchor rods shall be galvanized according to AASHTO M 232 except stainless steel bolts as noted.

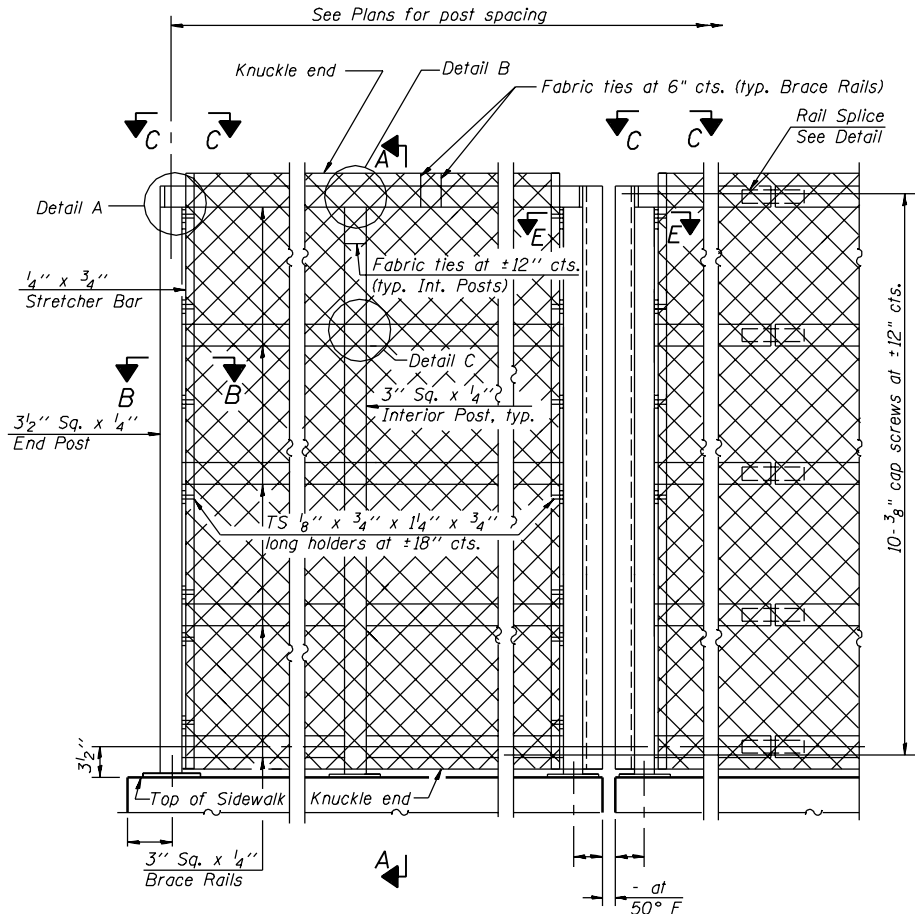
Vent holes for galvanizing shall be placed in the posts and rails at locations that will not allow the accumulation of moisture in the members.

The chain link fabric shall conform to the requirements of Article 1006.27(a)(1)a, b or c of the Standard Specifications.

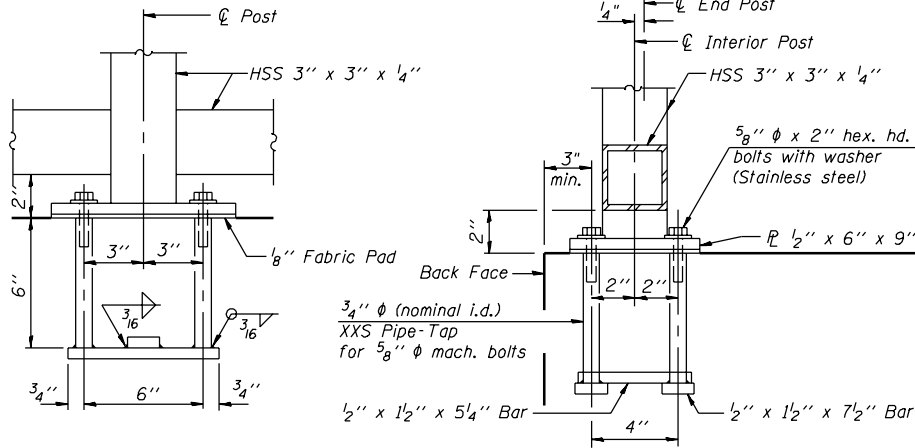
B. When railing is painted:

All post, railing, splices, anchor devices, and plates shall be painted using the (List the appropriate paint system for Structural Steel). The chain link fabric shall conform to the requirements of Article 1006.27(a)(1)d of the Standard Spec's.

Only one of the above notes would appear on Contract Plans.



ELEVATION  
(Inside Face)



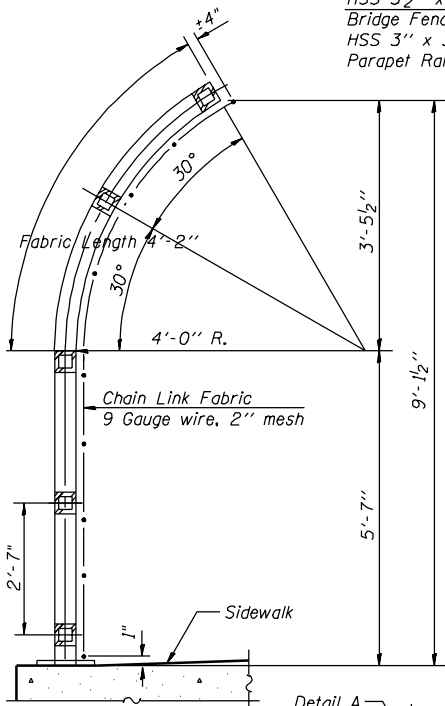
ANCHOR BOLT DETAILS

In lieu of the cast-in-place anchor device shown, the Contractor has the option of drilling and epoxy grouting 5/8\"/>

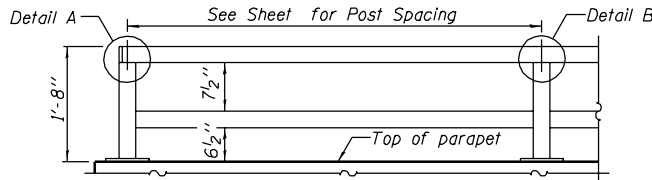
DESIGNED -	200
CHECKED -	ENGINEER OF BRIDGE DESIGN
DRAWN -	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -	

R-33

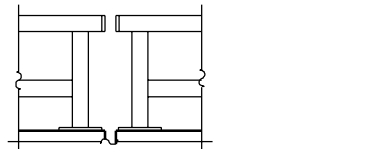
10-22-04 (10'-0\"/>



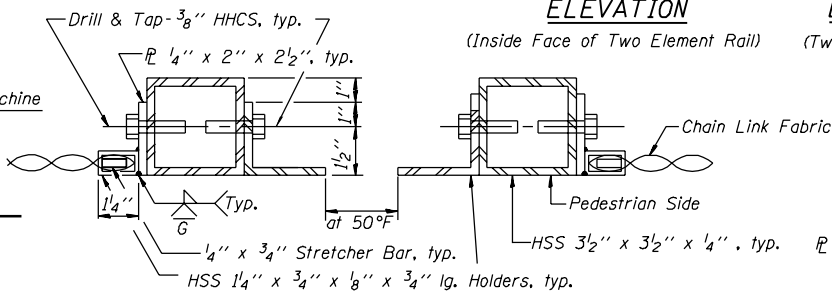
SECTION A-A



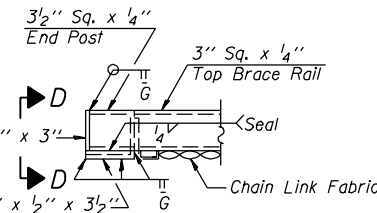
PARAPET RAILING  
ELEVATION  
(Inside Face of Two Element Rail)



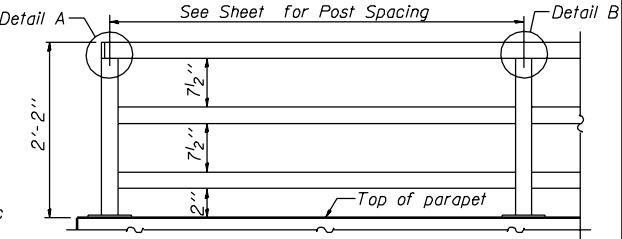
PARAPET RAILING  
ELEVATION AT EXPANSION JOINT  
(Two Element Rail Shown - Three Element Rail Similar)



SECTION E-E  
(At Expansion Joint)



VIEW C-C



PARAPET RAILING  
ELEVATION  
(Inside Face of Three Element Rail)

BILL OF MATERIAL

Item	Unit	Quantity
Bridge Fence Railing (Sidewalk)	Foot	
Parapet Railing	Foot	

BRIDGE FENCE RAILING  
SIDEWALK MOUNTED

SECTION B-B

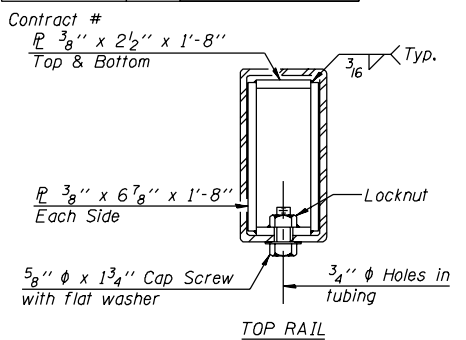
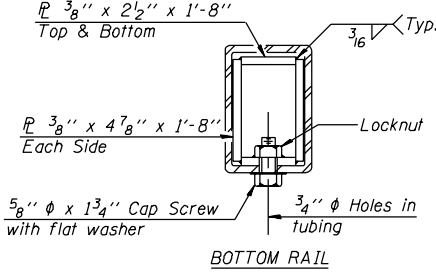
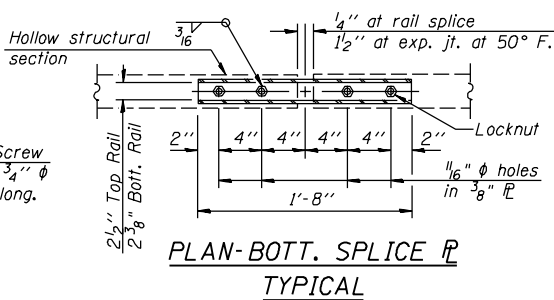
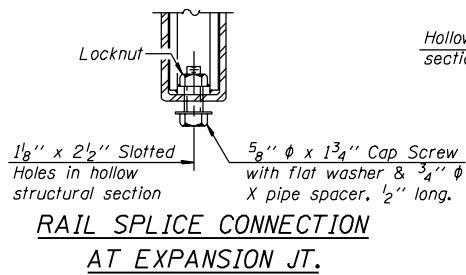
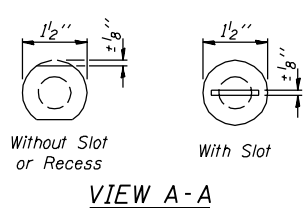
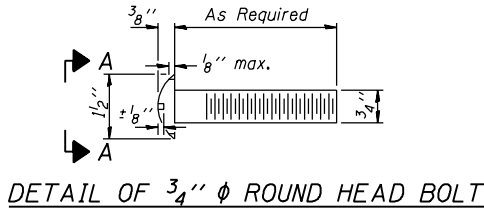
RAIL SPLICE

VIEW D-D



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT-		SHEETS



SECTIONS AT RAIL SPLICE

NOTES

Hollow structural sections shall conform to the requirements of ASTM designation A 500 Grade B Structural Steel Tubing and shall meet the longitudinal CVN requirements of 15 ft-lbs at 0° F.

All other steel shapes and plates shall conform to the requirements of AASHTO M 270 Grade 36 except posts and angles shall conform to AASHTO M 270, Grade 50.

Bolts, cap screws, and nuts shall conform to the requirements of ASTM designation A 307 except for high strength bolts, nuts and washers noted which shall conform to AASHTO M 164.

All bolts, nuts, cap screws, washers and lock washers shall be galvanized according to AASHTO M 232.

All posts, railing, rail splices, anchor devices and angles shall be galvanized after shop fabrication according to AASHTO M 111 and ASTM A 385. Galvanized rail shall not be painted.

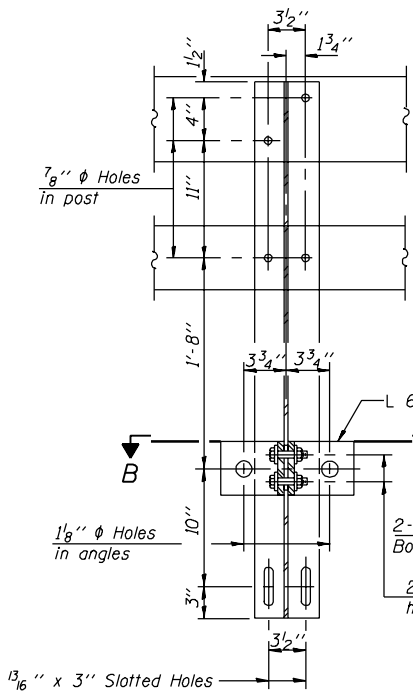
Railing shall be according to Section 509 of the Standard Specifications, except as noted, and will be paid for at the contract unit price per foot for Steel Bridge Rail, Type SM.

All field drilled holes shall be coated with an approved zinc rich paint before erection.

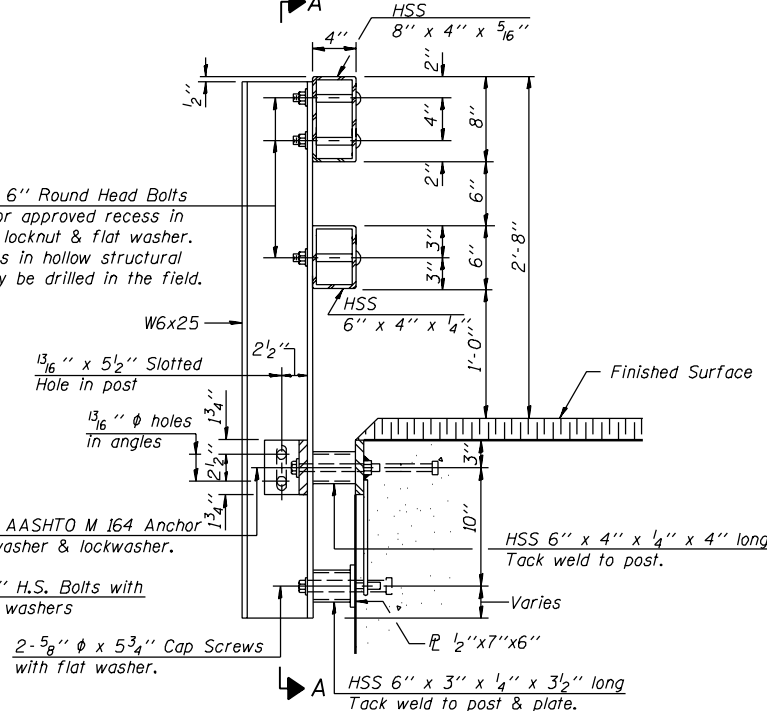
For multi-span bridges, sufficient 1/4" x 6" x 1'-2" galvanized steel shims shall be provided to align rail between adjacent spans. Cost included with Steel Bridge Rail, Type SM.

The 1/2" x 7" x 6" plates that come in contact with concrete shall receive two coats of asphalt paint conforming to Section 1060.07 Type II or place 1/8" fabric bearing pads between the plates and concrete.

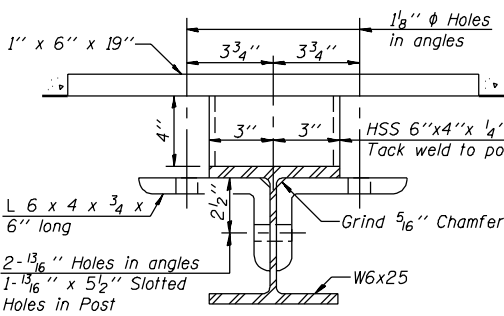
The 3/4" high strength bolts used to connect the 6 x 4 x 3/4 angles to the post shall be tightened according to Article 505.04(f)(2) of the Standard Specifications. The 1" high strength bolts connecting the angles to the concrete shall be tightened to a snug fit and given an additional 1/8 turn. The 5/8" cap screws in bottom of posts shall be tightened to a snug fit only.



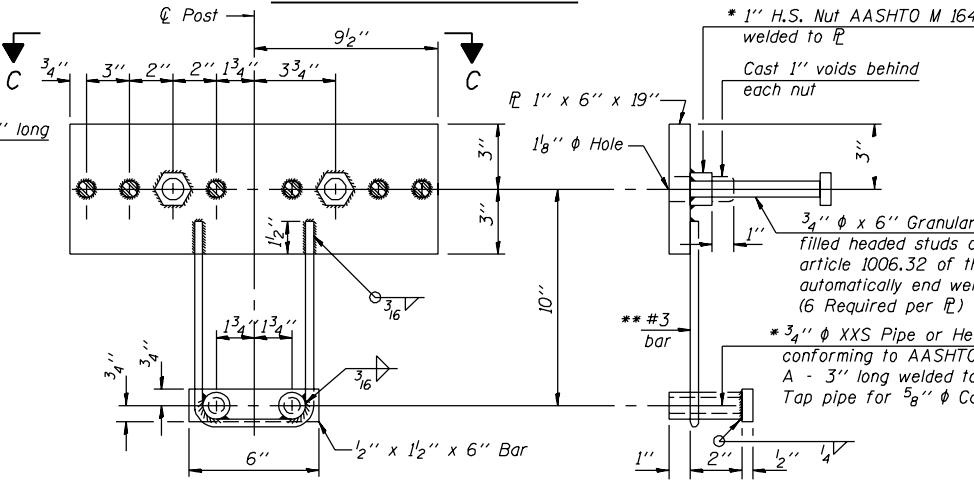
SECTION A-A



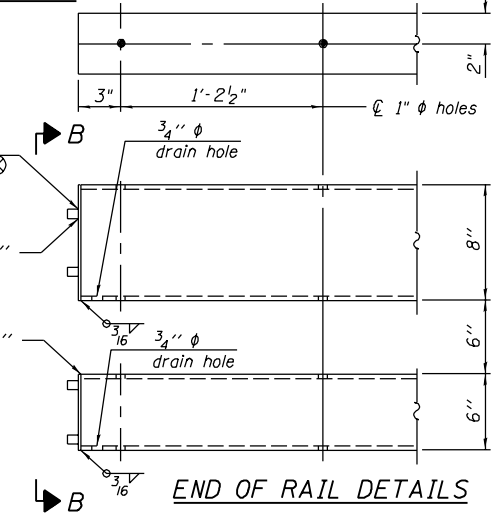
SECTION AT RAIL POST



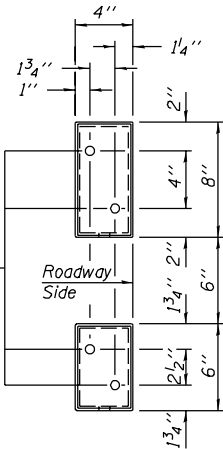
SECTION B-B



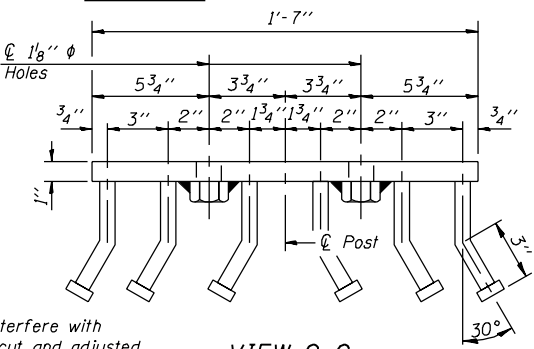
ANCHOR DEVICE



END OF RAIL DETAILS



VIEW B-B



VIEW C-C

BILL OF MATERIAL

Item	Unit	Quantity
Steel Bridge Rail, Type SM	Foot	

TYPE SM  
STEEL BRIDGE RAIL SIDE MOUNTED

DESIGNED -	200
CHECKED -	EXAMINED
DRAWN -	PASSED
CHECKED -	ENGINEER OF BRIDGE DESIGN
	ENGINEER OF BRIDGES AND STRUCTURES

\* Threaded areas shall be plugged or blocked off during casting of beam. Galvanized after fabrication.

\*\* Whenever the lower insert assemblies interfere with strand locations, the #3 bars shall be cut and adjusted in order to allow raising or lowering of the lower inserts. Maximum adjustment not to exceed 1/2".



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-		
-	-	-		
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -

- SHEETS

Technical drawing of a bridge deck cross-section showing reinforcement details. The drawing includes dimensions for the deck width (1'-7"), parapet height (1'-2"), and various reinforcement bar specifications. Key features include:

- #4  $d_3(E)$  bars at 11" cts. Outside Face
- #5  $d_1(E)$  bars at 11" cts. Inside Face
- #6  $b_1(E)$  bars at  $\pm 12$ " cts. Top of slab over piers
- #5  $b(E)$  bars equally spaced at  $\pm 12$ " cts. Top of slab
- #6  $b_1(E)$  bars spaced as shown in cross section. Bottom of slab
- #6  $b_1(E)$  bars at  $\pm 12$ " cts. Top of slab over pier
- #6  $a_2(E)$  bars at cts. Top (Lap with alternate  $a(E)$  bars) end to end deck
- 2- #6  $b_1(E)$  bars Top of slab
- 3 x - #5  $b(E)$  bars Top of slab
- 2- #6  $b_1(E)$  bars Top of slab

The drawing also shows the 'Back of Abut.' and 'Aluminum sheeted construction joints in base of parapet'.

Technical drawing of a bridge deck cross-section, showing two views: a side elevation and a longitudinal section.

**Side Elevation (Left):**

- Overall width: 1'-7"
- Parapet width: 1'-7"
- Deck thickness: 2"
- Reinforcement bars:  $a_1(E)$ ,  $b_1(E)$ ,  $a_2(E)$ ,  $b_2(E)$ ,  $a_3(E)$ ,  $b_3(E)$ ,  $d_1(E)$ ,  $d_2(E)$ ,  $d_3(E)$
- Longitudinal construction joints

**Longitudinal Section (Right):**

- Overall width: 1'-7"
- Parapet width: 1'-7"
- Deck thickness: 2"
- Reinforcement bars:  $a_2(E)$ ,  $b_2(E)$ ,  $a_3(E)$ ,  $b_3(E)$ ,  $d_1(E)$ ,  $d_2(E)$ ,  $d_3(E)$
- Total Drop = 2'-10"
- Reinforcement bars: - #  $b_2(E)$  bars at cts. typ. btwn. beams

Labels: *a. to a. deck*, *face to face parapets*, *slope per ft.*, *Longitudinal construction joints*, *spaces at*

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

10-22-04

(Looking )

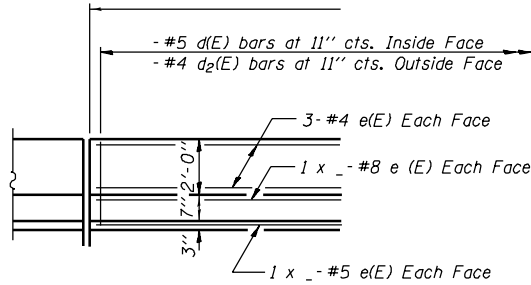


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

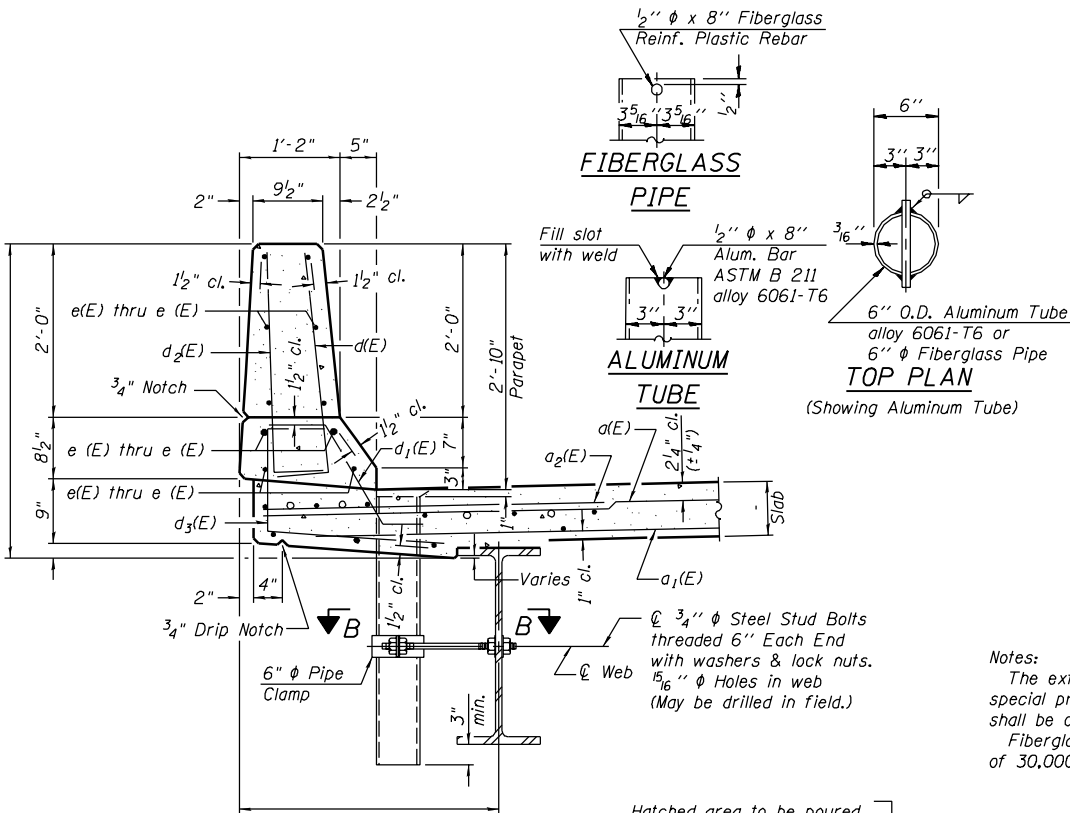
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

SHEET NO. -  
SHEETS

Contract #



INSIDE ELEVATION OF PARAPET



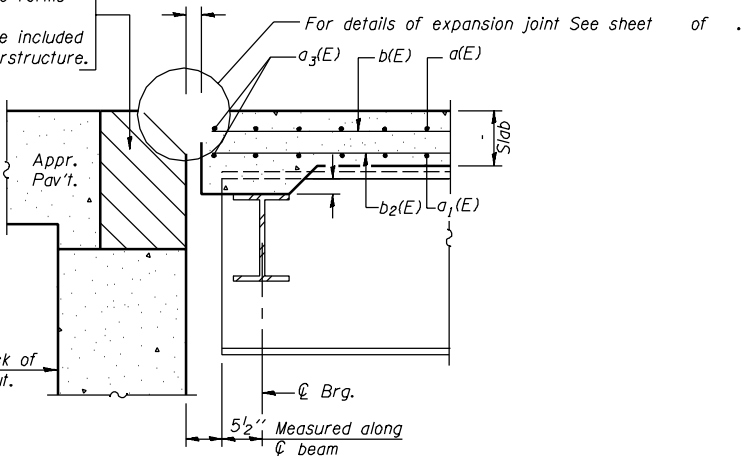
Non-staining gray one component non-sag elastomeric gun grade polyurethane sealant meeting the requirements of ASTM C-920, Type S, Grade NS, Class 25, Use T.

5/8"  $\phi$  Backer Rod  
1/2" Preformed Self-Expanding Cork Joint Filler according to Article 1051.07 of the Std. Spec. Cost included with Concrete Superstructure.  
Const. Jts. at Piers  
8" Aluminum sheet ASTM B 209 alloy 3003-H14. Cost included with Concrete Superstructure

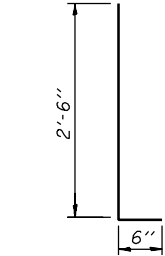
PARAPET JOINT DETAILS

Notes:  
The exterior surfaces of the floor drains shall be painted with the finish coat as specified in the special provisions for Cleaning and Painting New Metal Structures. The exterior surfaces of the drains shall be cleaned according to Steel Structures Painting Council's Spec. SSPC-SP1 prior to painting. Fiberglass pipe shall conform to ASTM D 2996, with short-time rupture strength hoop tensile stress of 30,000 p.s.i. minimum.

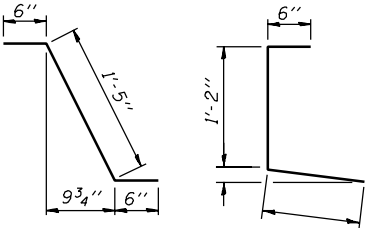
Hatched area to be poured after superstructure forms have been removed. Quantity of concrete included with Concrete Superstructure.



SECTION A-A



BARS d(E) & d2(E)



BAR d1(E) BAR d3(E)

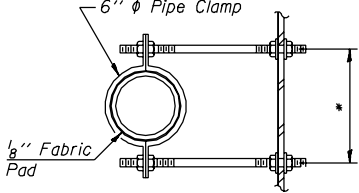
SUPERSTRUCTURE  
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
d(E)				
a1(E)				
a2(E)	#6	4'-6"		
b(E)	#5			
b1(E)	#6			
b2(E)				
b3(E)				
b4(E)				
d(E)	#5	3'-0"		
d1(E)	#5	2'-5"		
d2(E)	#4	3'-0"		
d3(E)	#4			
e(E)	#4			
e1(E)	#4			
e2(E)				
e3(E)				
Reinforcement Bars, Epoxy Coated	Pound			
Concrete Superstructure	Cu. Yds.			

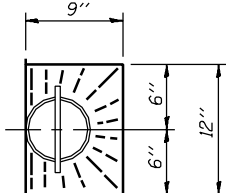
Reinforcement bars designated (E) shall be epoxy coated.  
Bars indicated thus 1 x - #5 etc. indicates 1 line of bars with lengths per line.

\* Dimension as required by Pipe Clamp

SECTION THRU PARAPET



SECTION B-B



TOP PLAN

DESIGNED -	200
CHECKED -	EXAMINED
DRAWN -	ENGINEER OF BRIDGE DESIGN
CHECKED -	PASSED
	ENGINEER OF BRIDGES AND STRUCTURES

S-1-D

10-22-04

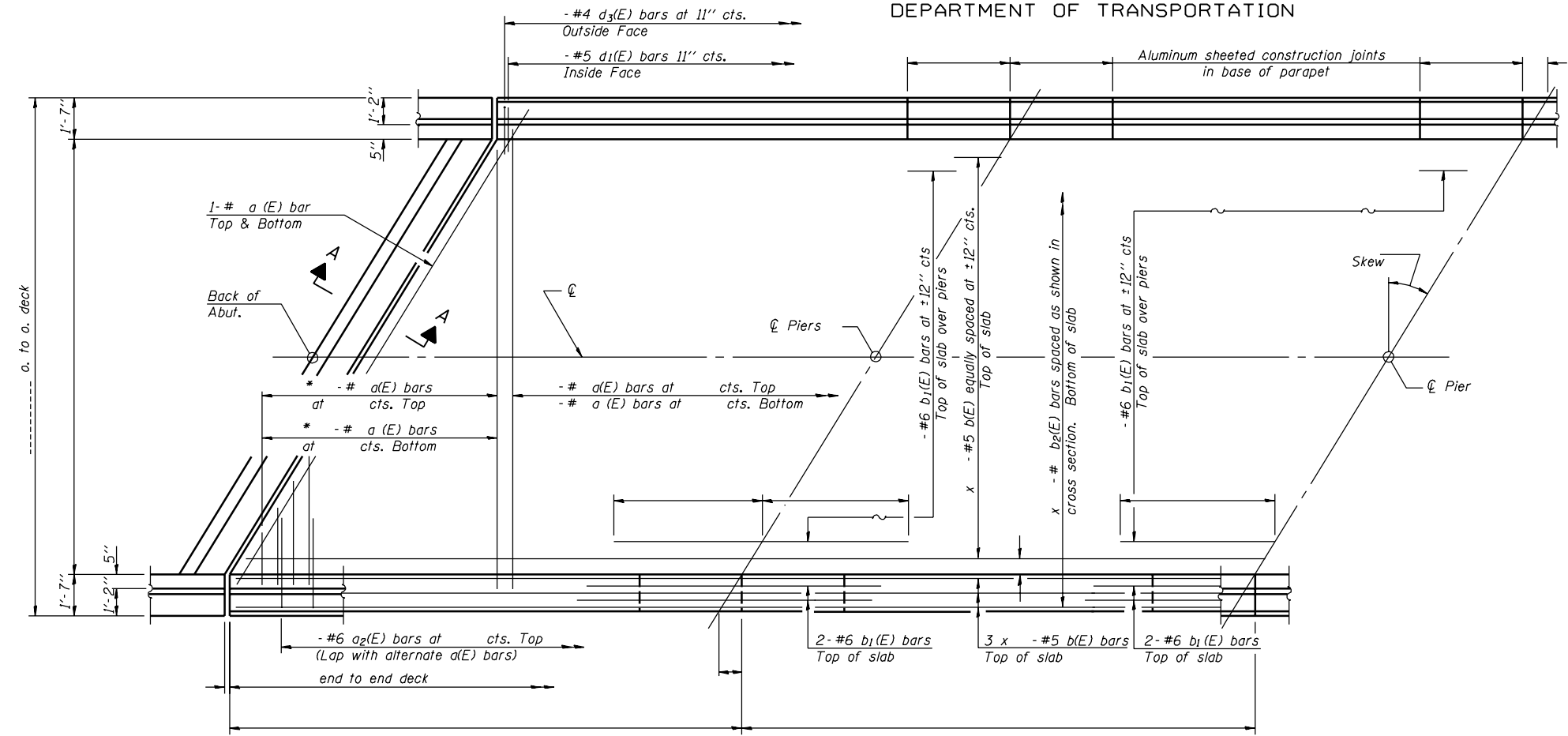


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT -	

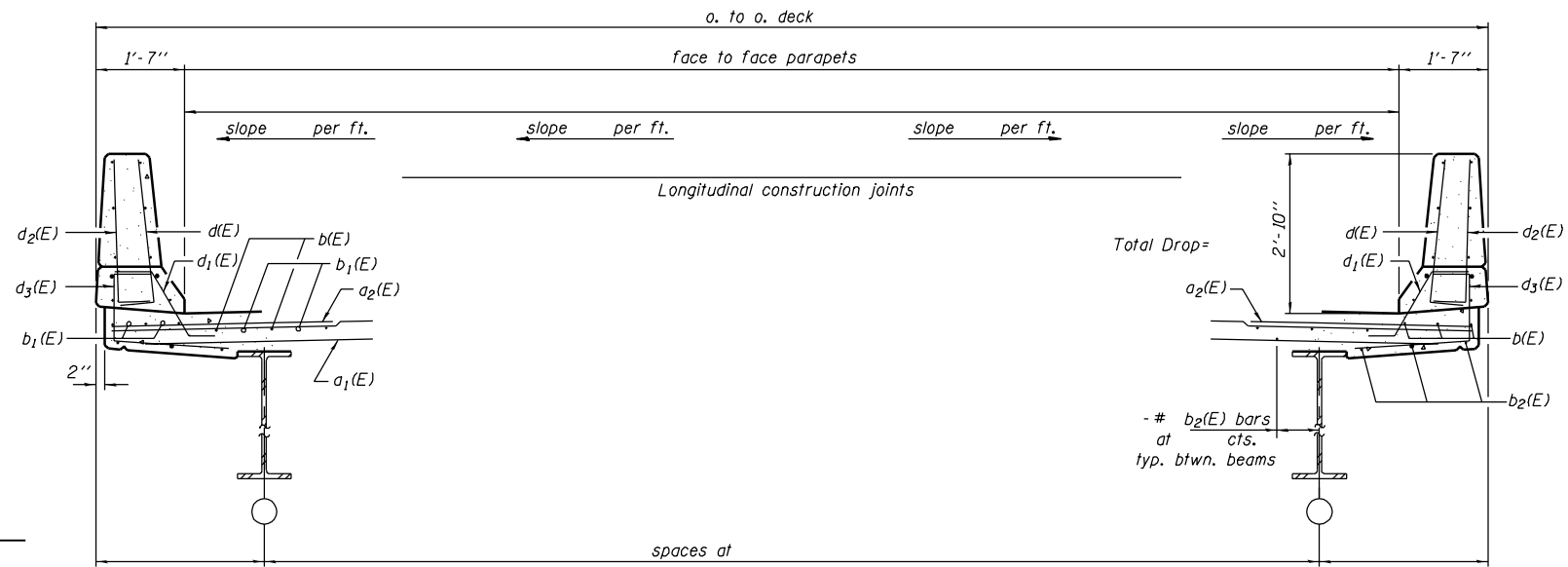
SHEET NO. -  
- SHEETS

Contract #



\* Order a(E) & a1(E) bars full length.  
Cut to fit skew and use remainder  
of bars in opposite end.

HALF PLAN



Notes:  
See Sheet of for superstructure details  
and Bill of Material.  
Reinforcement bars designated (E) shall be  
epoxy coated.  
Bars indicated thus 20 x 3-#5 etc. indicates  
20 lines of bars with 3 lengths per line.  
See Sheet of for parapet reinforcement.

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

EXAMINED  
ENGINEER OF BRIDGE DESIGN  
PASSED  
ENGINEER OF BRIDGES AND STRUCTURES

S-1-L(>15°) 10-22-04

CROSS SECTION  
(Looking )



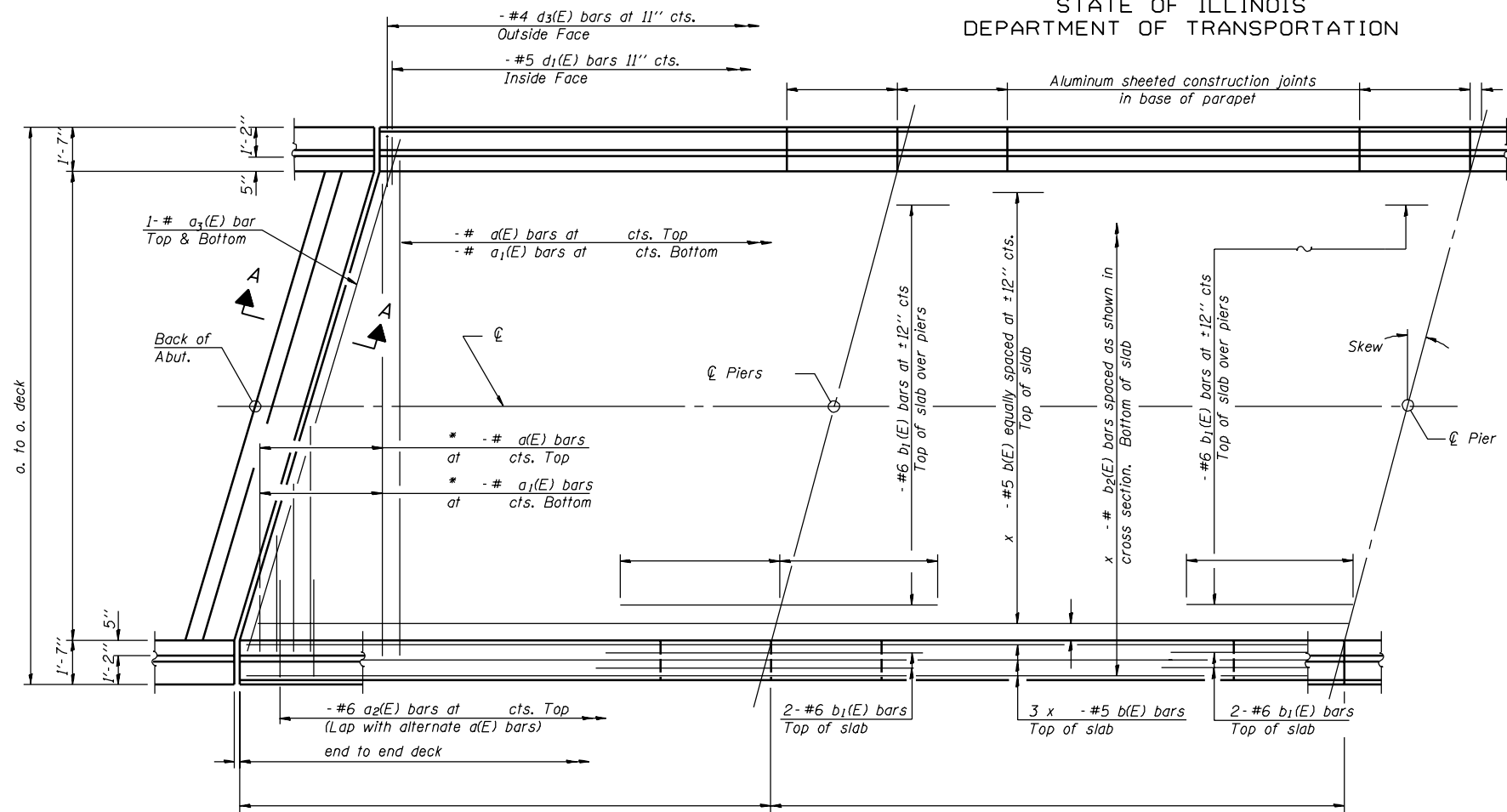
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

SHEET NO. -

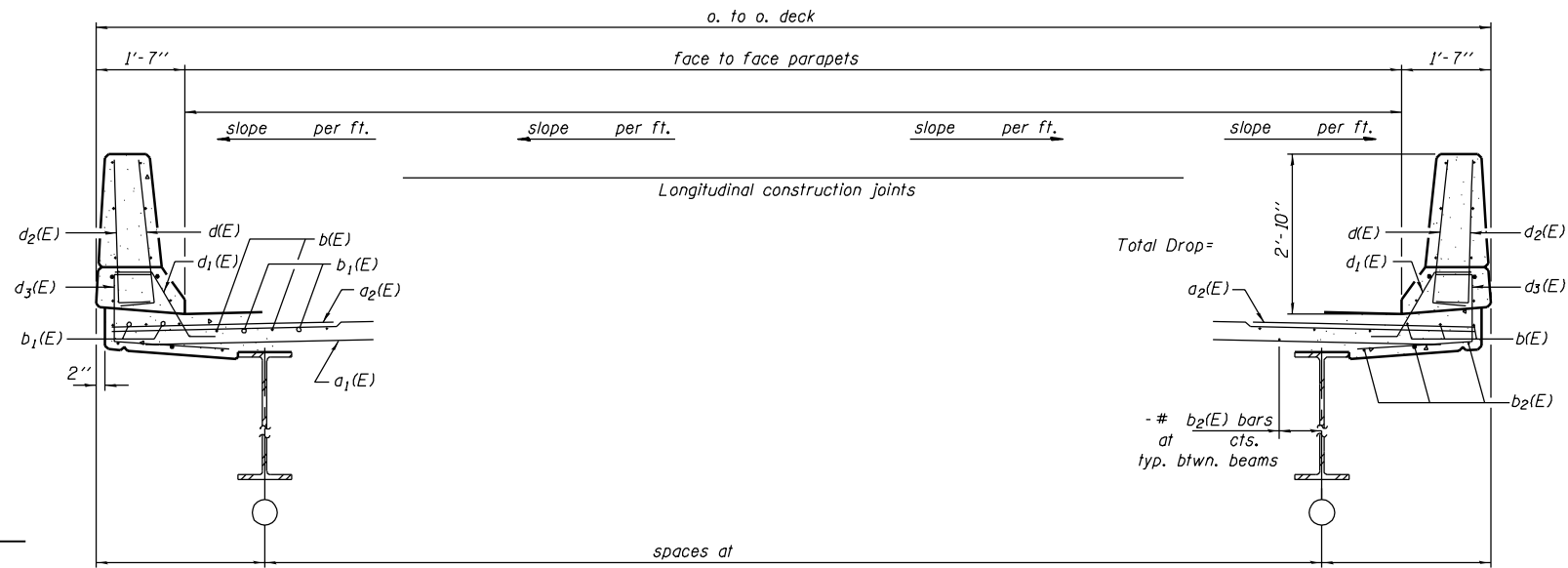
- SHEETS

Contract #



HALF PLAN

\* Order a(E) & a1(E) bars full length.  
Cut to fit skew and use remainder  
of bars in opposite end.



CROSS SECTION  
(Looking )

Notes:  
See Sheet of for superstructure details  
and Bill of Material.  
Reinforcement bars designated (E) shall be  
epoxy coated.  
Bars indicated thus 20 x 3-#5 etc. indicates  
20 lines of bars with 3 lengths per line.  
See Sheet of for parapet reinforcement.

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

S-1-L(15°)

10-22-04

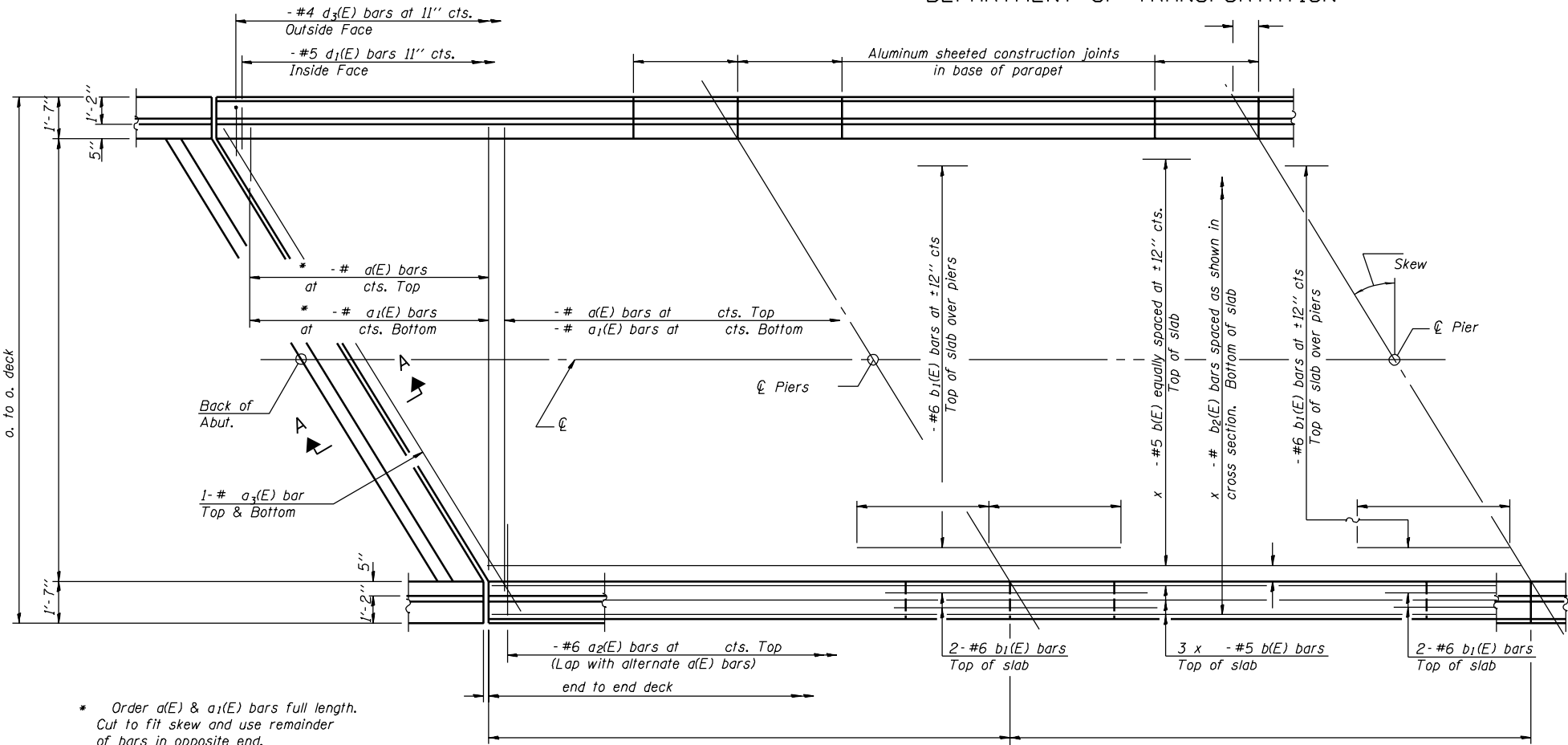


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

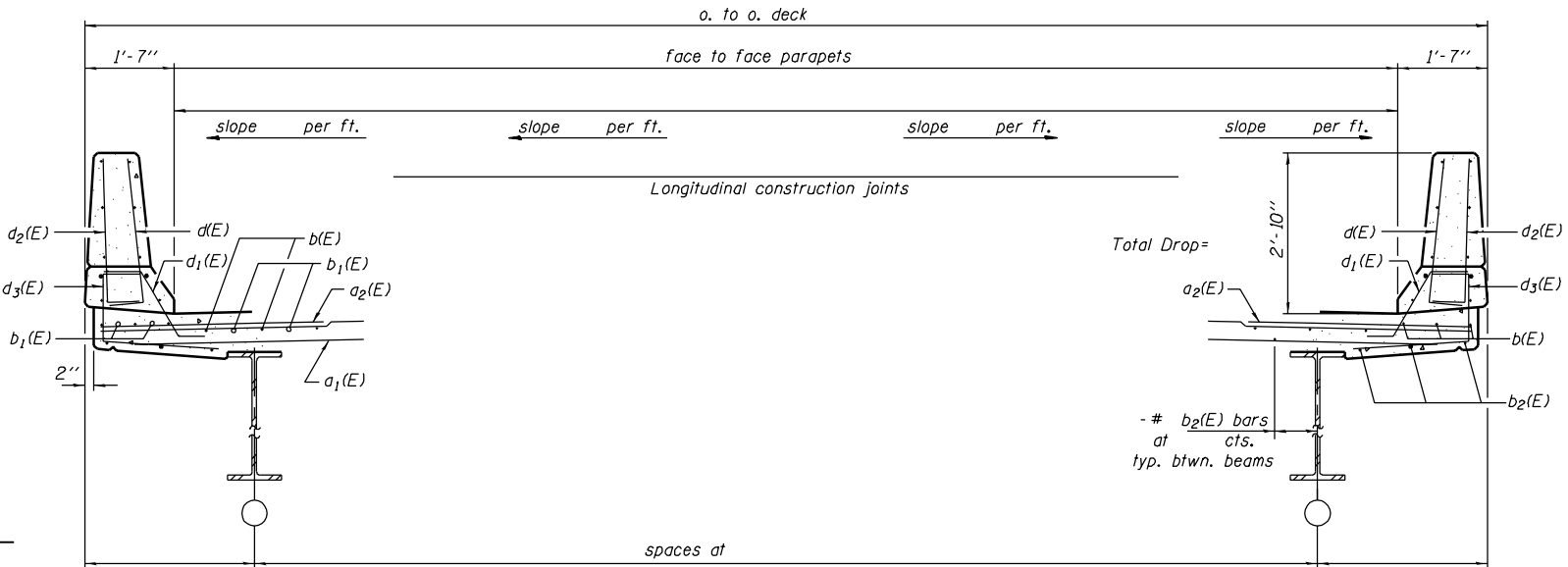
SHEET NO. -  
- SHEETS

Contract #



\* Order  $a(E)$  &  $a_1(E)$  bars full length.  
Cut to fit skew and use remainder  
of bars in opposite end.

HALF PLAN



Notes:  
See Sheet of for superstructure details  
and Bill of Material.  
Reinforcement bars designated (E) shall be  
epoxy coated.  
Bars indicated thus 20 x 3- #5 etc. indicates  
20 lines of bars with 3 lengths per line.  
See Sheet of for parapet reinforcement.

DESIGNED -	-
CHECKED -	-
DRAWN -	-
CHECKED -	-

EXAMINED	200
PASSED	ENGINEER OF BRIDGE DESIGN
	ENGINEER OF BRIDGES AND STRUCTURES

S-1-R(>15°) 10-22-04

CROSS SECTION  
(Looking )



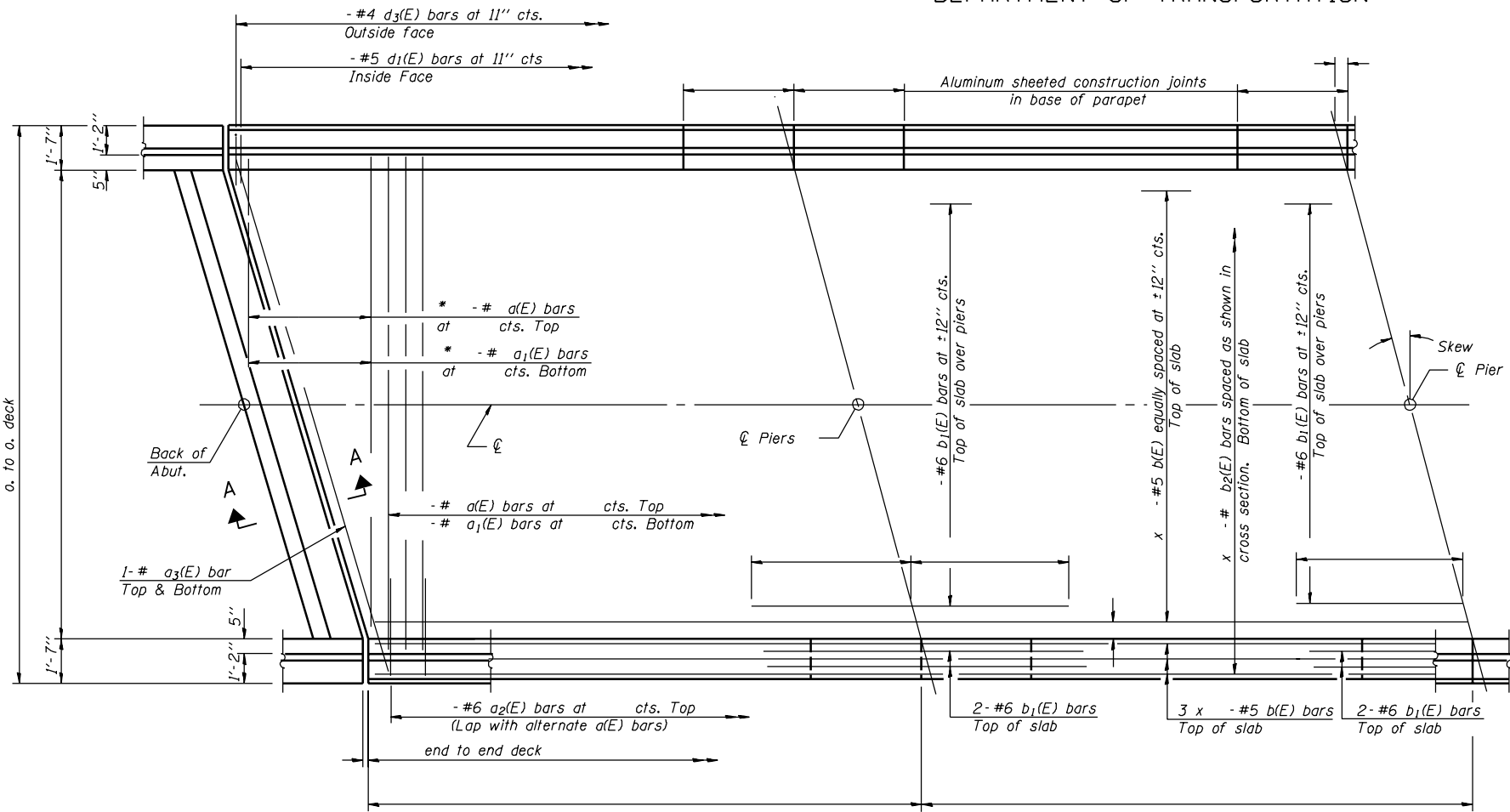
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT -	

SHEET NO. -

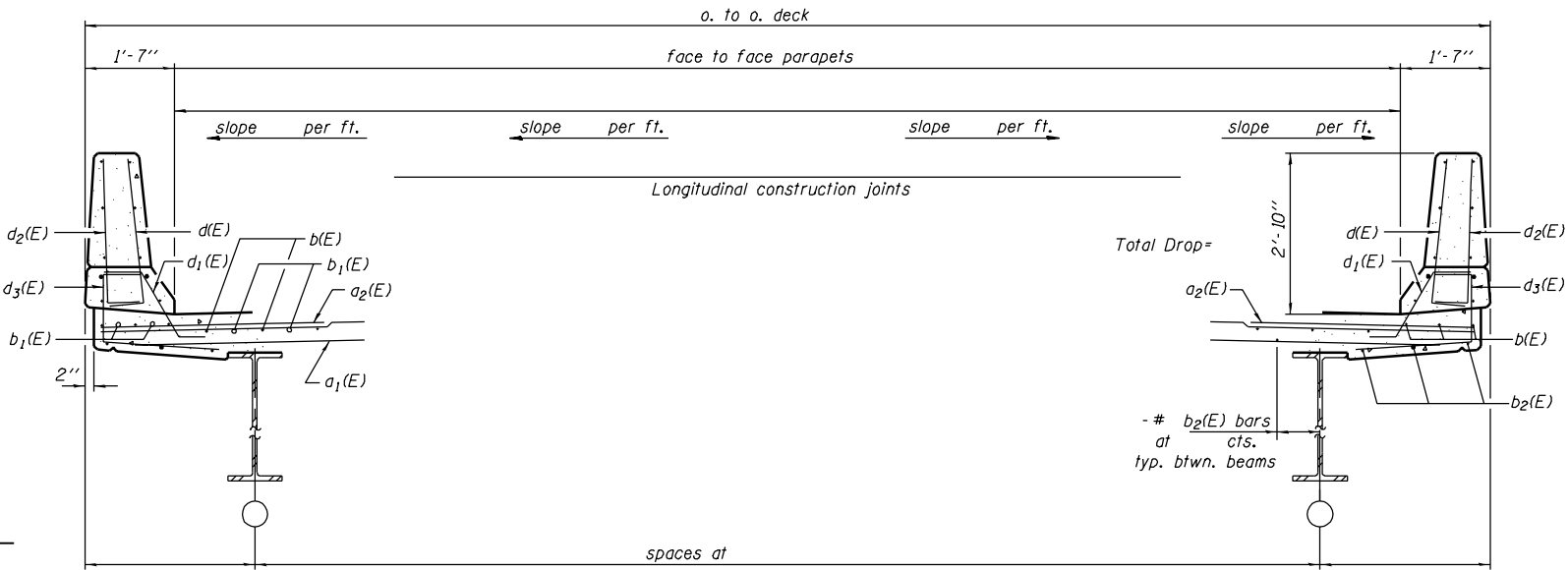
- SHEETS

Contract #



HALF PLAN

\* Order a(E) & a1(E) bars full length.  
Cut to fit skew and use remainder  
of bars in opposite end.



CROSS SECTION  
(Looking )

Notes:  
See Sheet of for superstructure details  
and Bill of Material.  
Reinforcement bars designated (E) shall be  
epoxy coated.  
Bars indicated thus 20 x 3-#5 etc. indicates  
20 lines of bars with 3 lengths per line.  
See Sheet of for parapet reinforcement.

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

EXAMINED	200
PASSED	ENGINEER OF BRIDGE DESIGN
	ENGINEER OF BRIDGES AND STRUCTURES

S-1-R(15°)

10-22-04



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-		
-	-	-		
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -

- SHEETS

Technical drawing of a bridge deck cross-section showing reinforcement details. The drawing includes dimensions for the deck width (1'-7"), parapet height (1'-2"), and various reinforcement bars (#4, #5, #6) and their spacing (11", 12", 12"). It also shows the location of the "Back of Abut." and "Pier" and the "Aluminum sheeted construction Joints in base of parapet".

Key dimensions and labels:

- Deck width: 1'-7"
- Parapet height: 1'-2"
- Reinforcement bars: #4  $a_3(E)$  bars at 11" cts., #5  $d_1(E)$  bars at 11" cts., #6  $a_2(E)$  bars at cts. Top, #6  $b_1(E)$  bars at cts. Top, #6  $b_2(E)$  bars spaced as shown in cross section, Bottom of slab, #6  $b_1(E)$  bars at  $\pm 12$ " cts., Top of slab over pier.
- Labels: Back of Abut., Pier, Aluminum sheeted construction Joints in base of parapet.

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

CROSS SECTION  
(Looking )



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-		
-				
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -  
- SHEETS

- #5 d(E) bars at 11" cts. Inside Face  
- #4 d<sub>2</sub>(E) bars at 11" cts. Outside Face

3- #4 e(E) Each Face

7'-0"

3"

1 x - #8 e (E) Each Face

1 x - #5 e(E) Each Face

3- #4 e (E) E.F.

1- #8 e (E) E.F.

1- #5 e (E) E.F.

3- #4 e(E) Each Face

1 x - #8 e (E) Each Face

1 x - #5 e (E) Each Face

Aluminum sheeted const.  
Joints in base of parapet

1/2"  $\phi$  x 8" Fiberglass

C/Pier

$\frac{1}{2}'' \phi \times 8''$  Fiberglass  
Reinf. Plastic Rebar

35% 35% 35%

3/16" 3/16" 3/16"

1/2"

FIBERGLASS  
PIPE

Fill slot  
with weld

$\frac{1}{2}'' \text{ } \phi \times 8''$   
Alum. Bar  
ASTM B 211  
alloy 6061-T6

3" 3"

(Showing Aluminum Tube)

Non-staining gray one component non-sag elastomeric gun grade polyurethane sealant meeting the requirements of ASTM C-920, Type S, Grade NS, Class 25. Use T.

5/8"  $\phi$  Backer R

1/4" 1/5" 1/4" 1/5" 2'-0" Parapet Jts. 10" Const. Jt. (Optional) Const. Jt. (Mandatory)

1/2" Preformed Self-Expanding Cork Joint Filler according to Article 1051. of the Std. Spec. Cost included with Concrete Superstructure.

Const. Jts. at Piers 1/8" Aluminum sheet ASTM B 209 alloy 3003-H14. Cost included with Concrete Superstructure

[illegible]

**FIBERGLASS PIPE**

Fill slot with weld

1/2"  $\phi$  Alum. ASTM alloy 6

3" 3"

**ALUMINUM TUBE**

2'-0" 8 1/2" 9" 2" 4" 3/4" Drip Notch

1'-2" 5" 2 1/2" 9 1/2" 2" 1 1/2" cl. 1 1/2" cl. e(E) thru e(E) d2(E) d3(E) 3/4" Notch e(E) thru e(E) e(E) thru e(E) d1(E) 7" 1 1/2" cl. 2'-10" Parapet 2'-0" 1" 1" cl. 1 1/2" cl. 2 1/4" cl. (+ 1/4") a2(E) a1(E) 1" cl. Varies cl. 6"  $\phi$  Pipe Clamp 3" min. 3/4"  $\phi$  Steel Stud Bolts Threaded 6" Each End with washers & lock nuts. 1 5/16"  $\phi$  Holes in web (May be drilled in field.) Hatched area to be covered by parapet cap

\* Dimension as required  
by Pipe Clamp

Diagram illustrating a 6"  $\phi$  Pipe Clamp with 1/8" Fabric Pads. The clamp is shown in cross-section, highlighting the internal structure and the placement of the fabric pads.

Technical drawing of a rectangular plate with a circular hole. The plate is 9 inches wide and 12 inches high. The hole has a diameter of 6 inches and is centered 6 inches from the top and bottom edges. The hole is surrounded by a pattern of radiating lines.

removed.  
concrete included  
the Superstructure.

For details of expansion joint See sheet of

Appr.  
Pav't.

Slab

$a_3(E)$   $b(E)$   $a(E)$

$b_2(E)$   $a_1(E)$

Back of  
Abut.

C Brg.

5 1/2" Measured along C beam

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -		ENGINEER OF BRIDGE DESIGN
CHECKED -	PASSED	
		ENGINEER OF BRIDGES AND STRUCTURES

S-2-D

10-22-04

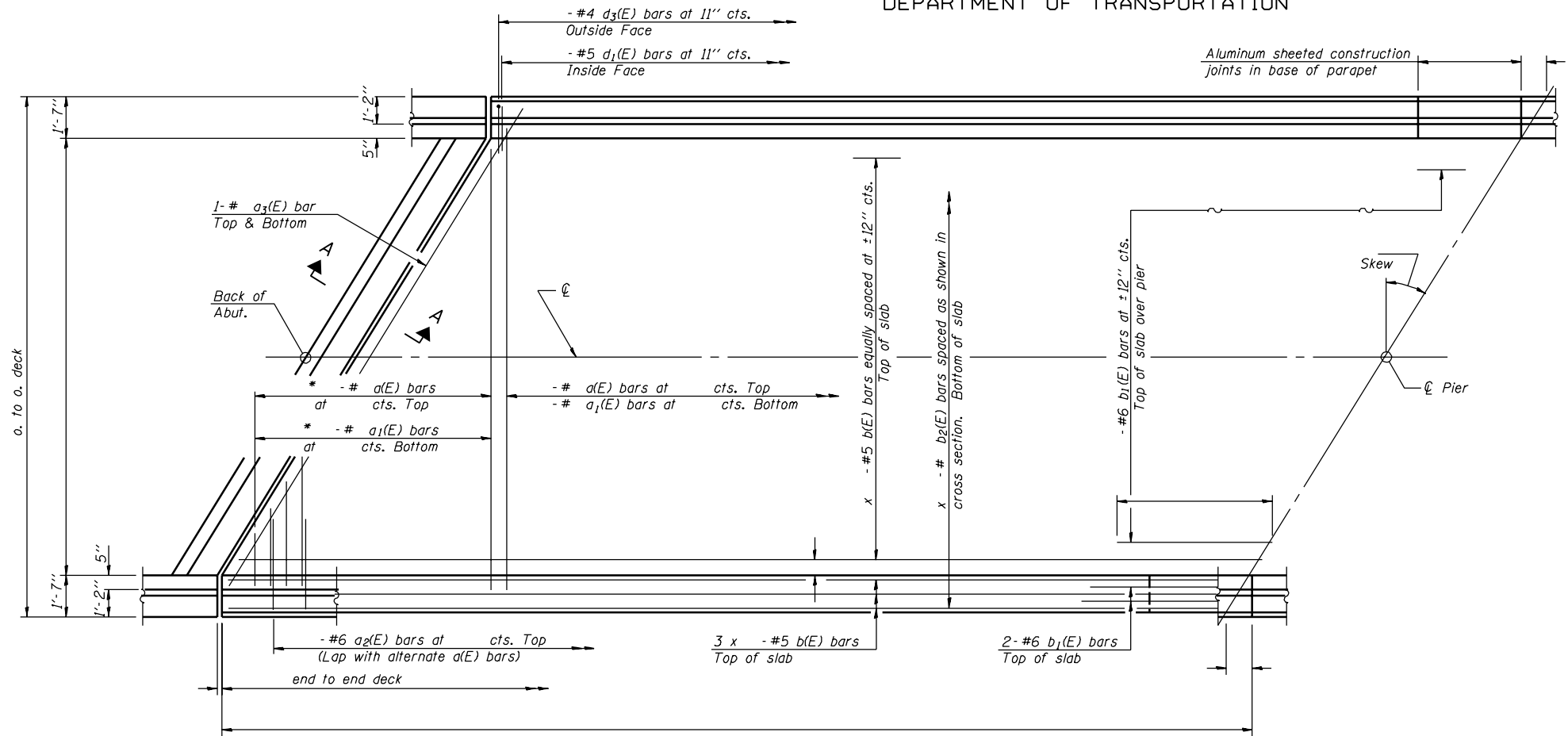


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

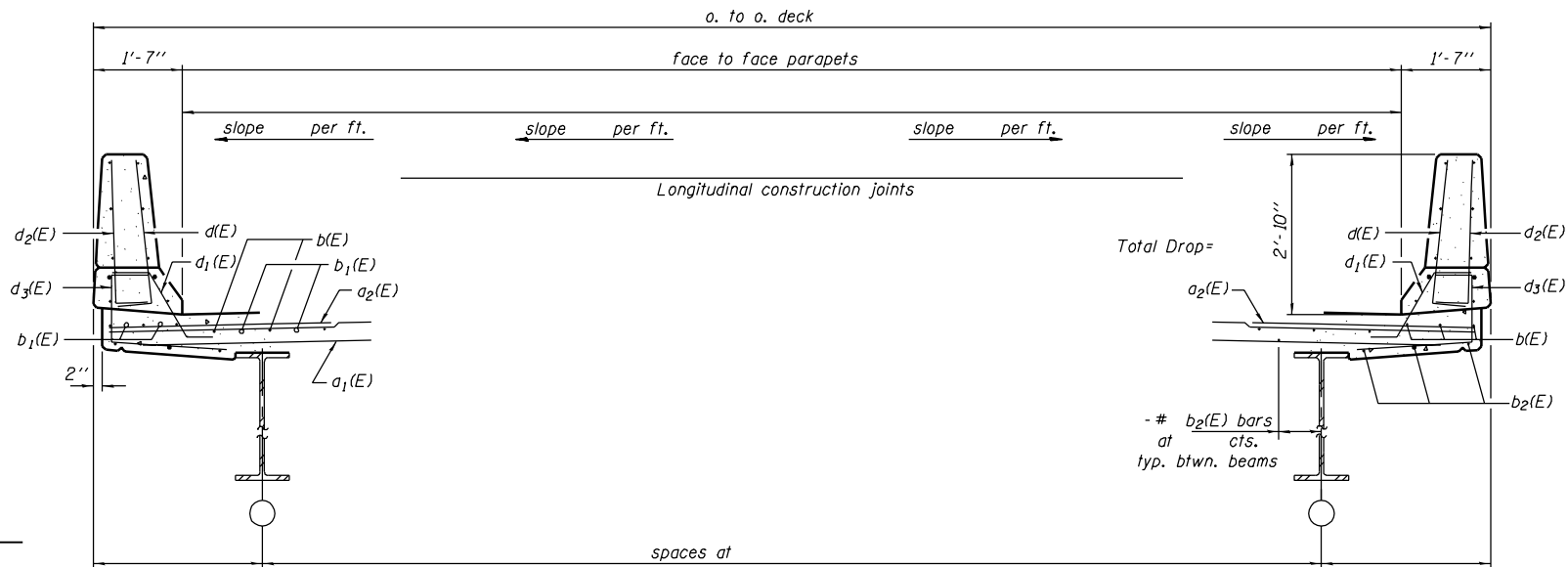
SHEET NO. -  
- SHEETS

Contract #



\* Order a(E) & a1(E) bars full length.  
Cut to fit skew and use remainder  
of bars in opposite end.

HALF PLAN



Notes:  
See Sheet of for superstructure details  
and Bill of Material.  
Reinforcement bars designated (E) shall be  
epoxy coated.  
Bars indicated thus 20 x 3-#5 etc. indicates  
20 lines of bars with 3 lengths per line.  
See Sheet of for parapet reinforcement.

DESIGNED -	-
CHECKED -	-
DRAWN -	-
CHECKED -	-

EXAMINED	200
PASSED	ENGINEER OF BRIDGE DESIGN
	ENGINEER OF BRIDGES AND STRUCTURES

S-2-L(>30°)

10-22-04

CROSS SECTION  
(Looking )



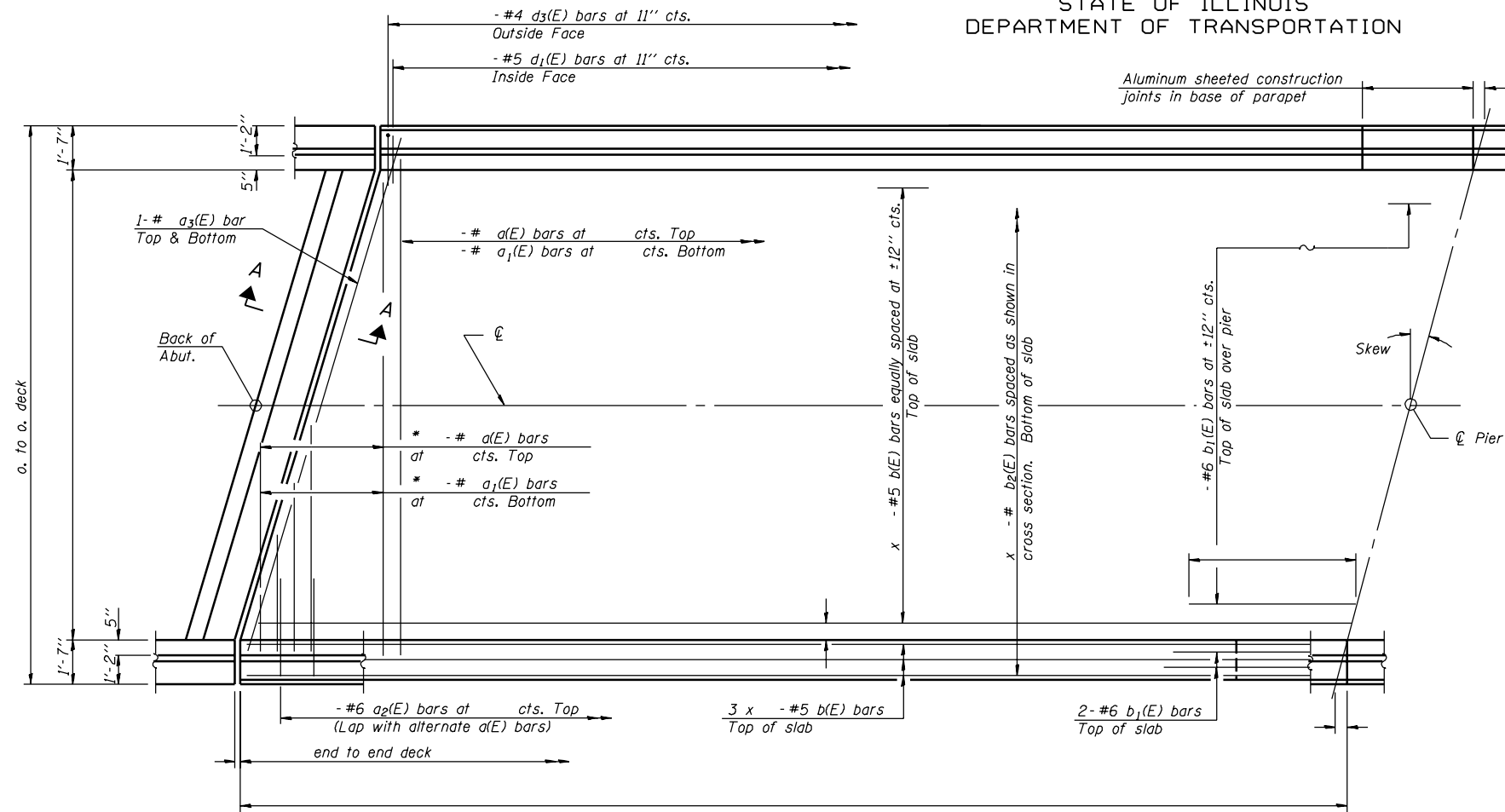
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT -	

SHEET NO. -

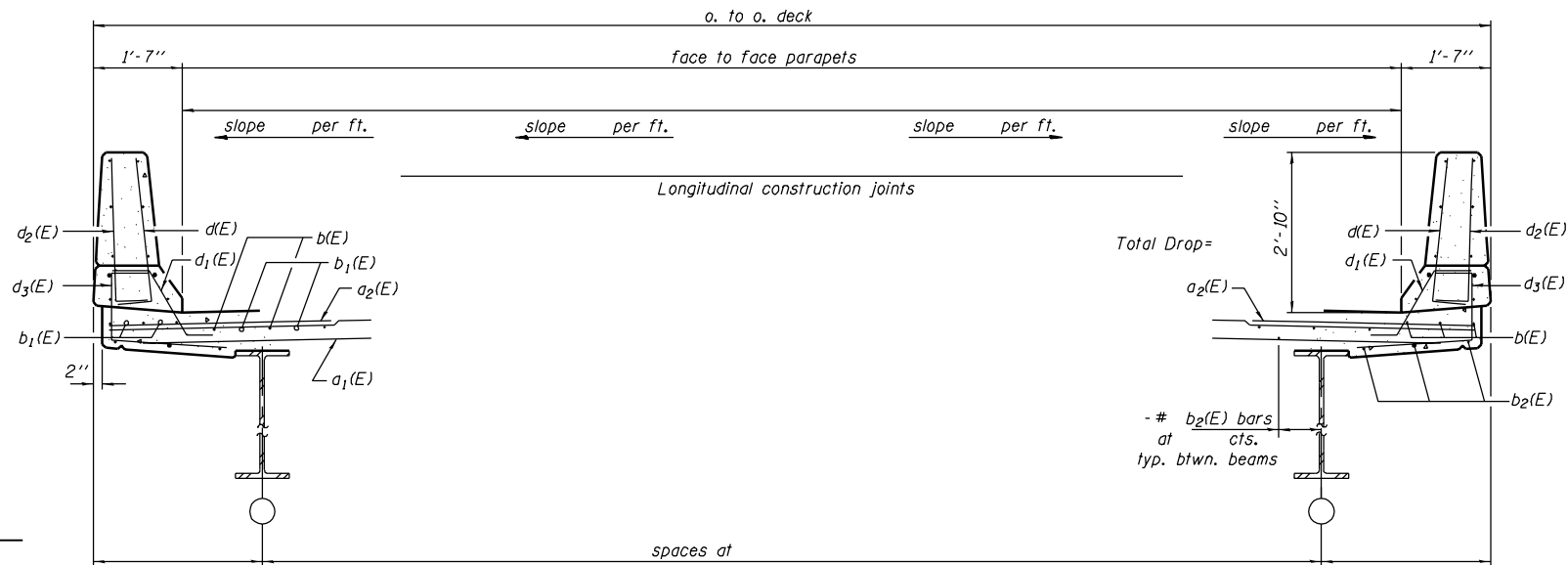
- SHEETS

Contract #



HALF PLAN

\* Order d(E) & a1(E) bars full length.  
Cut to fit skew and use remainder  
of bars in opposite end.



CROSS SECTION  
(Looking )

Notes:  
See Sheet of for superstructure details  
and Bill of Material.  
Reinforcement bars designated (E) shall be  
epoxy coated.  
Bars indicated thus 20 x 3-#5 etc. indicates  
20 lines of bars with 3 lengths per line.  
See Sheet of for parapet reinforcement.

DESIGNED -	-
CHECKED -	-
DRAWN -	-
CHECKED -	-

EXAMINED	200
PASSED	ENGINEER OF BRIDGE DESIGN
	ENGINEER OF BRIDGES AND STRUCTURES

S-2-L<30°

10-22-04



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-		
-	-	-		
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -  
- SHEETS

Figure 10: Reinforcement details for a bridge deck. The diagram shows a plan view of a bridge deck with various reinforcement bars labeled. Key features include:

- Deck Dimensions:** Total width is  $a$ , total length is  $a$ . The deck is divided into sections of  $1'-7''$  and  $5''$ .
- Reinforcement Bars:**
  - Top Deck:**  $- \#4 d_3(E)$  bars at  $11''$  cts. (Outside Face),  $- \#5 d_1(E)$  bars at  $11''$  cts. (Inside Face).
  - Bottom Deck:**  $- \#6 a_2(E)$  bars at cts. Top (Lap with alternate  $a(E)$  bars),  $3 \times - \#5 b(E)$  bars Top of slab,  $2 - \#6 b_1(E)$  bars Top of slab.
  - Internal Reinforcement:**  $- \# a(E)$  bars at cts. Top,  $- \# a_1(E)$  bars at cts. Bottom,  $- \# a(E)$  bars at cts. Top,  $- \# a_1(E)$  bars at cts. Bottom,  $- \#5 b(E)$  bars equally spaced at  $\pm 12''$  cts. Top of slab,  $- \# b_1(E)$  bars spaced as shown in cross section. Bottom of slab,  $- \#6 b_1(E)$  bars at  $\pm 12''$  cts. Top of slab over pier.
- Other Details:**
  - Back of Abut.** and **Skew** are indicated.
  - Aluminum sheeted construction joints in base of parapet** are shown.
  - Ordering Note:** \* Order  $a(E)$  &  $a_1(E)$  bars full length. Cut to fit skew and use remainder of bars in opposite end.

The image contains two technical drawings of a parapet structure. The top drawing is a longitudinal section showing the 'face to face parapets' with a total width of 'o. to o. deck' and 'face to face parapets' dimensioned as '1'-7"'. It shows the 'slope per ft.' and 'Longitudinal construction joints'. The bottom drawing is a cross-section showing the 'Total Drop = 2'-10"'. It details the parapet structure with various dimensions and labels: 'd<sub>2</sub>(E)', 'd<sub>1</sub>(E)', 'b<sub>1</sub>(E)', 'a<sub>2</sub>(E)', 'b<sub>2</sub>(E)', 'd<sub>3</sub>(E)', 'b(E)', 'a<sub>1</sub>(E)', and '2"'. It also indicates the 'spaces at' and the number of bars 'b<sub>2</sub>(E) bars at cts. typ. btwn. beams'.

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

10-22-04

CROSS SECTION  
(Looking )



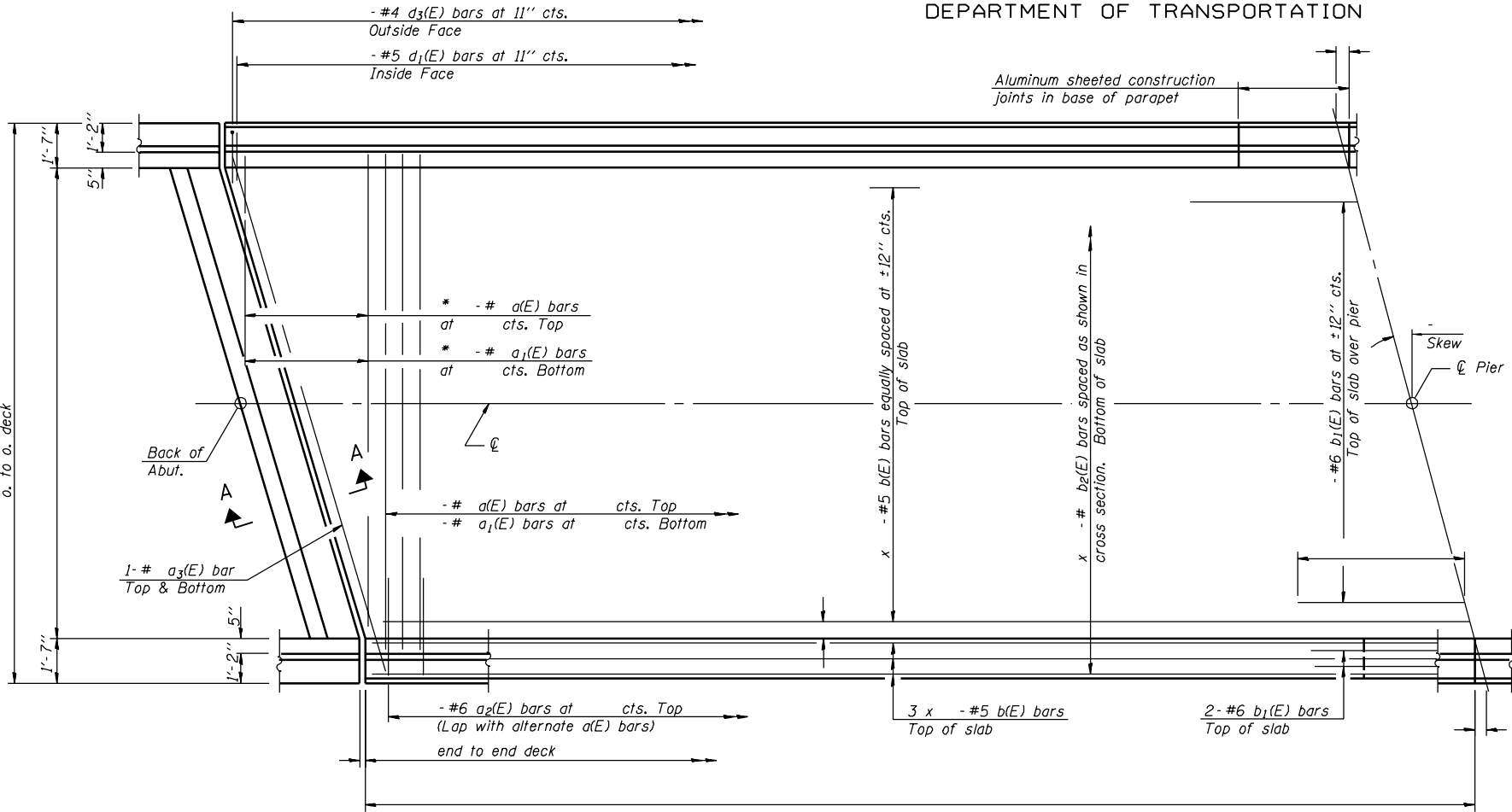
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT -	

SHEET NO. -

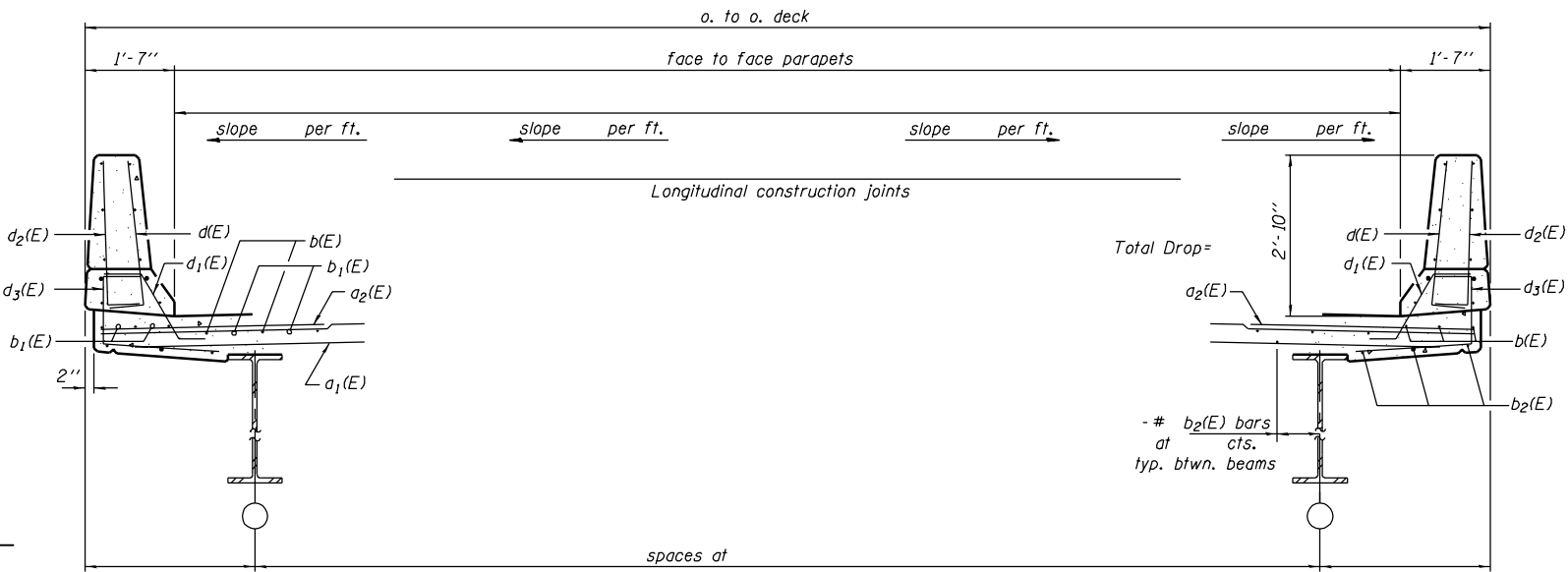
- SHEETS

Contract #



\* Order a(E) & a1(E) bars full length.  
Cut to fit skew and use remainder  
of bars in opposite end.

HALF PLAN



CROSS SECTION

(Looking )

Notes:  
See Sheet of for superstructure details  
and Bill of Material.  
Reinforcement bars designated (E) shall be  
epoxy coated.  
Bars indicated thus 20 x 3-#5 etc. indicates  
20 lines of bars with 3 lengths per line.  
See Sheet of for parapet reinforcement.

DESIGNED -	-
CHECKED -	-
DRAWN -	-
CHECKED -	-

EXAMINED	200
PASSED	ENGINEER OF BRIDGE DESIGN
	ENGINEER OF BRIDGES AND STRUCTURES

S-2-R(30°)

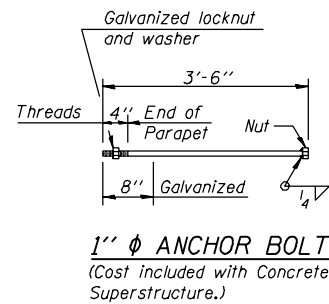
10-22-04



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-		
-	-	-		
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

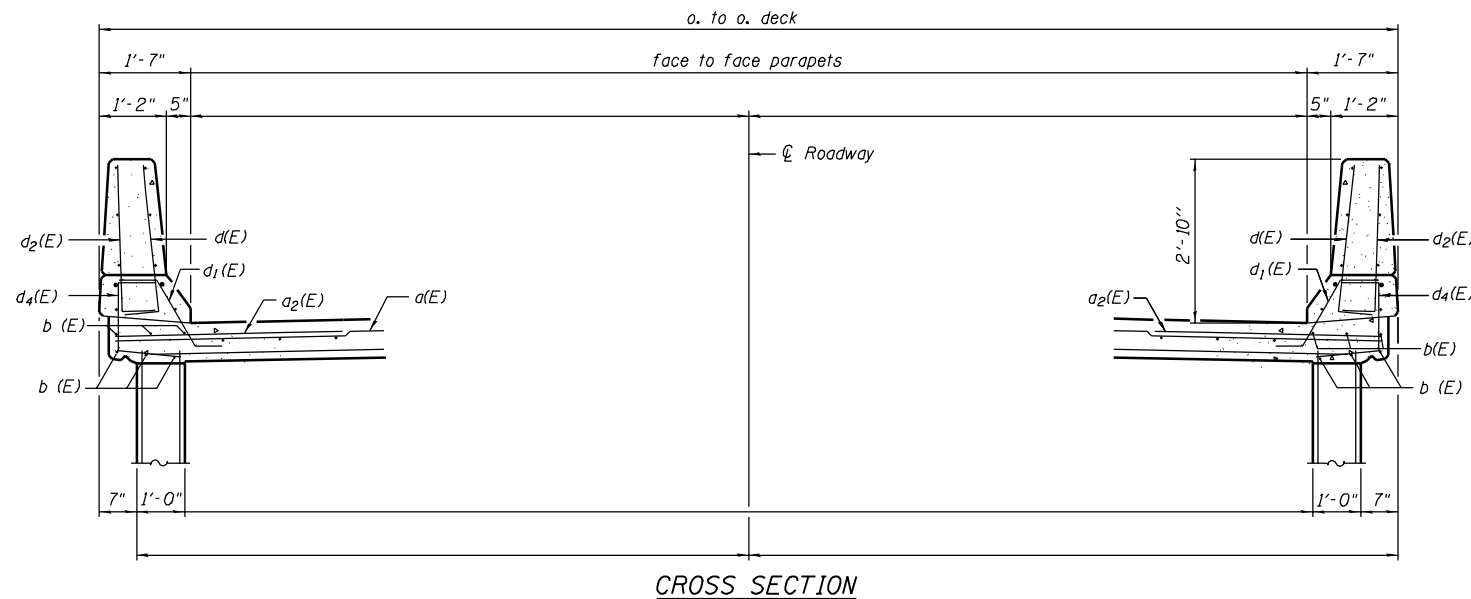
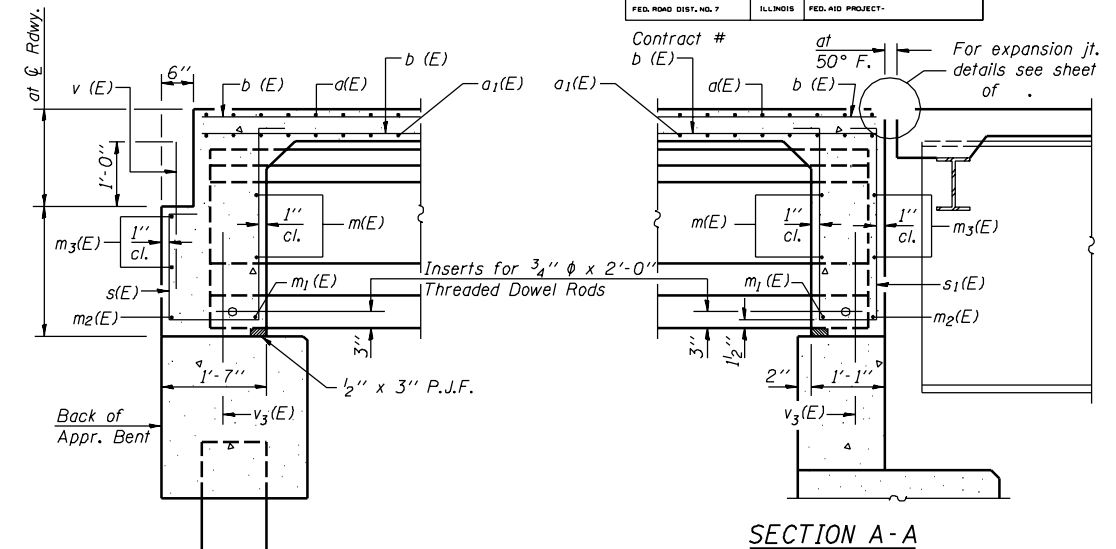
SHEET NO. -

- SHEETS



DESIGNED -
CHECKED -
DRAWN -
CHECKED -

10-22-04



Notes:  
See sheets and of for  $v_3(E)$   
thru  $v_6(E)$  bars.

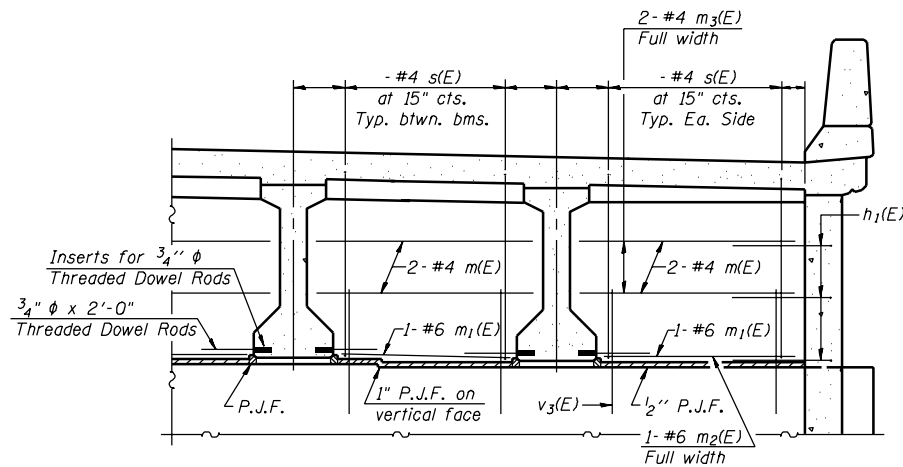


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

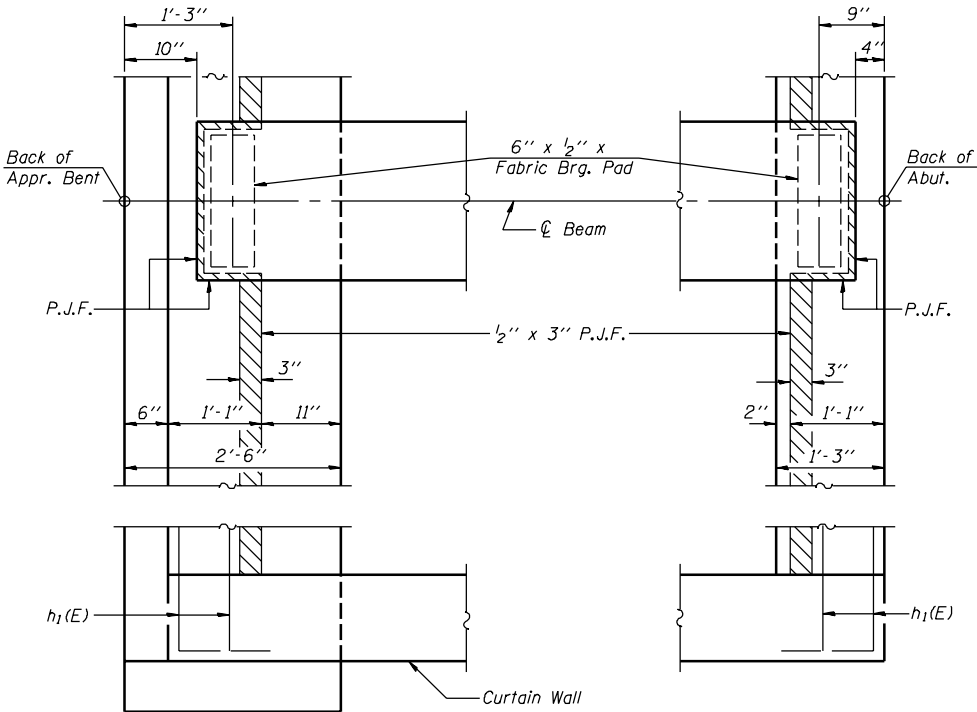
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -  
- SHEETS

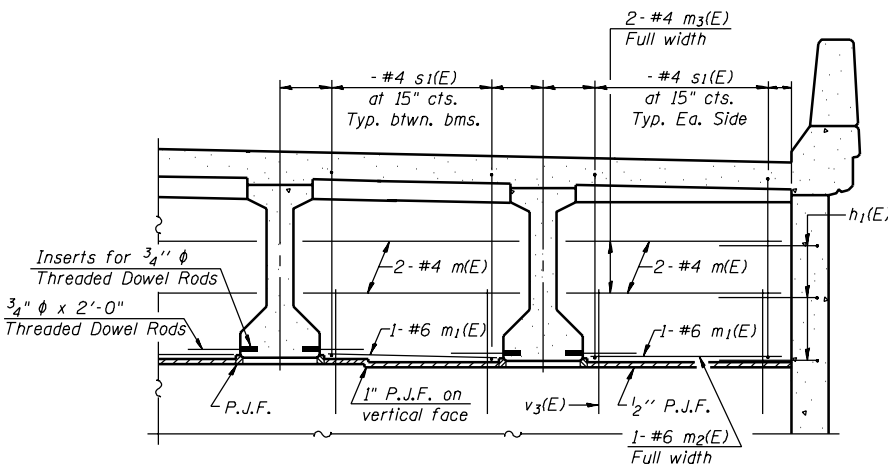
Contract #



**DIAPHRAGM AT APPROACH BENT**  
For location of m(E), m1(E), m2(E), and m3(E) bars  
see Section B-B on sheet of .



**PARTIAL PLAN**

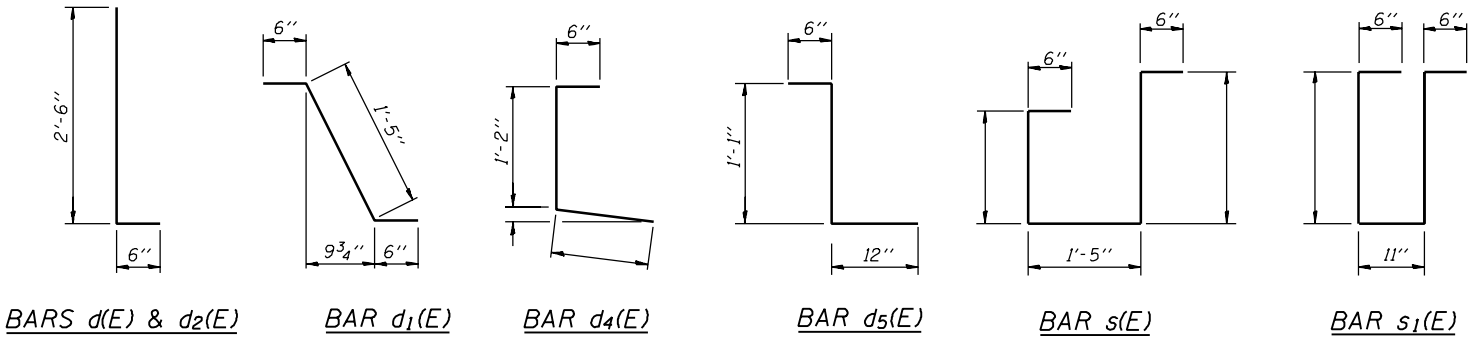


**DIAPHRAGM AT ABUTMENT**  
For location of m(E), m1(E), m2(E), and m3(E) bars  
see Section A-A on sheet of .

**TWO APPROACH SLABS**  
**BILL OF MATERIAL**

Bar	No.	Size	Length	Shape
d(E)				
d1(E)				
d2(E)		#6	4'-6"	
b(E)		#5		
b(E)				
d(E)		#5	3'-0"	
d1(E)		#5	2'-5"	
d2(E)		#4	3'-0"	
d4(E)		#4		
d5(E)		#5	2'-7"	
e(E)		#4		
e(E)		#5		
e(E)		#8		
m(E)		#4		
m1(E)		#6		
m2(E)		#6		
m3(E)		#4		
s(E)		#4		
s1(E)		#4		
v(E)		#5		
Reinforcement Bars, Epoxy Coated			Pound	
Concrete Superstructure			Cu. Yd.	

Notes:  
Reinforcement bars designated (E) shall be epoxy coated.  
See sheet and of for h1(E) and v3(E) bars.



**BARS d(E) & d2(E)**

**BAR d1(E)**

**BAR d4(E)**

**BAR d5(E)**

**BAR s(E)**

**BAR s1(E)**

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	ENGINEER OF BRIDGE DESIGN	
CHECKED -	PASSED	
	ENGINEER OF BRIDGES AND STRUCTURES	

SA-ID-0

10-22-04

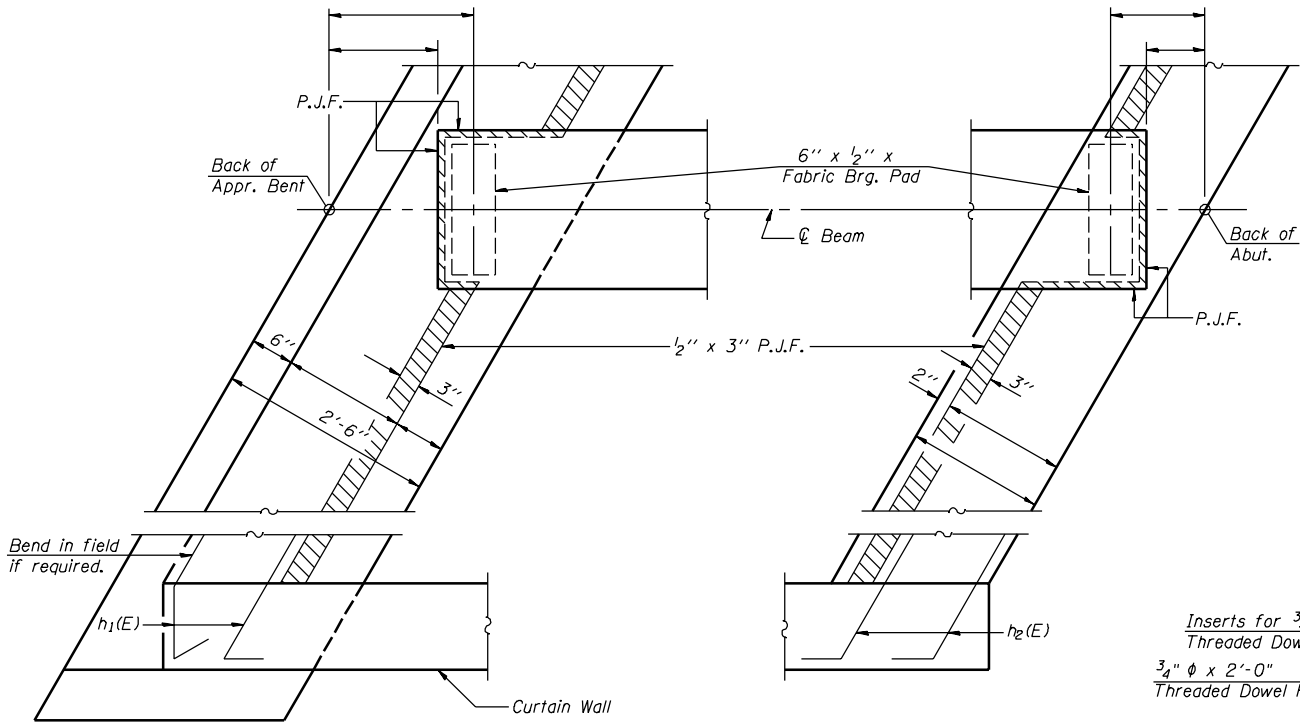


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

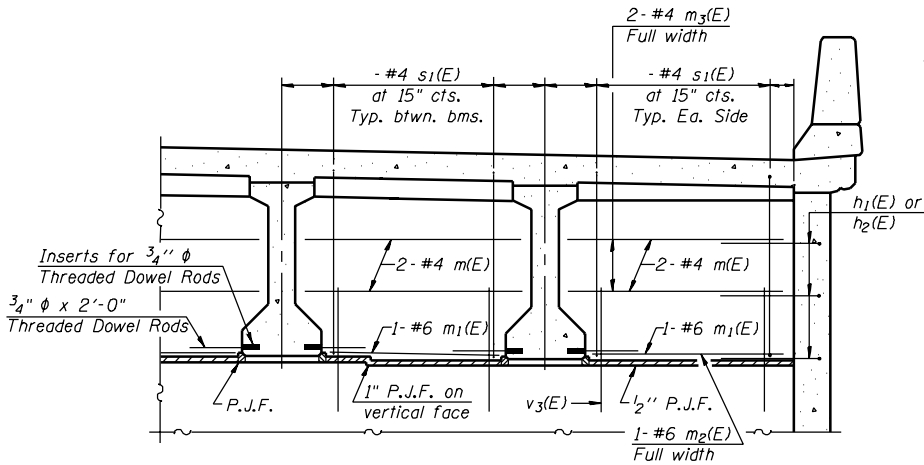
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -  
- SHEETS

Contract #

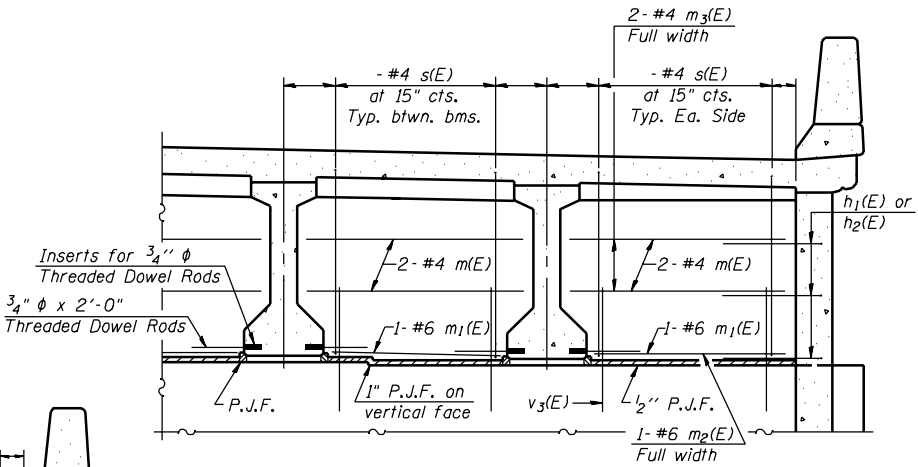


PARTIAL PLAN



DIAPHRAGM AT ABUTMENT

For location of m(E), m1(E), m2(E), and m3(E) bars  
see Section A-A on sheet of .



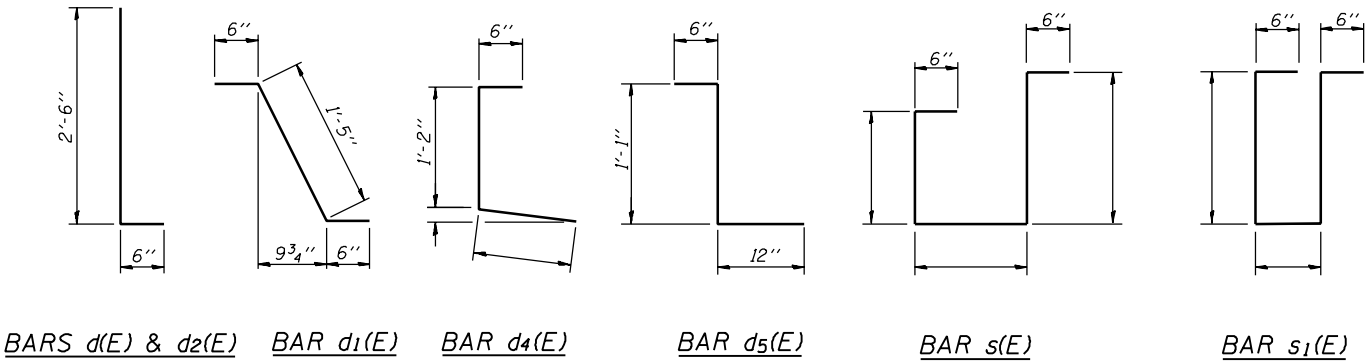
DIAPHRAGM AT APPROACH BENT

For location of m(E), m1(E), m2(E), and m3(E) bars  
see Section B-B on sheet of .

TWO APPROACH SLABS  
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
d(E)				
d1(E)				
d2(E)		#6	4'-6"	
d3(E)				
b (E)		#5		
b (E)				
d(E)		#5	3'-0"	
d1(E)		#5	2'-5"	
d2(E)		#4	3'-0"	
d4(E)		#4		
d5(E)		#5	2'-7"	
e (E)		#4		
e (E)		#5		
e (E)		#8		
m(E)		#4		
m1(E)		#6		
m2(E)		#6		
m3(E)		#4		
s(E)		#4		
s1(E)		#4		
v (E)		#5		
Reinforcement Bars, Epoxy Coated			Pound	
Concrete Superstructure			Cu. Yd.	

Notes:  
Reinforcement bars designated (E) shall be epoxy coated.  
See sheet and of for h1(E), h2(E) and v3(E) bars.



DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	ENGINEER OF BRIDGE DESIGN	
CHECKED -	PASSED	
	ENGINEER OF BRIDGES AND STRUCTURES	

SA - 1D - L

10-22-04

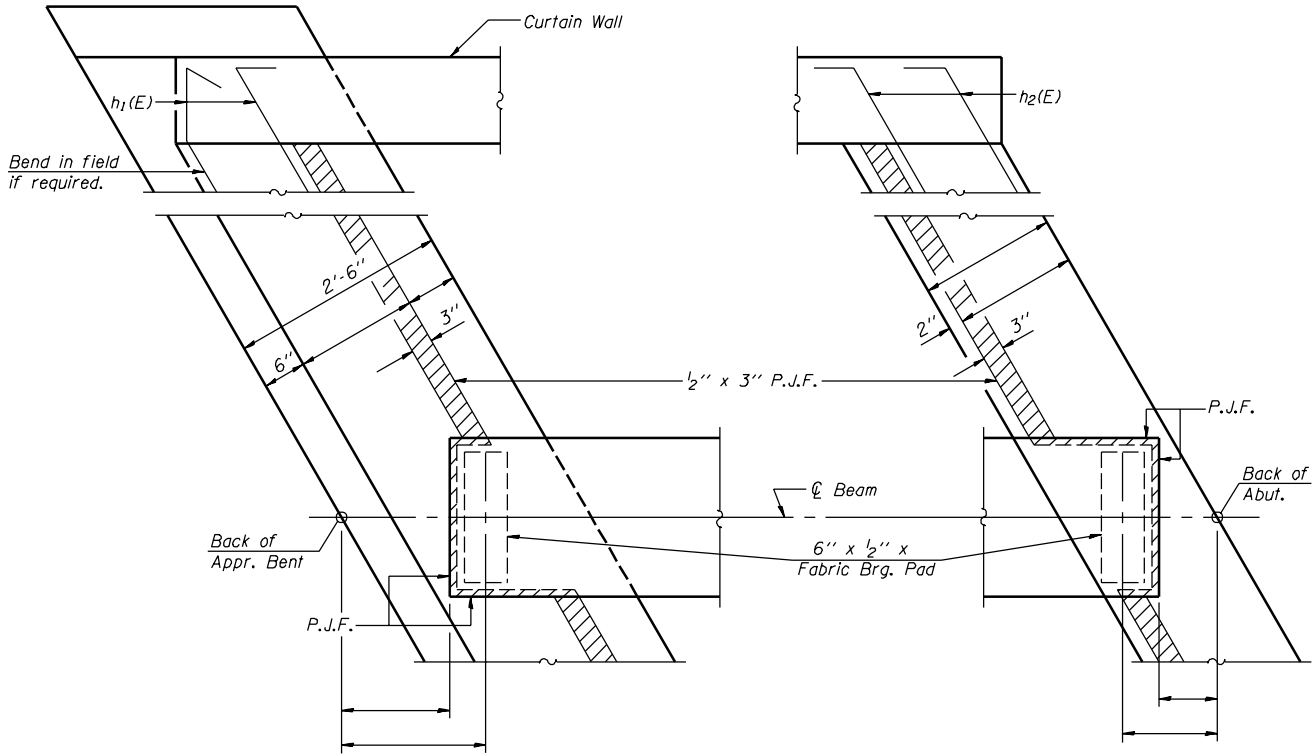


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT-		

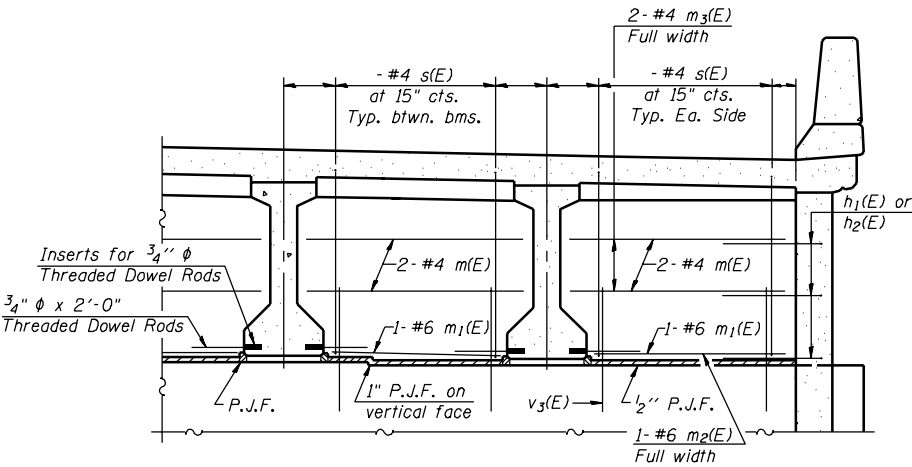
SHEET NO. -  
- SHEETS

Contract #



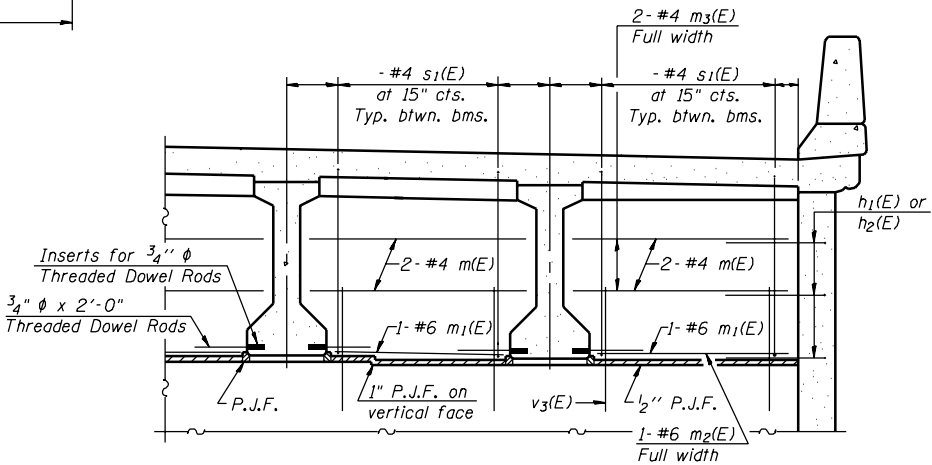
PARTIAL PLAN

Notes:  
Reinforcement bars designated (E) shall be epoxy coated.  
See sheet and of for  $h_1(E)$ ,  $h_2(E)$  and  $v_3(E)$  bars.



DIAPHRAGM AT APPROACH BENT

For location of  $m(E)$ ,  $m_1(E)$ ,  $m_2(E)$ , and  $m_3(E)$  bars see Section B-B on sheet of .

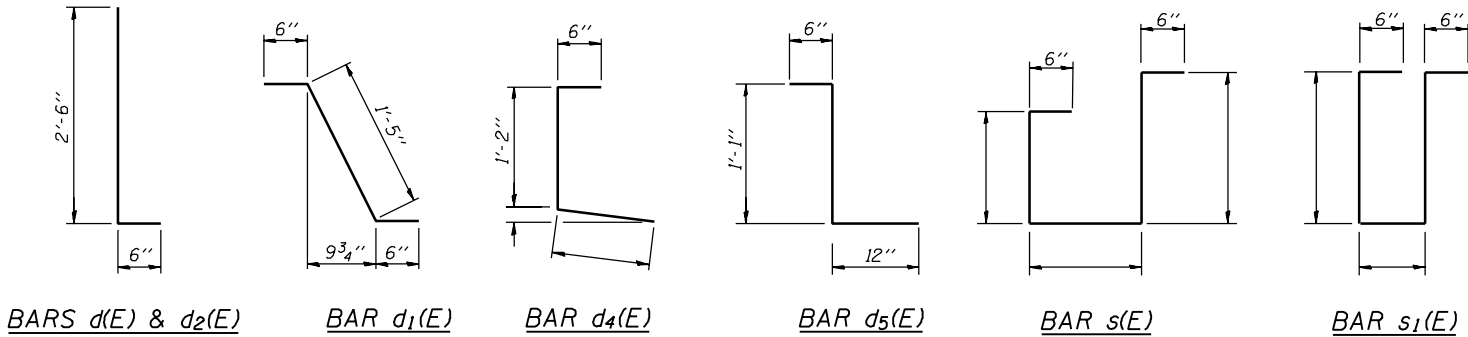


DIAPHRAGM AT ABUTMENT

For location of  $m(E)$ ,  $m_1(E)$ ,  $m_2(E)$ , and  $m_3(E)$  bars see Section A-A on sheet of .

TWO APPROACH SLABS  
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
$d(E)$				
$d_1(E)$				
$d_2(E)$	#6	4'-6"		
$d_3(E)$				
$b(E)$	#5			
$b(E)$				
$d(E)$	#5	3'-0"		
$d_1(E)$	#5	2'-5"		
$d_2(E)$	#4	3'-0"		
$d_4(E)$	#4			
$d_5(E)$	#5	2'-7"		
$e(E)$	#4			
$e(E)$	#5			
$e(E)$	#8			
$m(E)$	#4			
$m_1(E)$	#6			
$m_2(E)$	#6			
$m_3(E)$	#4			
$s(E)$	#4			
$s_1(E)$	#4			
$v(E)$	#5			
Reinforcement Bars, Epoxy Coated			Pound	
Concrete Superstructure			Cu. Yd.	



DESIGNED -
CHECKED -
DRAWN -
CHECKED -

EXAMINED	200
PASSED	ENGINEER OF BRIDGE DESIGN
	ENGINEER OF BRIDGES AND STRUCTURES

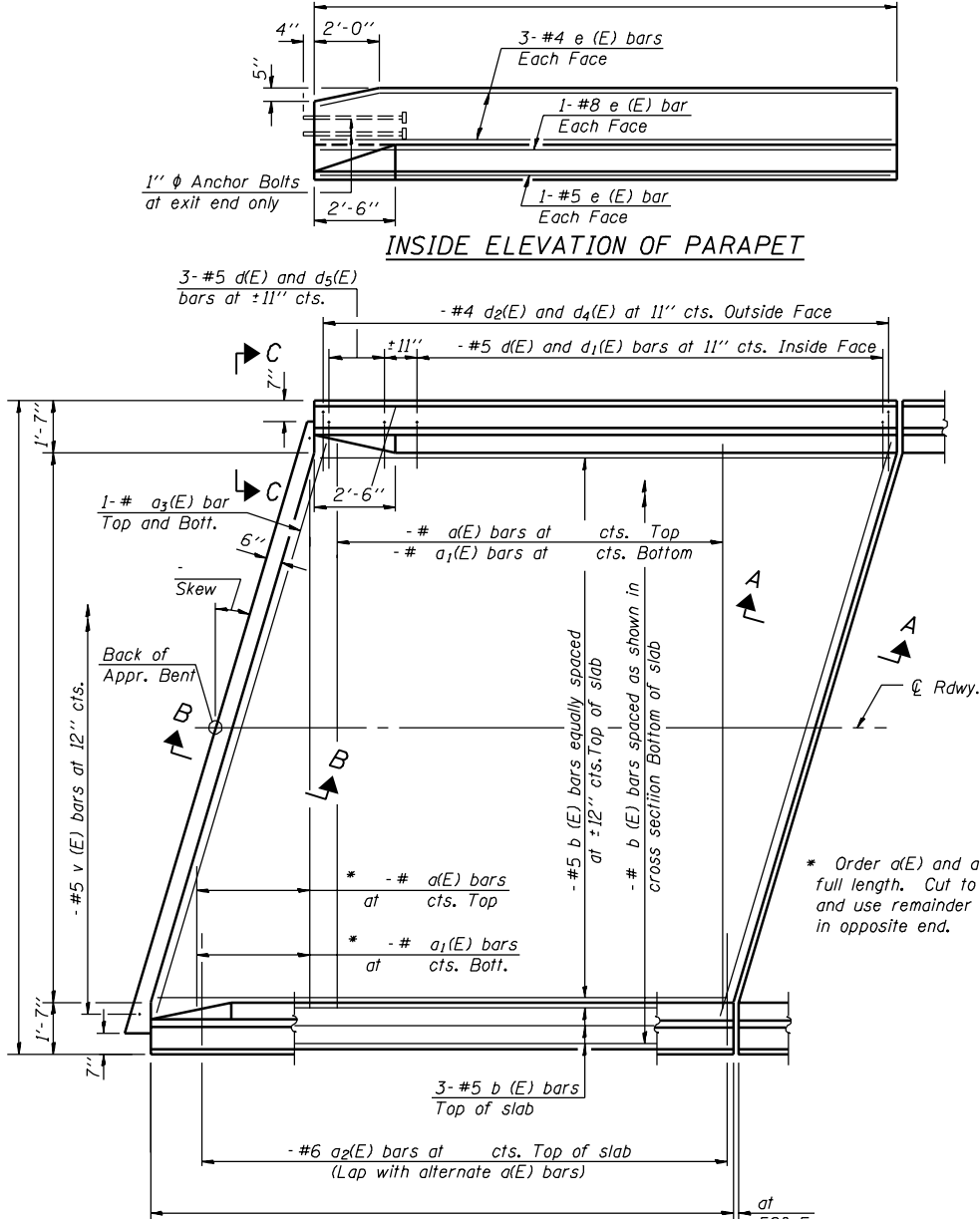
SA-ID-R

10-22-04

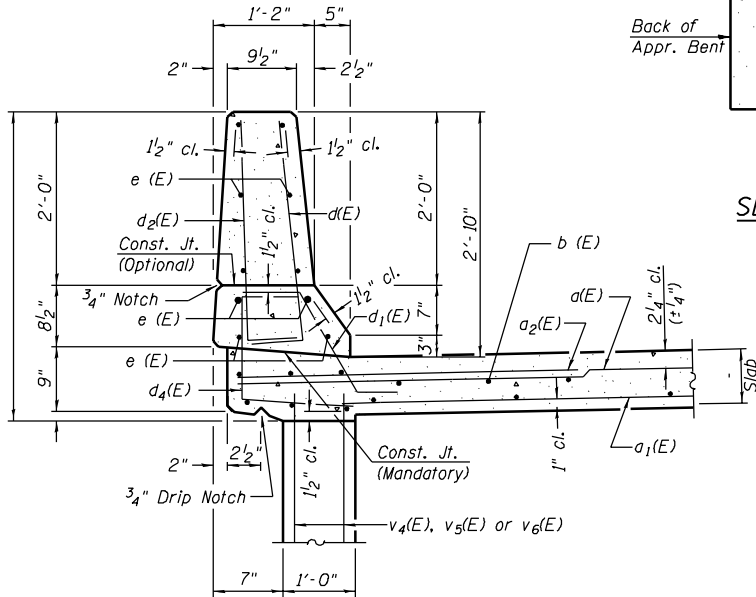


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

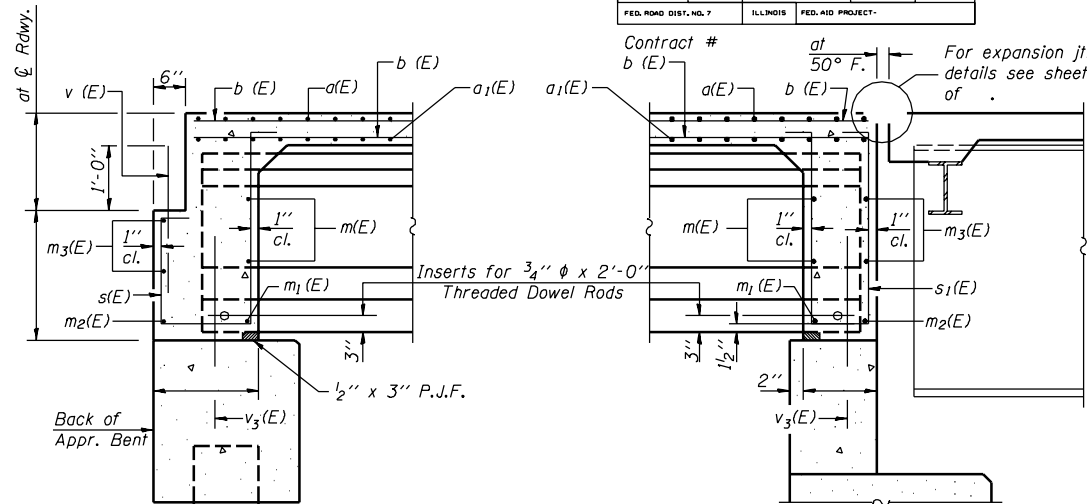
ROUTE NO.	SECTION	COUNTY	100% SHEETS	SHEET NO.
FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		SHEETS



INSIDE ELEVATION OF PARAPET



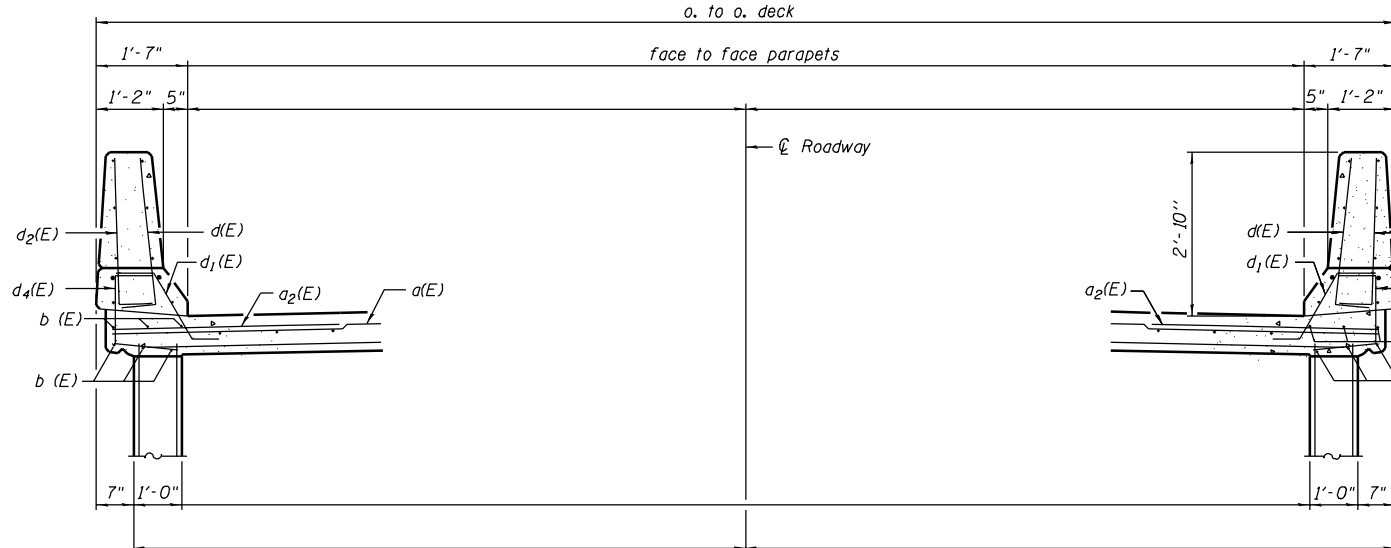
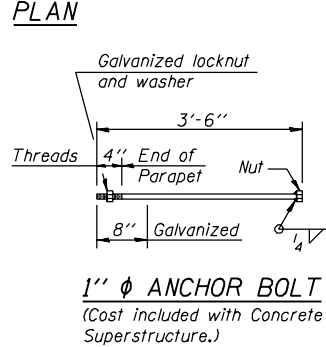
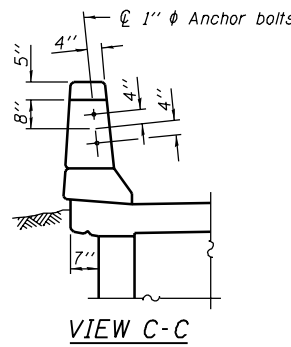
SECTION THRU PARAPET



SECTION A-A

SECTION B-B

Notes:  
See sheets and of for v3(E)  
thru v6(E) bars.



CROSS SECTION

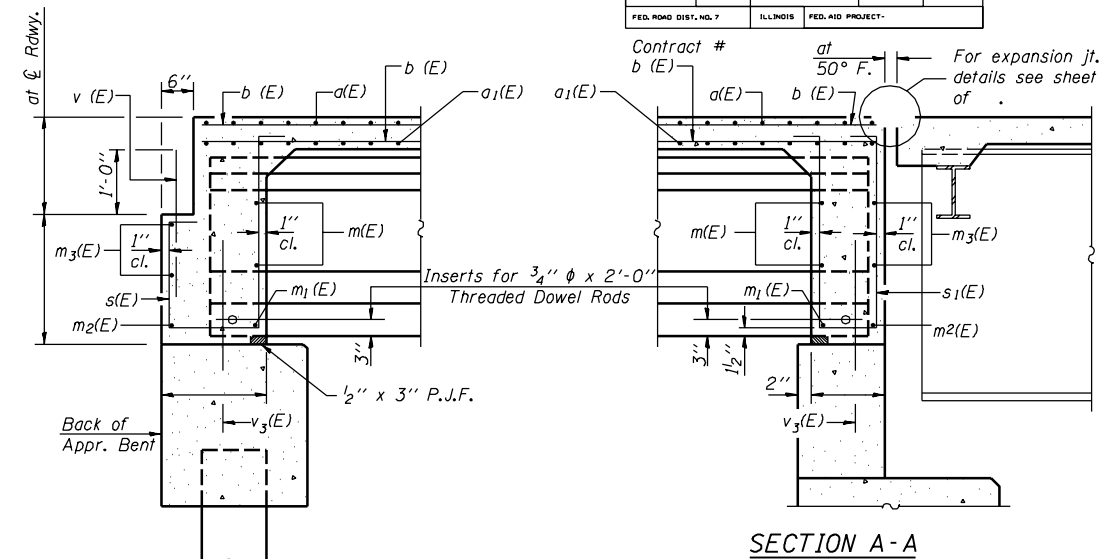
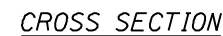
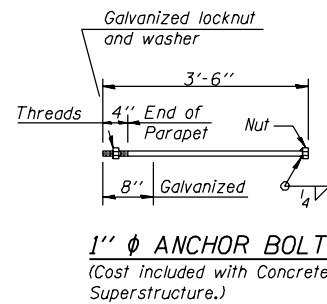
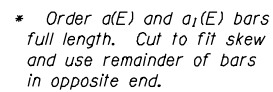
DESIGNED -	200
CHECKED -	ENGINEER OF BRIDGE DESIGN
DRAWN -	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -	



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-		
-	-	-		
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -

- SHEETS



Notes:  
See sheets and of for  $v_3(E)$   
thru  $v_6(E)$  bars.

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

SA-1-R

10-22-04

EXAMINED \_\_\_\_\_  
PASSED \_\_\_\_\_  
ENGINEER OF BRIDGE DESIGN  
ENGINEER OF BRIDGES AND STRUCTURES



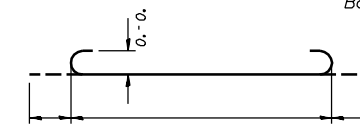
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-		
-	-	-		
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -

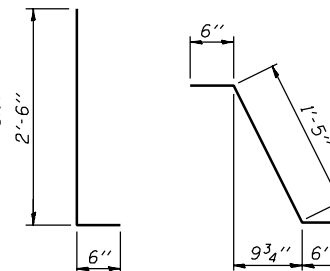
- SHEETS

[illegible]

SECTION A - A



BAR  $b_{10}(E)$

BARS  $d(E)$  &  $d_2(E)$ 
$$\underline{BAR \ d_1(E)}$$

Bar	No.	Size	Length	Shape
a <sub>2</sub> (E)		#6	4'-6"	_____
a <sub>3</sub> (E)		#5		_____
a <sub>4</sub> (E)				_____
b <sub>9</sub> (E)		#5		_____
b <sub>10</sub> (E)				C_____
d(E)		#5	3'-0"	_____
d <sub>1</sub> (E)		#5	2'-5"	_____
d <sub>2</sub> (E)		#4	3'-0"	_____
d <sub>4</sub> (E)		#4		_____
d <sub>5</sub> (E)		#5	4'-8"	_____
e(E)		#4		_____
e(E)		#5		_____
e(E)		#8		_____
Reinforcement Bars, Epoxy Coated			Pound	
Concrete Superstructure			Cu. Yd.	

4"

5"

2'-0"

3-#4 e (E) bars  
Each Face

1-#8 e (E) bar  
Each Face

1-#5 e (E) bar  
Each Face

1"  $\phi$  Anchor Bolts  
at exit end only

2'-6"

**INSIDE ELEVATION OF PARAPET**

3-#5 d(E) & d<sub>5</sub>(E)  
bars at 11" cts.

-#4 d<sub>2</sub>(E) & d<sub>4</sub>(E) at 11" cts. Outside Face

±11" -#5 d(E) & d<sub>1</sub>(E) bars at 11" cts. Inside Face

1'-1"

2'-6"

-#5 a<sub>3</sub>(E) bars at 18" cts. Top

-# a<sub>4</sub>(E) bars at 18" cts. Bottom

6"

2'-0"

-#5 b<sub>9</sub>(E) bars at 18" cts.  
Top of slab

-# b<sub>10</sub>(E) bars at 18" cts.  
Bottom of slab

1'-0"

Back of  
Appr. Bent

1'-1"

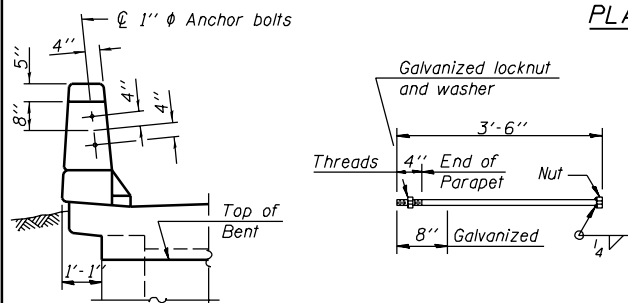
2-#5 b<sub>9</sub>(E) Top of slab

3-#5 b<sub>9</sub>(E) Bottom of Slab

-#6 a<sub>2</sub>(E) bars at 12" cts. Top of slab

at  
50°

PLAN



VIEW C-C

1"  $\phi$  ANCHOR BOLT  
(Cost included with  
Concrete Superstructure)

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

EXAMINED \_\_\_\_\_

\_\_\_\_\_  
ENGINEER OF BRIDGE DESIGN

PASSED \_\_\_\_\_

\_\_\_\_\_  
ENGINEER OF BRIDGES AND STRUCTURES

Figure 10 is a detailed cross-section of a bridge deck. The central part is the roadway, flanked by two side parapets. The total width from the outer edge of the left parapet to the outer edge of the right parapet is 1'-7". The width of each parapet is 1'-2", and the height of the parapets is 5". The roadway width is 2'-10". The deck thickness is 2 1/4". The diagram shows various reinforcement details, including top bars  $d_2(E)$ ,  $d_1(E)$ , and  $d(E)$ ; bottom bars  $b_9(E)$  and  $b_{10}(E)$ ; and vertical bars  $v(E)$  or  $v(E)$  bars billed with abut. The central roadway is labeled "Roadway".

CROSS SECTION

NEAR ABUTMENT

$$\underline{BAR} \ d_4(E)$$
$$\underline{BAR} \ d_5(E)$$

SA-2-0

10-22-04

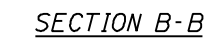


ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
"	"	"		
"	"	"		
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. "

" SHEETS

For details of expansion  
joint see Sheet of



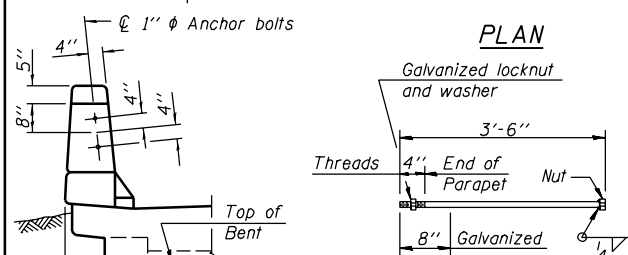
BAR  $d_1(E)$

Bar	No.	Size	Length	Shape
a <sub>2</sub> (E)		#6	4'-6"	_____
a <sub>3</sub> (E)		#5		_____
a <sub>4</sub> (E)				_____
b <sub>9</sub> (E)		#5		_____
b <sub>10</sub> (E)				C _____
d(E)		#5	3'-0"	_____
d <sub>1</sub> (E)		#5	2'-5"	┐
d <sub>2</sub> (E)		#4	3'-0"	_____
d <sub>4</sub> (E)		#4		└
d <sub>5</sub> (E)		#5	4'-8"	┐
e (E)		#4		_____
e (E)		#5		_____
e (E)		#8		_____
Reinforcement Bars, Epoxy Coated			Pound	
Concrete Superstructure			Cu. Yd.	

Diagram showing the elevation view of a wall section. The vertical section is 1'-2" high and 6" wide. The horizontal section is 9" high and 2'-3" long.

A diagram of a stepped profile. The top horizontal segment is labeled 6". The vertical segment is labeled 1' - 1". The bottom horizontal segment is labeled 12".

BAR  $d_5(E)$

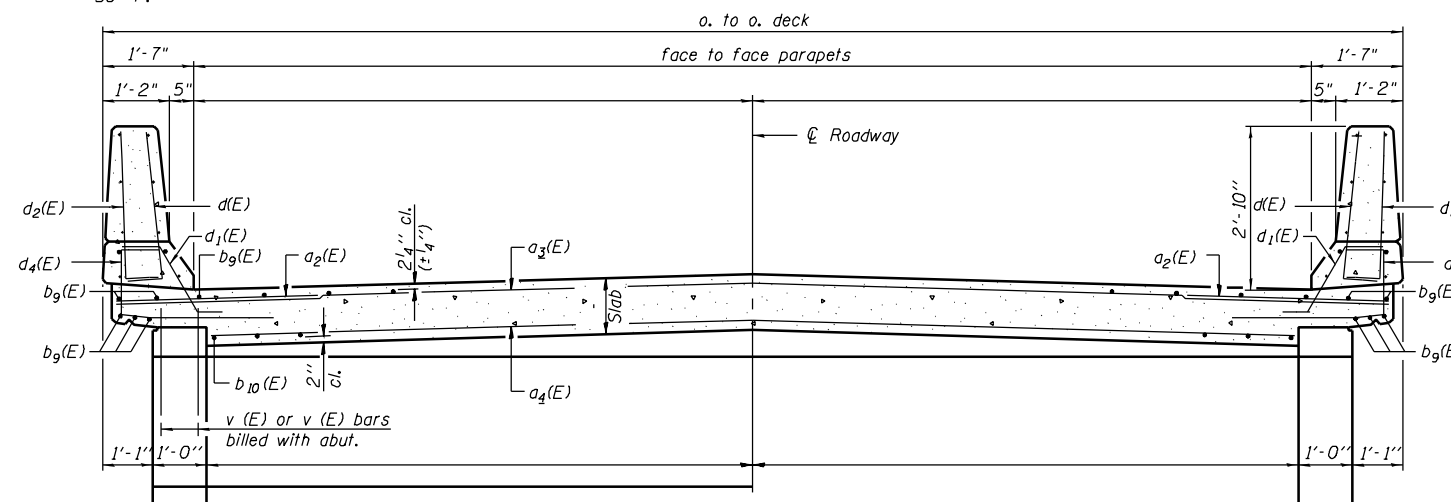


1"  $\phi$  ANCHOR BOLT  
(Cost included with  
Concrete Superstructure)

EXAMINED  
\_\_\_\_\_  
PASSED  
\_\_\_\_\_  
ENGINEER OF BRIDGE DESIGN  
ENGINEER OF BRIDGES AND STRUCTURES

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

10-22-04



CROSS SECTION

NEAR APPR. BENT

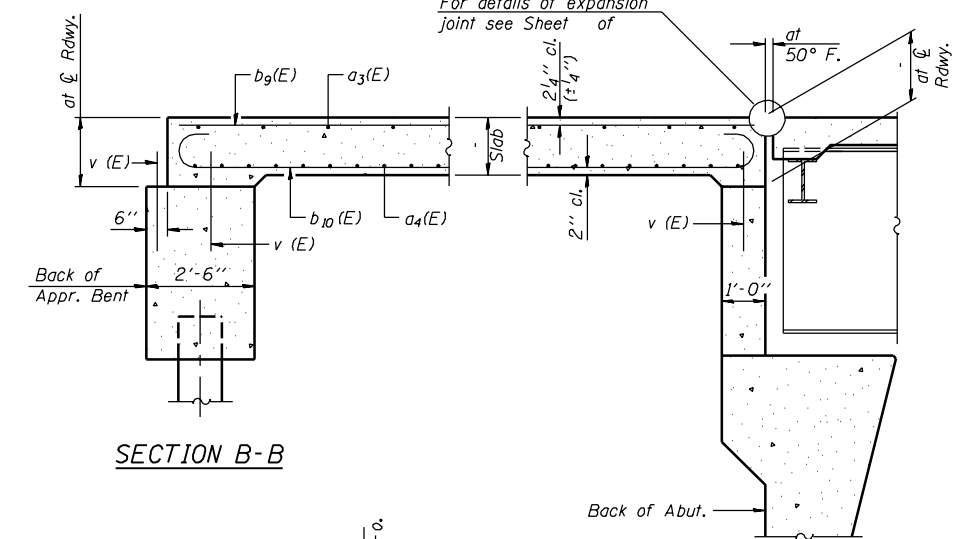
NEAR ABUTMENT



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

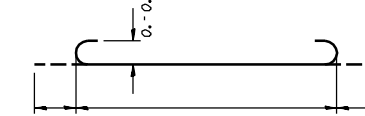
SHEET NO. -  
 - SHEETS

For details of expansion  
joint see Sheet of

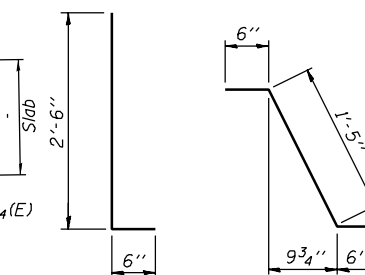
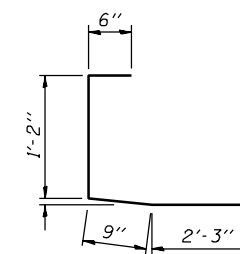


SECTION B-B

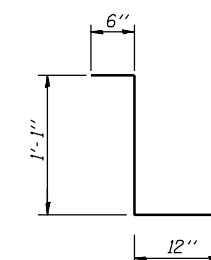
SECTION A - A



BAR  $b_{10}(E)$

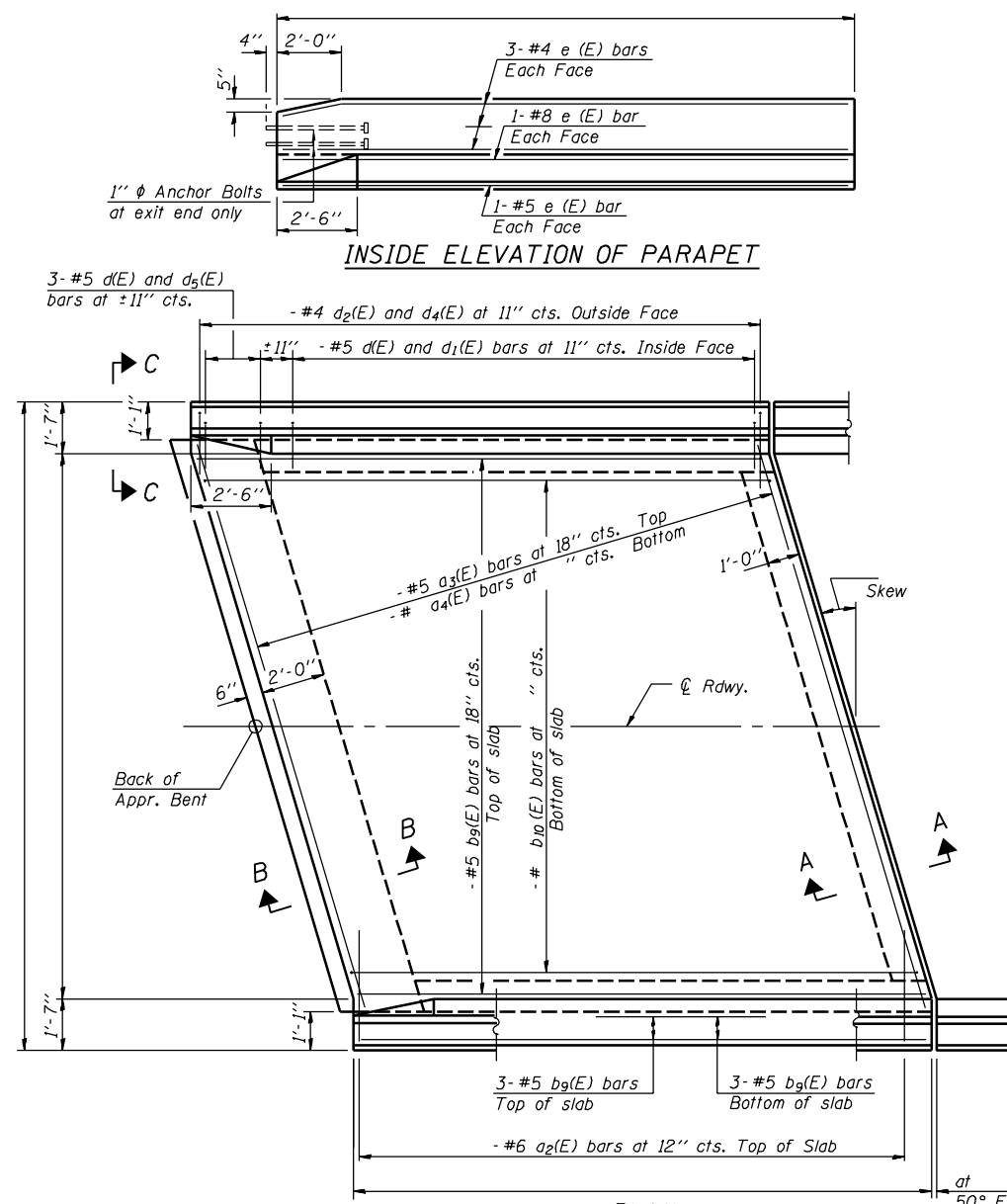
BARS  $d(E)$  &  $d_2(E)$ BAR  $d_1(E)$ 

BAR  $d_4(E)$

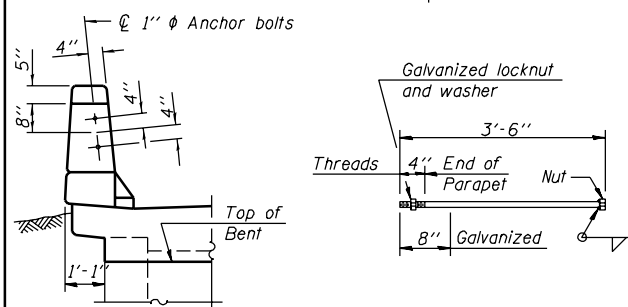

$$\underline{BAR \ d_5(E)}$$

Bar	No.	Size	Length	Shape
a <sub>2</sub> (E)		#6	4'-6"	_____
a <sub>3</sub> (E)		#5		_____
a <sub>4</sub> (E)				_____
b <sub>9</sub> (E)		#5		_____
b <sub>10</sub> (E)				C U
d(E)		#5	3'-0"	—
d <sub>1</sub> (E)		#5	2'-5"	∟
d <sub>2</sub> (E)		#4	3'-0"	—
d <sub>4</sub> (E)		#4		└
d <sub>5</sub> (E)		#5	4'-8"	∟
e (E)		#4		_____
e (E)		#5		_____
e (E)		#8		_____
Reinforcement Bars, Epoxy Coated			Pound	
Concrete Superstructure			Cu. Yd.	

Reinforcement bars designated (E) shall be epoxy coated.



### PLAN



VIEW C-C

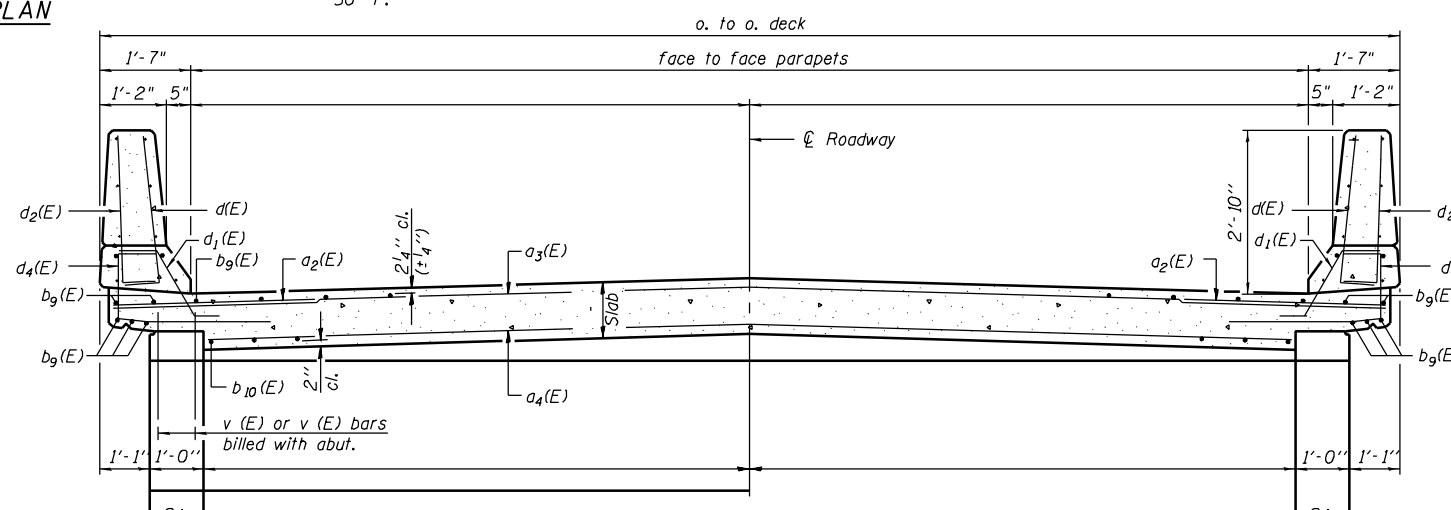
1"  $\phi$  ANCHOR BOLT  
(Cost included with  
Concrete Superstructure)

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

EXAMINED \_\_\_\_\_  
PASSED \_\_\_\_\_

ENGINEER OF BRIDGE DESIGN

ENGINEER OF BRIDGES AND STRUCTURES



NEAR APPR. BENT

CROSS SECTION

NEAR ABUTMENT



STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
-	-	-	-	-
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -

- SHEETS

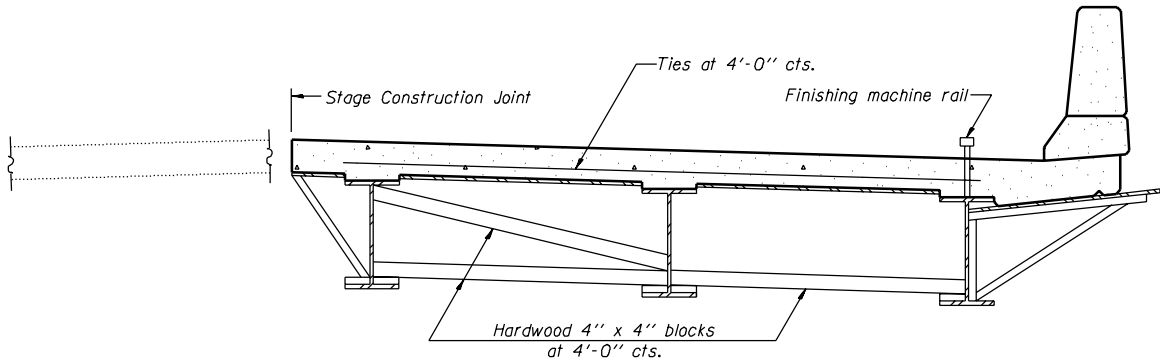
Contract #

When cantilever forming brackets are used, the work shall be done according to Article 503.06, except as modified below and in the details shown on this sheet.

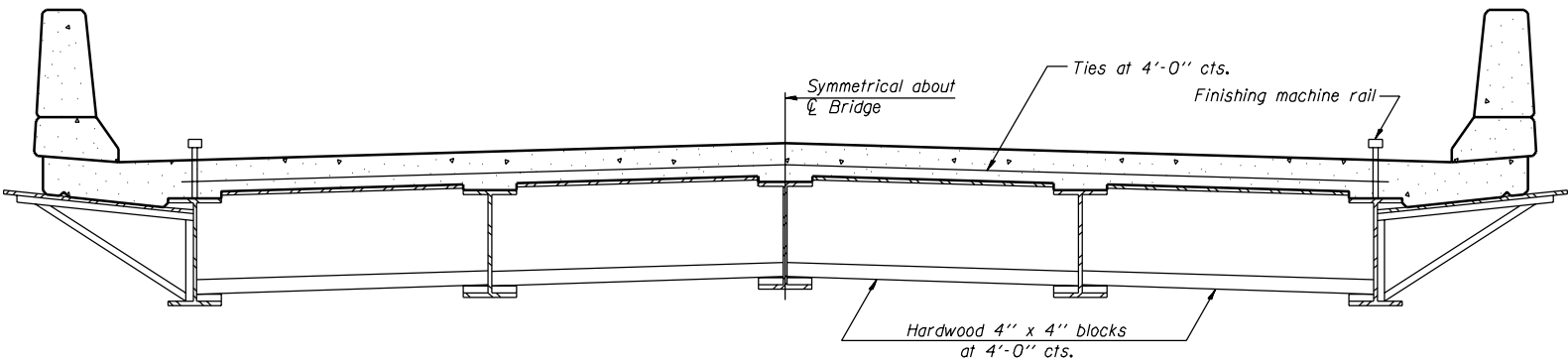
The finishing machine rails shall be placed on the top flange of the exterior beams.

The beams or girders, supporting cantilever forming brackets, shall be tied together at 4 foot intervals.

For Standard construction, or Stage Construction the Hardwood bracing materials shall be placed as shown between webs of beams in each bay.



FORM BRACES FOR  
STAGE CONSTRUCTION



FORM BRACES FOR  
STANDARD CONSTRUCTION

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

200
EXAMINED
ENGINEER OF BRIDGE DESIGN
PASSED
ENGINEER OF BRIDGES AND STRUCTURES

SB-1

10-22-04

CANTILEVER FORMING BRACKETS  
FOR SUPERSTRUCTURES WITH  
W27 BEAMS AND SMALLER

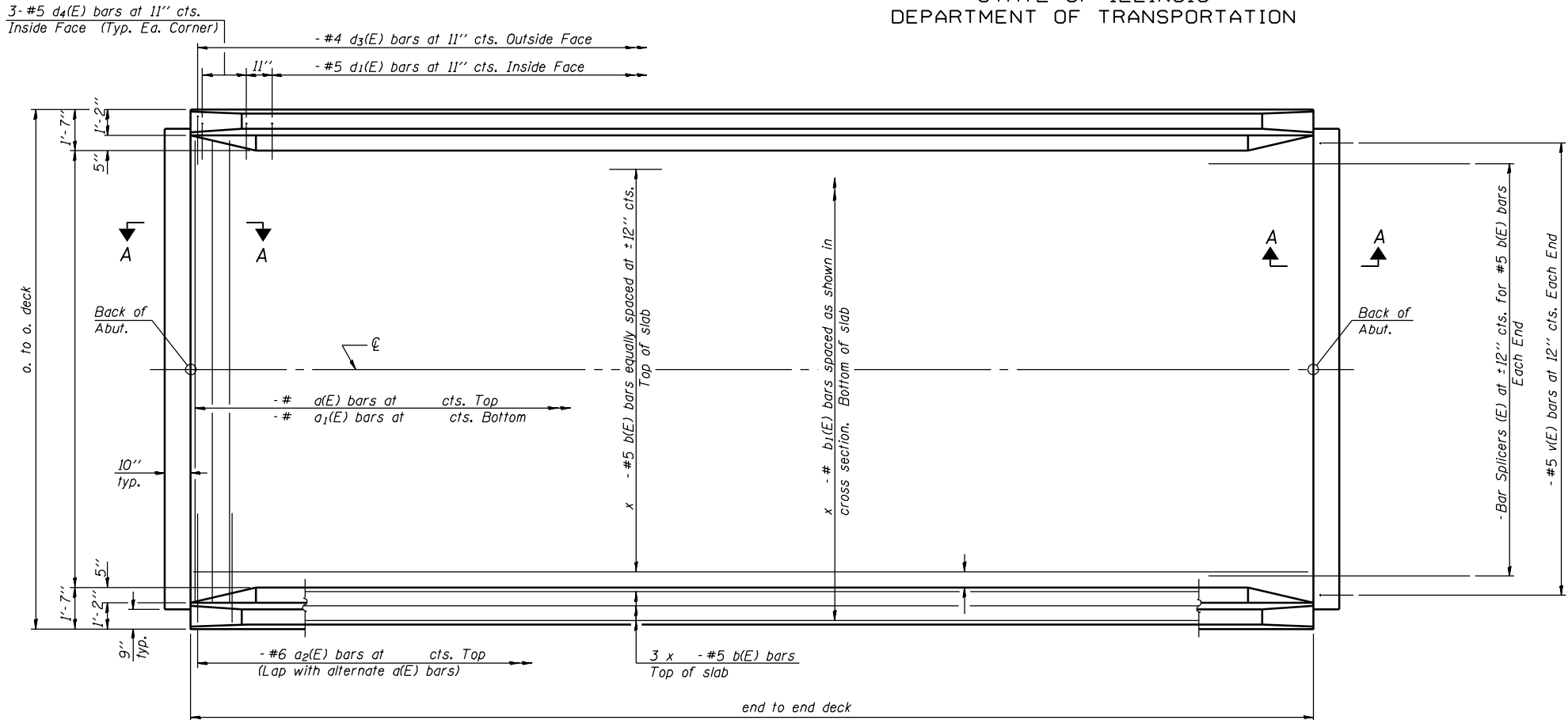


STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

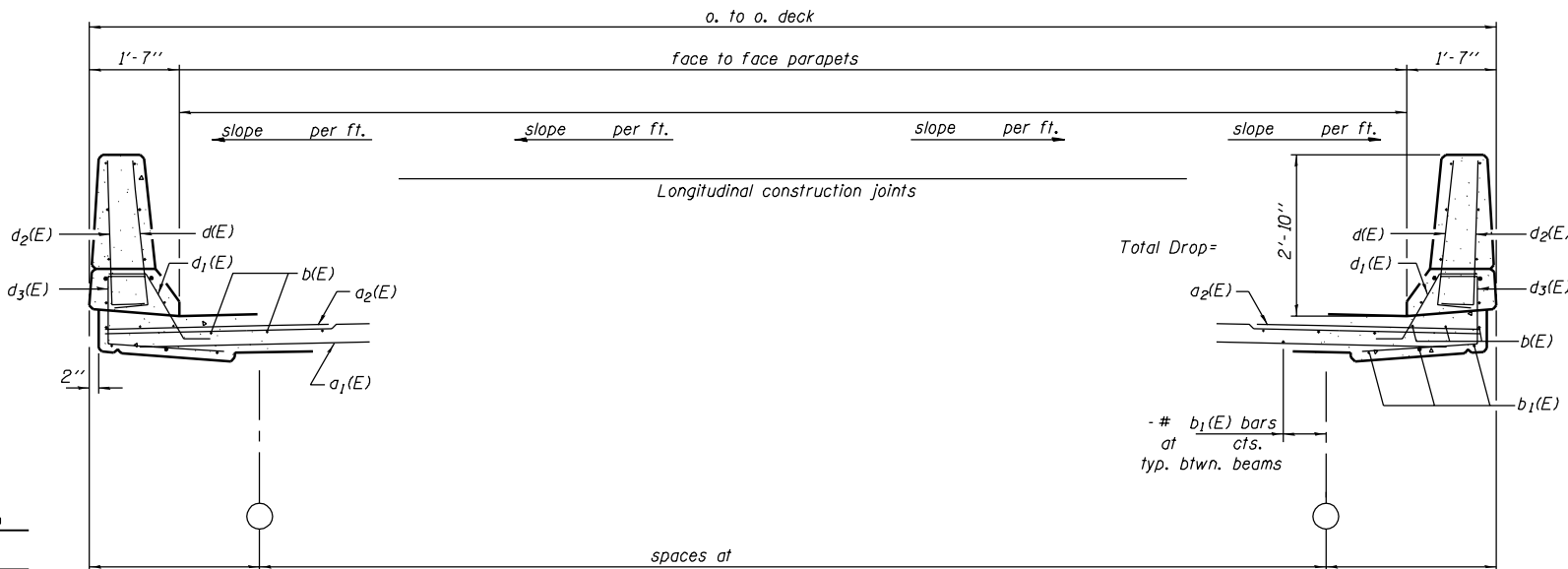
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

SHEET NO. -  
- SHEETS

Contract #



PLAN



CROSS SECTION

(Looking )

Notes:  
See Sheet of for superstructure details and Bill of Material.  
Reinforcement bars designated (E) shall be epoxy coated.  
Bars indicated thus 20 x 3- #5 etc. indicates 20 lines of bars with 3 lengths per line.  
See Sheet of for parapet reinforcement.

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

EXAMINED  
ENGINEER OF BRIDGE DESIGN

PASSED  
ENGINEER OF BRIDGES AND STRUCTURES

SI-1-0

10-22-04



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -

- SHEETS

STATE OF ILLINOIS  
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*o. to o. deck*

*Inside Face (Typ. Ea. Corner)*

- #4  $d_3(E)$  bars at 11" cts. Outside Face

- #5  $d_1(E)$  bars at 11" cts. Inside Face

1'-7"

1'-2"

5"

10" typ.

Back of Abut.

1-#  $a_3(E)$  bar Top & Bottom

\* - #  $a(E)$  bars at cts. Top

\* - #  $a_1(E)$  bars at cts. Bottom

- #  $a(E)$  bars at cts. Top

- #  $a_1(E)$  bars at cts. Bottom

- #5  $b(E)$  bars equally spaced at ±12" cts. Top of slab

x

- #  $b_1(E)$  bars spaced as shown in cross section. Bottom of slab

x

Back of Abut.

-Bar Splicers (E) at ±12" cts. for #5  $b(E)$  bars Each End

#5 w(E) bars at 12" cts. Each End

1'-7"

1'-2"

5"

9" typ.

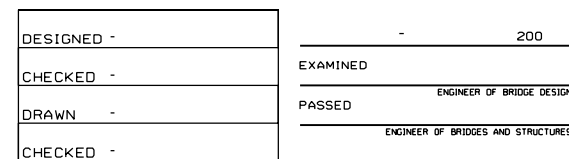
- #6  $a_2(E)$  bars at cts. Top (Lap with alternate  $a(E)$  bars)

3 x - #5  $b(E)$  bars Top of slab

end to end deck

\* Order  $a(E)$  and  $a_1(E)$  bars full length. Cut to fit skew and use remainder of bars in opposite end.

\* Order  $a(E)$  and  $a_1(E)$  bars full length.  
Cut to fit skew and use remainder  
of bars in opposite end.



10-22-04

CROSS SECTION  
(Looking )

Notes:  
See Sheet of for superstructure details  
and Bill of Material.  
Reinforcement bars designated (E) shall be  
epoxy coated.  
Bars indicated thus 20 x 3- #5 etc. indicates  
20 lines of bars with 3 lengths per line.  
See Sheet of for parapet reinforcement.



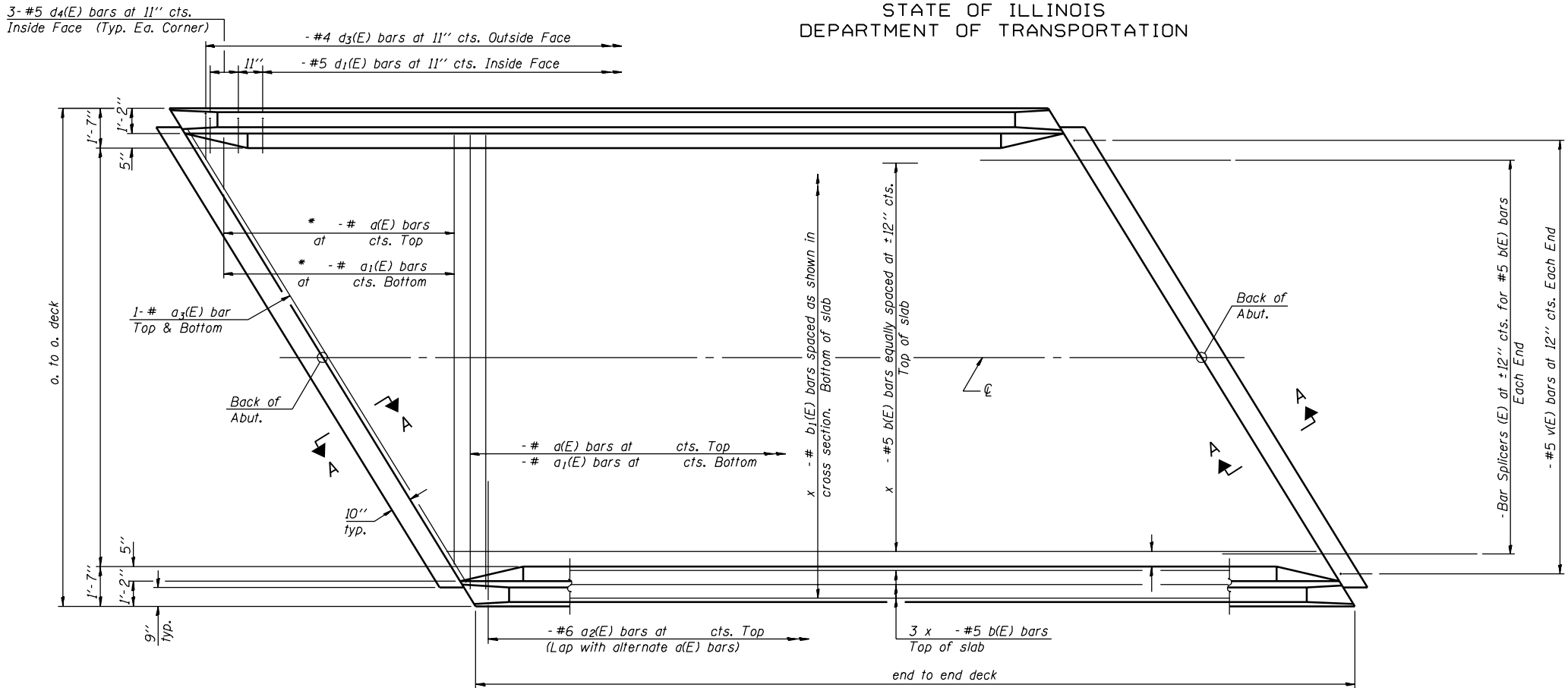
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT		

SHEET NO. -

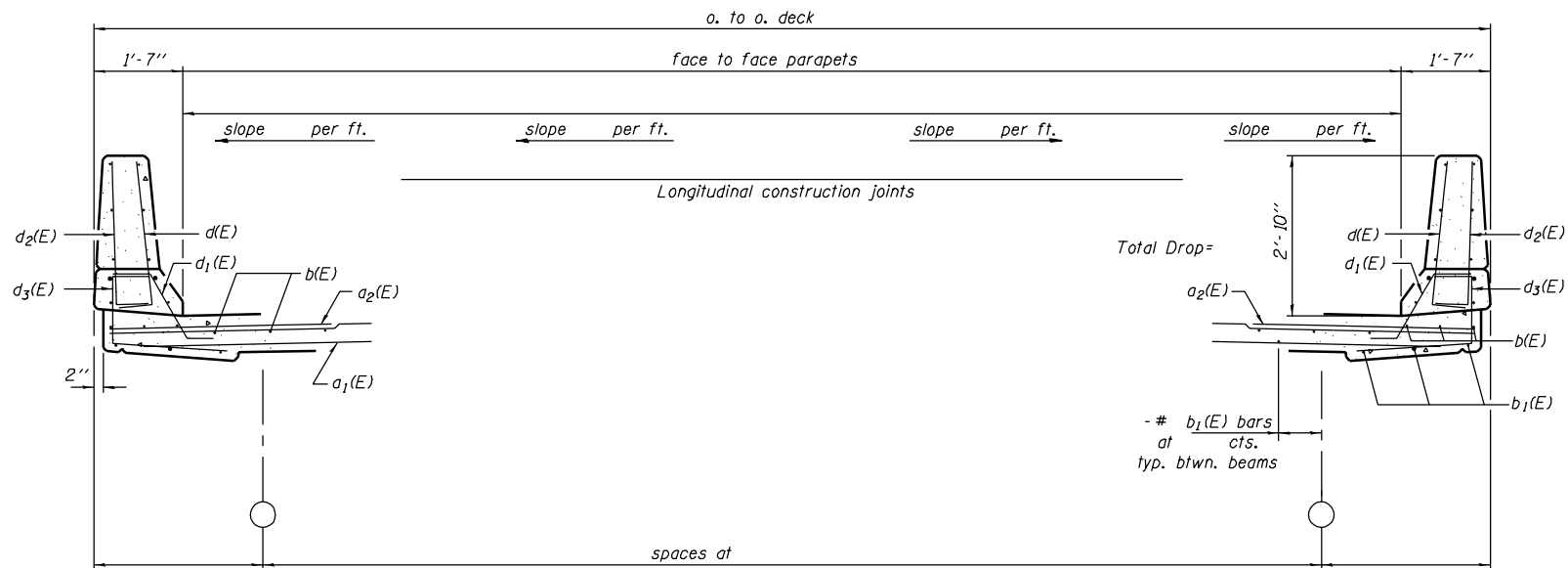
- SHEETS

Contract #



\* Order a(E) and a1(E) bars full length.  
Cut to fit skew and use remainder of  
bars in opposite end.

PLAN



CROSS SECTION  
(Looking )

Notes:  
See Sheet of for superstructure details  
and Bill of Material.  
Reinforcement bars designated (E) shall be  
epoxy coated.  
Bars indicated thus 20 x 3-#5 etc. indicates  
20 lines of bars with 3 lengths per line.  
See Sheet of for parapet reinforcement.

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

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10-22-04



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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"	"	"		
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

- SHEETS

3-#5  $d_4(E)$  bars at 11" cts. Inside Face (Typ. Ea. Corner)

-#4  $d_3(E)$  bars at 11" cts. Outside Face

-#5  $d_1(E)$  bars at 11" cts. Inside Face

Aluminum sheeted construction joints in base of parapet

1'-7" 1'-2" 5" 1'-2" 5" 1'-7"

-#5  $v(E)$  bars at 12" cts. Each End

-Bar Splicers (E) at  $\pm 12$ " cts. for #5  $b(E)$  bars Each End

Back of Abut.

10" typ.

-#  $a(E)$  bars at cts. Top

-#  $a_1(E)$  bars at cts. Bottom

x -#5  $b(E)$  bars equally spaced at  $\pm 12$ " cts. Top of slab

x -#  $b_2(E)$  bars spaced as shown in cross section. Bottom of slab

-#6  $a_2(E)$  bars at cts. Top (Lap with alternate  $a(E)$  bars)

3 x -#5  $b(E)$  bars Top of slab

2-#6  $b_1(E)$  bars Top of slab

-#6  $b_1(E)$  bars at  $\pm 12$ " cts. Top of slab over pier

end to end deck

o. to o. deck

Pier

Figure 10 is a technical drawing of a bridge deck cross-section. It shows a wide deck with a central section and two side sections. The top width is labeled "o. to o. deck" and "face to face parapets", both with a dimension of 1'-7". The deck is divided into sections with slopes indicated as "slope per ft.". The left side shows a cross-section of a parapet with dimensions  $d_2(E)$ ,  $d_3(E)$ ,  $b_1(E)$ ,  $d_1(E)$ ,  $b_2(E)$ , and  $a_1(E)$ . The right side shows a cross-section of a parapet with dimensions  $d_2(E)$ ,  $d_3(E)$ ,  $b_2(E)$ ,  $d_1(E)$ , and  $a_2(E)$ . The total drop is indicated as 2'-10". The bottom of the diagram shows the spacing between beams, labeled "spaces at".

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

10-22-04

CROSS SECTION  
(Looking )

NEAR MIDSPAN

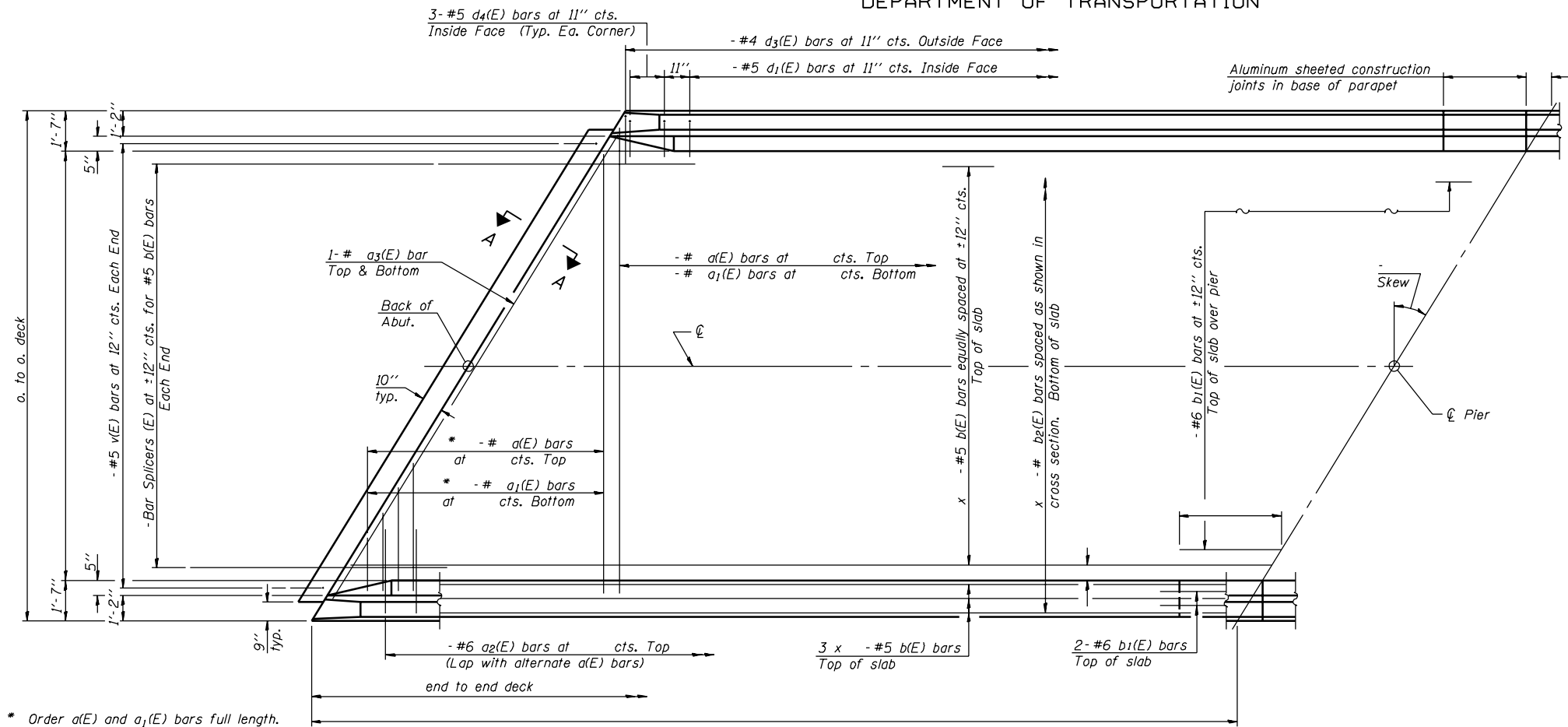


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DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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FED. ROAD DIST. NO. 7	ILLINOIS	FED. AID PROJECT-		

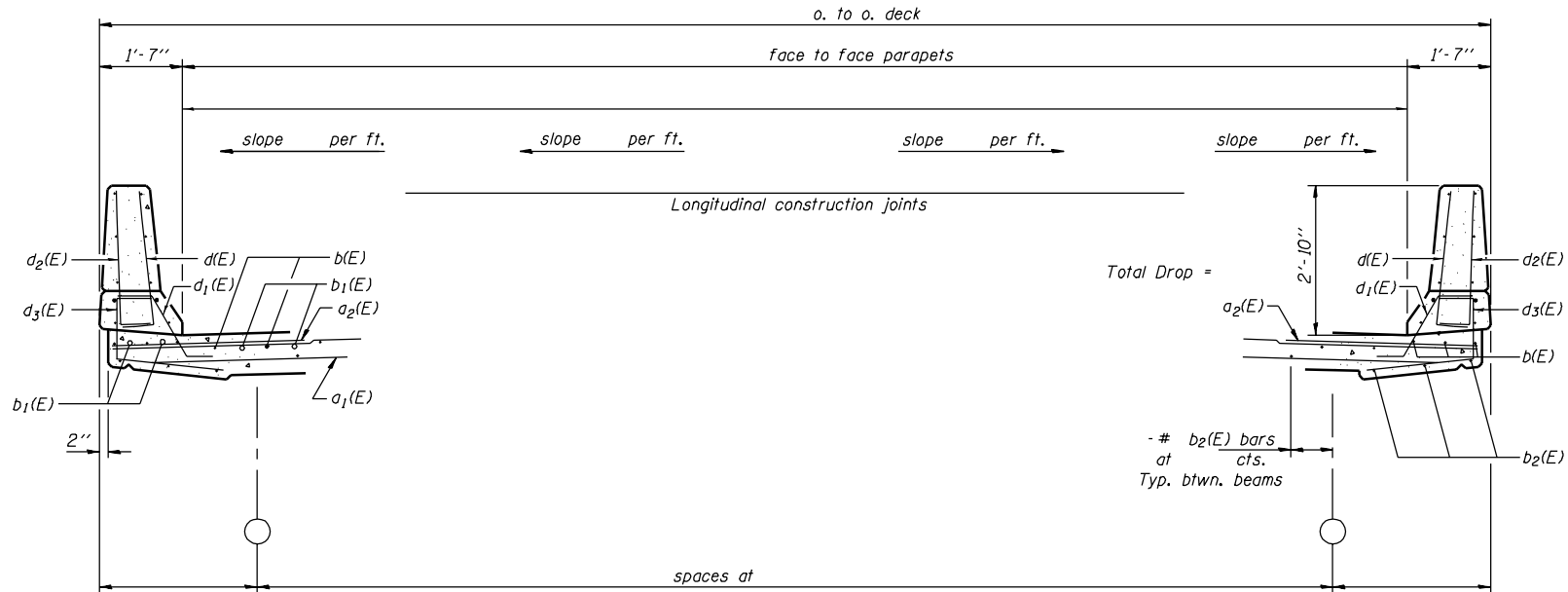
SHEET NO. -  
- SHEETS

Contract #



\* Order a(E) and a1(E) bars full length.  
Cut to fit skew and use remainder  
of bars in opposite end.

HALF PLAN



Notes:  
See Sheet of for superstructure details  
and Bill of Material.  
Reinforcement bars designated (E) shall be  
epoxy coated.  
Bars indicated thus 20 x 3-#5 etc. indicates  
20 lines of bars with 3 lengths per line.  
See Sheet of for parapet reinforcement.

DESIGNED -
CHECKED -
DRAWN -
CHECKED -

EXAMINED  
ENGINEER OF BRIDGE DESIGN  
PASSED  
ENGINEER OF BRIDGES AND STRUCTURES

SI-2-L

10-22-04

CROSS SECTION  
(Looking )



ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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-	-	-		
FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT-	

SHEET NO. -

- SHEETS

[illegible]

DESIGNED -	-	200
CHECKED -	EXAMINED	ENGINEER OF BRIDGE DESIGN
DRAWN -	PASSED	ENGINEER OF BRIDGES AND STRUCTURES
CHECKED -		

10-22-04

CROSS SECTION  
(Looking )



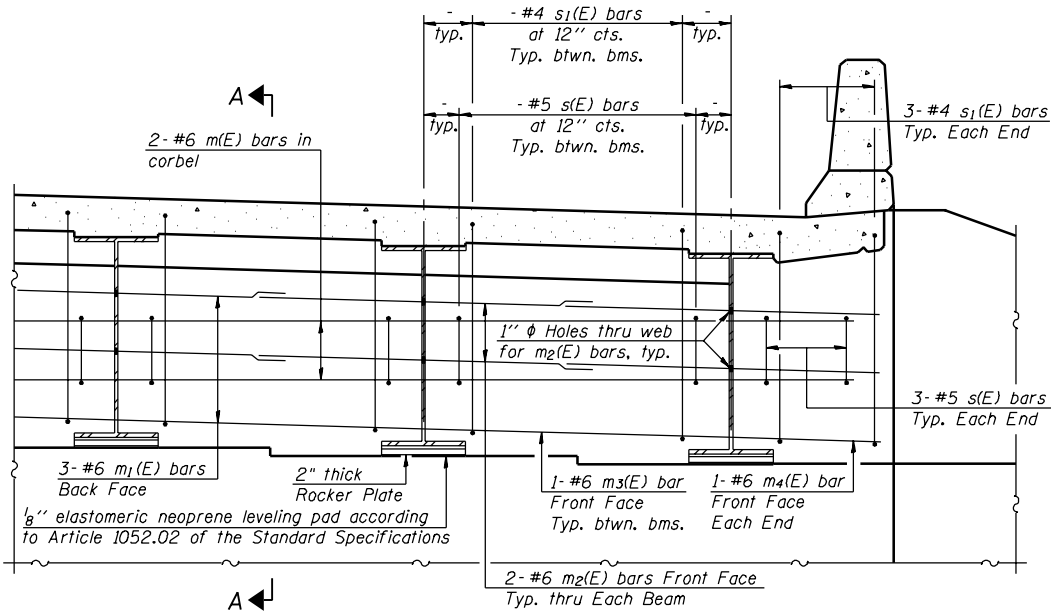
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT -	

SHEET NO. -

- SHEETS

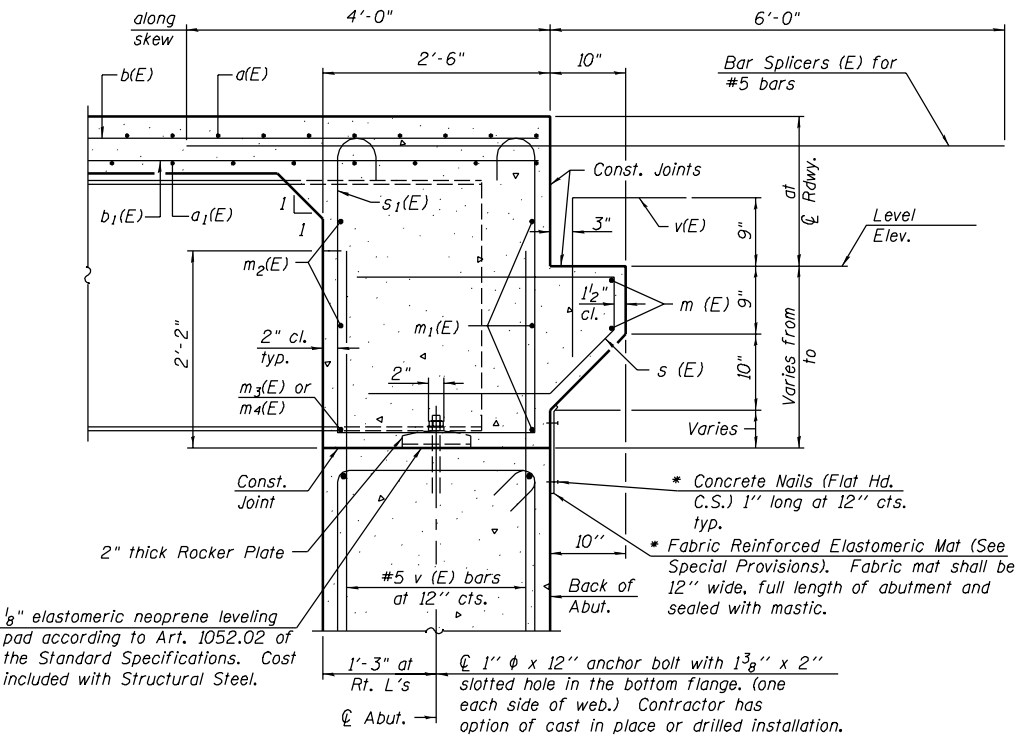
Contract #



DIAPHRAGM ELEVATION AT ABUTMENT

Notes:  
Reinforcement bars in diaphragm are billed with superstructure on sheet of .  
Concrete in diaphragm is included with Concrete Superstructure on sheet of .  
For details of bars s(E) & s<sub>1</sub>(E) see sheet of .  
The s(E) and s<sub>1</sub>(E) bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles to the beams.  
For anchor bolt details see sheet of .

**MIN. BAR LAP**  
#6 bar = 2'-9"



SECTION A-A

Dimensions at right angles to abutment, except as shown.  
\* Cost included with Concrete Superstructure.

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	ENGINEER OF BRIDGE DESIGN	
CHECKED -	PASSED	
	ENGINEER OF BRIDGES AND STRUCTURES	

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10-22-04

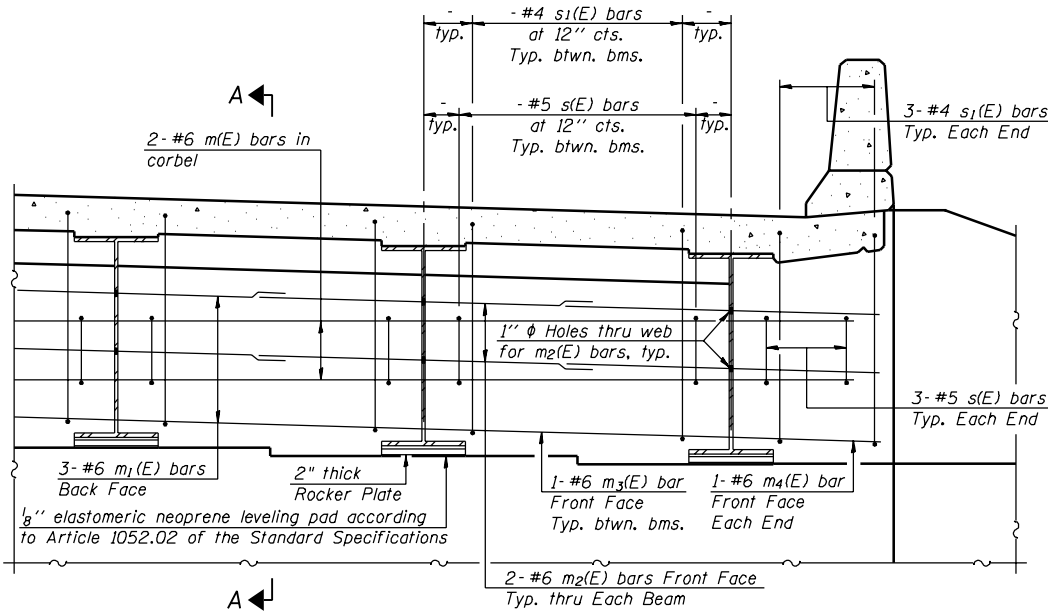


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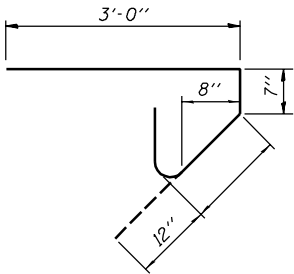
ROUTE NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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FED. ROAD DIST. NO. 7		ILLINOIS	FED. AID PROJECT	

SHEET NO. -  
- SHEETS

Contract #



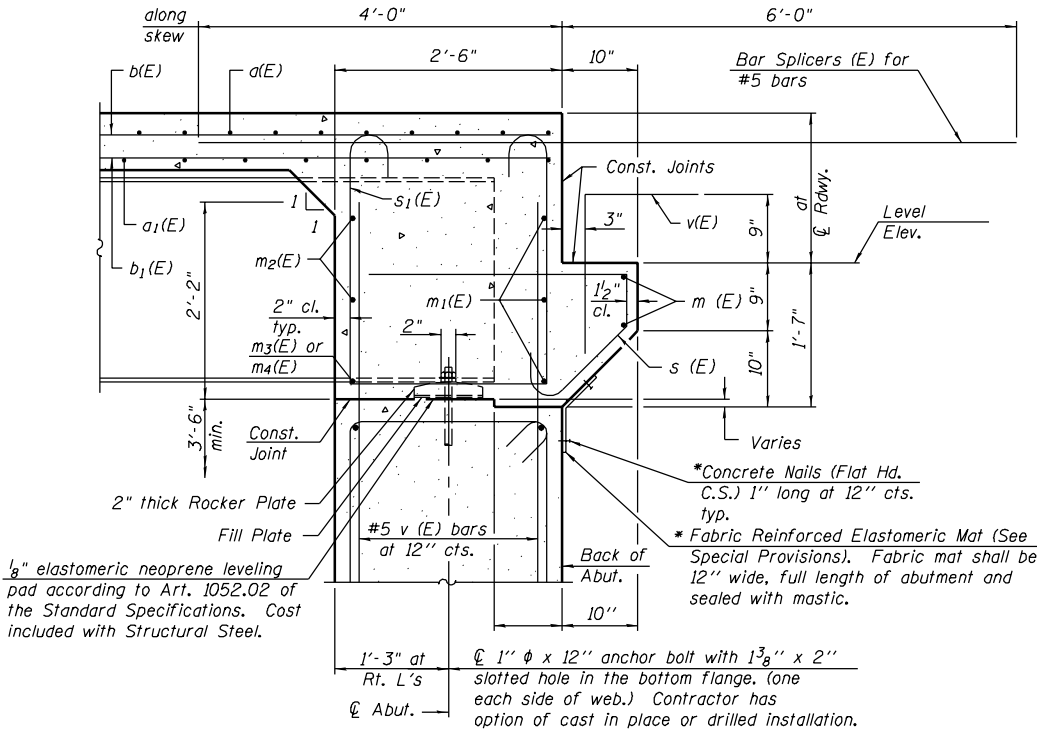
DIAPHRAGM ELEVATION AT ABUTMENT



BAR s(E)

Notes:  
Reinforcement bars in diaphragm are billed with superstructure on sheet of .  
Concrete in diaphragm is included with Concrete Superstructure on sheet of .  
For details of bars s(E) & s<sub>1</sub>(E) see sheet of .  
The s(E) and s<sub>1</sub>(E) bars shall be placed parallel to the beams. Spacing for these bars shall be at right angles to the beams.  
For anchor bolt details see sheet of .

MIN. BAR LAP  
#6 bar = 2'-9"



SECTION A-A

Dimensions at right angles to abutment, except as shown.  
\* Cost included with Concrete Superstructure.

DESIGNED -	-	200
CHECKED -	EXAMINED	
DRAWN -	PASSED	ENGINEER OF BRIDGE DESIGN
CHECKED -		ENGINEER OF BRIDGES AND STRUCTURES